

Town of Blooming Grove Planning Board  
Attn: Christina Gleason  
6 Horton Road  
Blooming Grove NY, 10914



**RE: Traffic Impact Study (Revision No. 2) for Proposed Commercial Development, Craigville Road, Town of Blooming Grove, Orange County, New York; CM Project No. 120-269**

Members of the Board:

Creighton Manning Engineering, LLP (CM) has completed a revised traffic impact study of the proposed commercial development located on Craigville Road (County Route 51) in the Town of Blooming Grove. This report was revised based on comments contained in the January 10, 2024 Completeness Review memorandum prepared by Nelson Pope Voorhis (NPV). This analysis is based on the Final Scoping Document for the Draft Environmental Impact Statement, last revised September 14, 2023, traffic engineering industry standards and the Site Plan prepared by Arden Consulting Engineers, PLLC, which is included under separate cover.

**1.0 Project Description**

The subject site, which is currently utilized for agriculture, is comprised of two parcels designated as Section 54, Block 1, Lot 50.12 as shown on the Orange County Tax Map. The proposed commercial development consists of constructing a new one-story 562,450-square-foot building located on the west side of Craigville Road. The site will be accessed via one full-movement driveway on Craigville Road approximately 610 feet south of the Orange Heritage Trail Overpass. The project is expected to be completed and operational by 2024. A map illustrating the project location and adjacent roadway network is shown in Exhibit 1.



**Exhibit 1 – Project Location**

## 2.0 Existing Conditions

### Roadways Serving the Site

**Craigville Road (County Route 51)** is classified as an Urban Major Collector roadway under the jurisdiction of the Orange County Department of Public Works (OCDPW). The roadway provides north-south travel between the Town of Goshen and NYS Route 17. In the vicinity of the project, the roadway consists of one 11-foot travel lane with a three-foot shoulder in each direction. Approximately 2,000 feet north of the NYS Route 17 Northbound Exit 128 Off-Ramp/Craigville Road intersection, the Orange Heritage Trail (OHT) utilizes the preserved Erie Railroad overpass to cross Craigville Road. The roadway pavement conditions are fair. Turn lanes and sidewalks are not provided along the roadway and there is no posted speed limit.

**NYS Route 17M** is classified as an Urban Minor Arterial roadway under the jurisdiction of New York State Department of Transportation (NYSDOT). The roadway generally runs north-south parallel to NYS Route 17 from the Town of Goshen to the Village of Harriman. In the vicinity of the project, the roadway provides one 11-foot travel lane with variable shoulder width in each direction. The roadway pavement has some wear and tear but is in generally fair condition. Turn lanes are generally not provided and there are no sidewalks provided along the roadway. The posted speed limit is 50 miles per hour.

**NYS Route 17** is classified as a Principal Arterial-Other Freeway/Expressway under the jurisdiction of NYSDOT. The roadway runs north-south parallel to NYS Route 17M from the Town of Goshen to the Village of Harriman. In the vicinity of the project, the roadway provides two 12-foot travel lanes with a ten-foot-wide shoulder on the right side of the road in each direction. The roadway pavement is in generally good condition. There are no turn lanes or sidewalks provided along the roadway. The posted speed limit is 55 miles per hour.

**Old Mansion Road** is classified as a Local roadway under the jurisdiction of the Town of Blooming Grove. The roadway provides north-south travel. In the vicinity of the project, the roadway provides one 11-foot travel lane with variable width shoulders in each direction. The roadway pavement has some wear and tear but is in generally fair condition. Turn lanes are generally not provided and there are no sidewalks provided along the roadway. There is no posted speed limit.

**Kings Highway (County Route 13)** is classified as a Major Collector roadway under the jurisdiction of the OCDPW. The roadway provides north-south travel. The roadway has one 11-foot travel lane with variable shoulder width in each direction. The roadway pavement is in generally good condition. Turn lanes are generally not provided and there are no sidewalks along the roadway. The posted speed limit is 30 miles per hour.

**Lehigh Avenue** is classified as a Local roadway under the jurisdiction of the Town of Chester. The roadway provides east-west travel, parallel to NYS Route 17. The roadway has one 11-foot travel lane with variable shoulder widths in each direction. The roadway pavement has some wear and tear but is in generally fair condition. Turn lanes are generally not provided and there are no sidewalks along the roadway. There is no posted speed limit.

**Academy Avenue** is classified as a Major Collector roadway under the jurisdiction of NYSDOT. The roadway provides east-west travel from Main Street to NYS Route 17. The roadway has one 11-foot travel lane with variable shoulder width in each direction. The roadway pavement has some wear and tear but is in generally fair condition. Sidewalks are provided in each direction along the segment of the roadway. The posted speed limit is 40 miles per hour.

**Summerville Way (NYS Route 94)** is classified as a Minor Arterial roadway under the jurisdiction of NYSDOT. The roadway provides east-west travel and has one 12-foot travel lane in each direction. The roadway pavement has some wear and tear and is in generally fair condition. There are turn lanes and sidewalks provided along a segment of the road. The posted speed limit is 40 miles per hour.

**Museum Village Road** is classified as a Local roadway under the jurisdiction of the Town of Blooming Grove. The roadway provides north-south travel and has one 13-foot lane in each direction. The roadway pavement is in generally good condition. There are no sidewalks provided along the roadway. There is no posted speed limit.

#### Study Intersections

- **NYS Route 17 Northbound Exit 128 Off Ramp/Craigville Road:** This is a three-way unsignalized intersection operating with stop control on the eastbound NYS Route 17 Northbound Exit 128 Off-Ramp approach. The eastbound NYS Route 17 Northbound Exit 128 Off-Ramp is a one-way approach that provides one shared left-turn/right-turn lane. The northbound Craigville Road approach provides one through lane. The southbound Craigville Road approach provides one through lane. Exhibit 2 depicts the intersection.



Exhibit 2 – NYS Route 17 Northbound Exit 128 Ramp and Craigville Rd Intersection

- **Craigville Road and Old Mansion Road:** This is a three-way unsignalized intersection operating with a stop control on the westbound Old Mansion Road. The westbound Old Mansion Road approach provides one shared left-turn/right-turn lane. The northbound Craigville Road approach provides one shared through/right-turn lane. The southbound Craigville Road approach provides a left-turn/through lane. Exhibit 3 depicts the intersection.



Exhibit 3 – Craigville Road and Old Mansion Road Intersection

- **NYS Route 17M/Craigville Road:** This is a three-way unsignalized intersection operating with stop control on the southbound Craigville Road approach. The eastbound NYS Route 17M approach provides one shared left-turn/through lane. The westbound NYS Route 17M approach provides one shared through/right-turn lane. The southbound Craigville Road approach provides one shared left-turn/right-turn lane. Exhibit 4 depicts the intersection.



Exhibit 4 – NYS Route 17M and Craigville Rd Intersection

- **NYS Route 17M/NYS Route 17 Southbound Exit 127 On-Ramp:** This is a three-way unsignalized intersection. The eastbound NYS Route 17M approach provides one shared left-turn/through lane. The westbound NYS Route 17M approach provides one shared through/right-turn lane. The third leg of the intersection is a one-way road departing the intersection as the NYS Route 17 Southbound Exit 127 On-Ramp. The NYS Route 17M eastbound and westbound approaches are provided separate receiving lanes for the on-ramp which eventually merge. Exhibit 5 depicts the intersection.



Exhibit 5 – NYS Route 17M/NYS Route 17 Southbound Exit 127 On-Ramp

- **NYS Route 17M/Kings Highway/Lehigh Avenue:** This is a four-leg signalized intersection operating under actuated-uncoordinated traffic signal control. The northbound and southbound legs of the intersection are offset by approximately 250 feet. The eastbound NYS Route 17M approach provides one shared left-turn/through lane and one exclusive right turn lane. The westbound NYS Route 17M approach provides one shared left-turn/through/right-turn lane. The northbound Kings Highway approach provides one exclusive left-turn lane and one channelized right-turn lane that serves as the de facto through lane to Lehigh Avenue and operates under yield control. The southbound Lehigh Avenue approach provides a shared left-turn/through/right-turn lane. Exhibit 6 depicts the intersection.



Exhibit 6 – NYS Route 17M/Kings Hwy/Lehigh Avenue Intersection

- **NYS Route 17M/NYS Route 94/Academy Avenue:** This is a four-leg signalized intersection operating under actuated-uncoordinated traffic signal control. The eastbound NYS Route 17M approach provides one exclusive left-turn lane, one exclusive through lane and one shared through/right-turn lane. The westbound NYS Route 17M approach provides dual left-turn lanes and one shared through/right-turn lane. The northbound NYS Route 94 approach provides one exclusive left-turn lane, one exclusive through lane and one exclusive right-turn lane. The southbound Academy Avenue approach provides one exclusive left-turn lane and one shared through/right-turn lane. All four legs of the intersection provide marked crosswalks, pedestrian signals, and countdown timers. Exhibit 7 depicts the intersection.



Exhibit 7 – NYS Route 17M/NYS Route 94/Academy Ave Intersection

- **NYS Route 94/NYS Route 17 Northbound Exit 126 On/Off-Ramp:** This is a four-leg signalized intersection operating under actuated-uncoordinated traffic signal control. The eastbound NYS Route 94 approach provides one exclusive left-turn lane and one exclusive through lane. The westbound NYS Route 94 approach provides one exclusive through lane and one exclusive right-turn lane. The northbound NYS Route 17 Exit 126 Off-Ramp approach provides one shared left-turn/through lane and one exclusive right-turn lane. The south leg of the intersection provides a marked cross-walk, pedestrian signals, and countdown timers. Exhibit 8 depicts the intersection.



Exhibit 8 – NYS Route 94/NYS Route 17 Northbound Exit 126 On/Off-Ramp Intersection

- **NYS Route 94/NYS Route 17 Southbound Exit 126 On/Off-Ramp:** This is a four-leg signalized intersection operating under actuated-uncoordinated traffic signal control. The eastbound NYS Route 94 approach provides one exclusive through lane and one exclusive right-turn lane. The westbound NYS Route 94 approach provides one exclusive left-turn lane and one exclusive through lane. The southbound NYS Route 17 Exit 126 Off-Ramp approach provides one exclusive left-turn lane and one shared through/right-turn lane. The south leg of the intersection provides a marked crosswalk, pedestrian signals, and countdown timers. Exhibit 9 depicts the intersection.



Exhibit 9 – NYS Route 94/NYS Route 17 Southbound Exit 126 On/Off-Ramp Intersection

- **NYS Route 17M/Museum Village Road:** This is a four-leg unsignalized intersection operating with stop control on the Museum Village Road approaches. The eastbound NYS Route 17M approach provides a shared left-turn/through/right-turn lane. The westbound NYS Route 17M approach provides a shared left turn/through/right-turn lane. The northbound Museum Village Road approach provides a shared left-turn/through/right-turn lane. The southbound Museum Village Road approach provides a shared left-turn/through/right-turn lane. Exhibit 10 depicts the intersection.



Exhibit 10 – NYS Route 17M and Museum Village Road Intersection

- **NYS Route 208/Museum Village Road:** This is a three-way unsignalized intersection operating with stop control on the Museum Village Road approach. The eastbound Museum Village Road approach provides a shared left-turn/right-turn lane. The northbound NYS Route 208 approach provides a shared left-turn/through lane. The southbound NY Route 208 approach provides a shared through/right-turn lane. Intersection improvements have been proposed by Collier's Engineering & Design based on a traffic impact study for a proposed commercial development last revised March 8, 2022. In this study it was discussed that in future conditions a traffic signal and geometric improvements will be completed. Therefore, for the No-Build and Build analysis in this study, this intersection will be modeled as a signalized intersection. The eastbound Museum Village Road approach will provide an exclusive left-turn lane and an exclusive right-turn lane. The northbound NYS Route 208 approach will provide an exclusive left-turn lane and an exclusive through lane. The southbound NYS Route 208 approach will provide an exclusive through lane and an exclusive right-turn lane. Exhibit 11 depicts the existing intersection configuration.



Exhibit 11 – NYS Route 208 and Museum Village Road Intersection

### Motor Vehicle Collision Analysis

Motor vehicle collision data was obtained from the NYSDOT for the most recent three-year period – March 1, 2019 to February 28, 2022. The data includes collisions that occurred at the study intersections. Table 1 summarizes the number of collisions at each intersection and the crash rate of each intersection.

**Table 1 – Crash Analysis of Study Intersections**

Study Intersection	Number of Crashes	Rate   State Average Rate   (Acc/MEV)
NYS Route 17M and Craigville Road	6	0.93   0.19
Craigville Road and Old Mansion Road	3	0.66   0.19
NYS Route 17 Northbound Exit 128 Off-Ramp and Craigville Road	0	0   0.19
NYS Route 17M and NYS Route 17 Southbound Exit 127 On-Ramp <sup>1</sup>	3	N/A
NYS Route 17M and Kings Highway/Lehigh Avenue	2	0.09   0.56
NYS Route 17M and NYS Route 94/Academy Avenue	21	0.66   0.26
NYS Route 94 and NYS Route 17 Northbound Exit 126 On/Off-Ramp	7	0.27   0.56
NYS Route 94 and NYS Route 17 Southbound Exit 126 On/Off-Ramp	7	0.30   0.56
NYS Route 17M and Museum Village Road	12	1.18   0.31
NYS Route 208 and Museum Village Road	15	0.56   0.19

<sup>1</sup>Crashed reported at this intersection were reported as non-intersection accidents. Therefore, intersection crash rate was not calculated this intersection.

As shown in Table 1, the majority of intersections have collision rates below the State average for similar intersections. It is noted that following intersections have collision rates above the State average for similar intersections:

- NYS Route 17M/Craigville Road
- Craigville Road/Old Mansion Road
- NYS Route 17M and NYS Route 94/Academy Avenue
- NYS Route 17M/Museum Village Road
- NYS Route 208/Museum Village Road

The above average intersection collision rate of Craigville Road/Old Mansion Road included one animal strike, and no apparent crash pattern. Regarding the other four intersections with above average intersection collision rates, please refer to **Section 6.0**. The Collision Summary Sheets and TE-213's are included under Attachment A.

### Transit

Coach USA/Shortline utilizes NYS Route 17M for a commuter bus service connecting the Orange County area with the New York Metropolitan Area. The Chester Park & Ride Station is located on NYS Route 94 just west of the NYS Route 94/NYS Route 17 Southbound Exit 126 On/Off-ramp.

CM also contacted the Washingtonville Central School District, Monroe-Woodbury Central School District, and the Chester Union Free School District in regards to school bus routes within the study area. A representative from the Monroe-Woodbury Central School District confirmed that buses travel within the study area from 5:45 AM to 9:15 AM and 1:15 PM to 6:00 PM. Approximately 15 buses run on Craigville Road between Oxford Depot Road and NYS Route 17M. Approximately 20 buses run on Museum Village Road between NYS 17M and NYS Route 208. Approximately 25 buses run on NYS Route 17M between Craigville Road and Museum Village Road. Due to confidentiality policies, the Washingtonville Central School District and Chester Union Free School District were unable to share their school bus routes for inclusion in this study. A record of our correspondence is included under Attachment B. As discussed Section 2.0 – Data Collection, the data collection occurred on dates when

[schools were in regular session. A more detailed discussion of the traffic operations in the Build condition is included under Section 3.0 Traffic Assessment – Traffic Operations. While the exact location of bus stops could not be obtained by our inquiries to the various school districts, it is assumed given the absence of residential dwellings along Craigville Road west of the proposed driveway, which is the direction where the majority of traffic is expected to arrive/depart, that there are no bus stops along this segment. Based on this assumption, there is little to no opportunity for school bus pick-up/drop-off operations to be impacted by the proposed development.](#)

### Data Collection

In accordance with the requirements of the DEIS Scoping Document, two Automatic Traffic Recorders (ATR's) were installed to collect traffic volume, speed, and classification data over several days in March 2022. These ATR's were placed at the following locations:

- Craigville Road | 400 feet North of NYS Route 17M
- NYS Route 17M | 176 feet East of Craigville Road

These ATRs collected continuous traffic volume data from Wednesday, March 2, 2022, to Tuesday, March 8, 2022. The traffic volume data confirmed that the weekday peak periods are 7:00 AM to 9:00 AM and 4:00 PM to 6:00 PM, and the weekend peak period is on Saturday from 11:00 AM to 2:00 PM. Therefore, turning movement counts (TMCs) were performed on Thursday, March 10, 2022, during weekday morning peak period of 7:00 AM to 9:00 AM and weekday evening peak period of 4:00 PM to 6:00 PM while schools were in session. TMCs were also performed on Saturday, March 26, 2022 from 11:00 AM to 2:00 PM. These periods coincide with the anticipated peak operation times of the proposed use as well as adjacent street traffic. The observed peak hours during the weekdays were 7:45 AM to 8:45 AM and 4:30 PM to 5:30 PM. The observed peak hour on Saturday was 11:45 AM to 12:45 PM. These counts were conducted at the following intersections:

- NYS Route 17M/Craigville Road
- Craigville Road/Old Mansion Road
- NYS Route 17 Southbound Exit 128 Off-Ramp/  
Craigville Road
- NYS Route 17/17 On-Ramp
- NYS Route 17M/NYS 17 Southbound Exit 127 On-  
Ramp
- NYS Route 17M/Kings Highway/Lehigh Avenue
- NYS Route 17M/NYS Route 94/Academy Avenue
- NYS Route 94/NYS Route 17 Northbound Exit 126  
On/Off- Ramp
- NYS Route 94/NYS Route 17 Southbound Exit 126  
On/Off- Ramp
- NYS Route 17M/Museum Village Road
- NYS Route 208/Museum Village Road
- NYS Route 17M/Goose Pond Parking Driveway

It is important to note that the Novel Coronavirus/COVID-19 pandemic was anticipated to have an effect on the turning movement counts. CM compared observed two-hour volumes from each study period to historical data collected by NYSDOT Automatic Traffic Recorders (ATRs).<sup>1</sup> CM found that the 2022 turning movement count volumes were higher than the historical pre-COVID traffic conditions. These existing traffic volumes are shown on Figure 1. The raw turning movement data and ATR data are included under Attachment C.

## **3.0 Traffic Assessment**

### Trip Generation

Trip generation determines the quantity of traffic expected to travel to and from a given site. The Institute of Transportation Engineers' (ITE) *Trip Generation Manual*, 11<sup>th</sup> Edition, is the industry standard used for estimating trip generation for proposed land uses based on data collected at similar uses. Upon review of the *Trip Generation Manual*, CM concluded that Land Use Code (LUC) 150 "Warehouse" best represented the proposed use. The ITE description for LUC 150 is as follows, "a warehouse is primarily devoted to the storage of materials, but it may also include office and maintenance areas." Table 2 summarizes the trip generation for the weekday AM, weekday PM peak hours, and Saturday midday peak hour.

<sup>1</sup> NYSDOT ATR's | Craigville Road Station 838100 (2018) | NYS Route 17M Station 830682 (2016) | Kings Highway Station 838034 (2018)

**Table 2 – Trip Generation Summary for Proposed Use**

Land Use	Independent Variable	Weekday AM Peak			Weekday PM Peak			Saturday Midday Peak		
		Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total
Warehouse – LUC 150	562,450 SF									
	Non-Trucks	64	16	80	17	60	77	18	10	28
	Trucks	6	5	11	9	8	17	0	0	0
	<b>Total Site-Generated Trips</b>	<b>70</b>	<b>21</b>	<b>91</b>	<b>26</b>	<b>68</b>	<b>97</b>	<b>18</b>	<b>10</b>	<b>28</b>

Table 2 shows the site will generate 91 total trips in the weekday AM peak hour, 97 total trips in the weekday PM peak hour, and 28 total trips in the Saturday midday peak hour. It is important to note that there is no “pass-by” component of the traffic associated with the proposed development.

#### Future Traffic Volumes

As stated earlier, the proposed development is anticipated to be completed and operational by 2024. In accordance with the DEIS Scoping Document and Town of Blooming Grove Planner, Bonnie Franson, traffic projections were prepared for a design year that is five years beyond the estimated time of completion (ETC+5) – 2029. CM reviewed traffic volume data from various ATRs published on the NYSDOT Traffic Data Viewer and calculated an annual growth rate of +1.0%.<sup>2</sup> This growth rate was applied to the 2022 existing volumes and compounded annually for two-years resulting in the Background Growth Traffic volumes as shown on Figure 2. Under Freedom of Information Law, CM requested data on proposed development projects in the Town of Blooming Grove, the Town of Monroe, and the Town of Chester. If approved and constructed, the following proposed development projects could potentially increase traffic within the study area. Listed in Table 3 are the proposed development projects that could potentially increase traffic in the study intersections.

**Table 3 – Other Planned Development Project**

Project	Type	Location	Source	Weekday AM	Weekday PM	Saturday Midday
Clovewood Development	Residential	NY 208 and Clove Road	Maser Consulting	96	131	0
Stonegate Dr.	Retail/Office Space	NY 208 and Stonegate Drive	ITE	23	35	3
Museum Village Rd.	Retail/ Office Space	NY 208 and Museum Village Road	ITE	323	477	511
Davidson Drive	Light Industrial	Davidson Drive	Maser Consulting	67	61	7
The Greens @ Chester	Residential	NY 94	John Meyer Consulting	213	283	261
Davidson Drive Holdings	Light Industrial	Lake Station Road	ITE	24	20	11
Trestle Tree	Industrial	NY 17 and Trestle Tree Lane	Maser Consulting	74	68	8
208 Business Center	Commercial	NY 208 and Museum Village Road	Colliers Engineering	9	18	19

The volumes generated by the other developments are shown in Figure 3. These volumes were then added to the grown 2024 background volumes to present the 2024 No-Build conditions which are shown on Figure 4.

<sup>2</sup> NYSDOT ATR's | NYS Route 17M Station 830682 | Kings Highway Station 838034

Traffic generated by the proposed project was distributed to the adjacent roadways based on existing observed travel patterns in the project area and the probable travel routes of truck drivers and employees. The distribution of employee vehicles for the warehouse and office uses is expected to be balanced between NYS Route 17 and NYS Route 17M. Non-truck trip distribution patterns for the site are shown in Figure 5A and associated volumes are shown in Figure 6A.

Access to NYS Route 17 is expected to influence the trip-making behavior of the truck drivers. This analysis assumes that the majority (60 percent) of truck drivers will be drawn from NYS Route 17 Northbound Exit 128 Off-Ramp. The remaining 40 percent of trucks are expected to arrive from the north via NYS Route 17 southbound accessing NYS Route 17M via Exit 126 to NYS Route 94 in Chester. Additionally, it is assumed that all outbound truck traffic will utilize the intersection of NYS Route 17M and Craigville Road en route to NYS Route 17 via the NYS Route 17 Southbound Exit 127 On-Ramp and NYS Route 17 Northbound Exit 126 On-Ramp. CM investigated an alternative route for trucks approaching from the west via NYS Route 17 Southbound Exit 129 and Exit 130 and found that restrictions on Museum Village Road and intersection geometry at NYS Route 17M/NYS Route 208 made the route either not feasible or undesirable. To ensure the anticipated truck routing, CM recommends that directional signs be placed at the Site Driveway and at the intersection of NYS Route 17M/Craigville Road to direct trucks north on NYS Route 17M. CM prepared truck turning diagrams at intersections where trucks associated with the proposed development are expected to enter.<sup>3</sup> The diagrams depict WB-67 trucks making maneuvers associated with their navigation to/from the site. For example, the truck turning diagram for the Craigville Road/NYS Route 17M intersections only depicts a truck turning right from Craigville Road onto NYS Route 17M and a truck turning left from NYS Route 17M onto Craigville Road. These diagrams can be found on Figure 10A-14B. The diagrams indicate that the existing configurations for the majority of intersections allow a WB-67 to conduct the anticipated movements. However, based on the diagram shown in Figure 11A, the shoulder on the northeast corner of the intersection of Craigville Road and NYS Route 17M may need to be widened and reinforced.

It is important to note that on July 14, 2021, the NYSDOT approved the designation of NYS Route 17M as an Access Highway for Special Dimension Vehicles and that such designation went into effect on September 1, 2021. The NYSDOT approval letter is included under Attachment D. Craigville Road has automatic access for the Special Dimension Vehicles given that it is within one mile of a NYS Route 17, a Qualifying Highway. Truck distribution patterns are shown on Figure 5B while associated site generated truck volumes are shown on Figure 6B. The totaled assigned volumes for non-truck and truck were added to the 2024 No-build traffic volumes, resulting in the 2024 Build traffic volumes shown in Figure 7.

In order to evaluate the ETC+5 analysis, the annual growth rate was applied to the 2024 grown background traffic volumes and compounded for five years resulting in the Background Growth Traffic volumes as shown in Figure 8. Other development volumes (Figure 3) and site generated volumes (Figure 6A and 6B) were then added to the 2029 grown traffic volumes, resulting in the 2029 Build ETC+5 volumes as shown in Figure 9.

### Traffic Operations

Intersection Level of Service (LOS) and capacity analysis relate traffic volumes to the physical characteristics of an intersection. Intersections evaluations were made using Synchro Version 11 software, which automates the procedures contained in the Highway Capacity Manual. Tables 4A-4C summarize the result of the level of service calculations for the proposed project. The detailed level of service analyses are included under Attachment E.

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<sup>3</sup> CM prepared truck turning diagrams using Autoturn. These diagrams, which are on Figures 10A-14B, depict WB-67 making the anticipated movements at the expected study intersections.

Table 4A – Level of Service Summary<sup>4</sup>

Intersection	Control	Weekday AM Peak Hour			
		2022 Existing	2024 No-Build	2024 Build	2029 Build
NYS Route 17M/Craigville Road	U				
NYS Route 17M EB      LT Craigville Road SB      LR		A (7.7) B (10.6)	A (7.7) B (11.1)	A (7.8) B (11.7)	A (7.9) B (11.8)
NYS Route 17 Northbound Exit 128 Off-Ramp/Craigville Road	U				
NYS Route 17 Northbound Exit 128 Off-Ramp EB      LR		A (9.0)	A (9.6)	B (10.0)	B (10.1)
Craigville Road/Site Driveway	U				
Site Driveway EB      L R Craigville Road NB      TL		-- --	-- --	B (10.6) A (8.9) A (7.5)	B (10.6) A (8.8) A (7.5)
NYS Route 17M/NYS Route 17 Southbound Exit 127 On-Ramp	U				
NYS Route 17M EB      LR		A (5.6)	A (8.4)	A (8.4)	A (8.5)
NYS Route 17M/Kings Hwy/Lehigh Ave <sup>5</sup>	S				
NYS Route 17M EB      T R NYS Route 17M WB      LTR Kings Highway NB      L TR Lehigh Avenue SB      LTR		D (32.3) C (29.9) C (31.8) D (39.0) D (28.5) D (37.7)	D (36.3) C (33.0) D (34.8) D (49.3) C (33.4) D (37.2)	D (38.9) C (33.1) D (35.3) D (50.9) C (34.9) D (37.5)	D (39.3) D (33.4) D (35.6) D (51.1) D (34.7) D (37.2)
NYS Route 17/NYS Route 94/Academy Ave	S				
NYS Route 17M EB      L T TR NYS Route 17M WB      L TR NYS Route 94 NB      L T R Academy Ave SB      L TR		D (43.1) C (22.5) C (23.0) C (24.3) B (16.8) C (34.7) B (15.9) B (19.1) D (53.4) C (23.6)	D (43.5) C (23.3) C (23.9) C (25.1) B (17.2) D (35.4) B (16.5) C (23.2) D (53.2) C (24.2)	D (43.2) C (23.3) C (23.9) C (25.2) B (17.1) D (35.5) B (16.6) C (23.8) D (52.7) C (24.5)	D (43.3) C (23.8) C (24.4) C (25.7) B (17.5) D (35.8) B (16.8) C (24.7) D (51.7) C (24.9)
Overall		C (23.8)	C (24.8)	C (24.9)	C (25.4)
NYS Route 94/NYS Route 17 Northbound Exit 126 On/Off-Ramp	S				
NYS Route 94 EB      L T NYS Route 94 WB      T TR NYS Route 17 Northbound Exit 126 On/Off-Ramp      LT NB      R		A (7.0) A (4.0) A (8.8) B (11.6) B (19.5) B (17.1)	A (8.3) A (4.8) A (9.8) B (13.0) C (20.7) C (17.7)	A (8.2) A (4.8) A (9.7) B (12.9) C (20.6) C (17.6)	A (8.4) A (4.9) A (9.8) B (13.1) C (20.9) C (17.8)
Overall		A (9.5)	B (10.4)	B (10.3)	B (10.4)

<sup>4</sup>U = Unsignalized intersection | S = Signalized intersection | EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches | L, T, R = Left-turn, Through, and/or Right-turn movements | X (Y.Y) = Level of service (Average delay in seconds per vehicle)

<sup>5</sup> There is no overall LOS provided for this intersection as it was evaluated as two separate intersections due to its operation as a clustered intersection.

**Table 4A cont. – Level of Service Summary<sup>6</sup>**

Intersection	Control	Weekday AM Peak Hour			
		2022 Existing	2024 No-Build	2024 Build	2029 Build
NYS Route 94/NYS Route 17 Southbound Exit 126 On/Off-Ramp	S	B (13.5)	B (14.9)	B (15.0)	B (15.1)
		B (15.6)	B (18.0)	B (18.0)	B (18.2)
NYS Route 94 WB		A (8.1)	A (9.6)	B (9.6)	B (9.8)
		A (6.3)	A (7.4)	A (7.5)	A (7.6)
NYS Route 17 Southbound Exit 126 Off-Ramp SB		B (16.1)	B (19.3)	B (19.1)	B (19.3)
		B (14.9)	B (16.7)	B (16.4)	B (16.6)
Overall		B (12.6)	B (14.7)	B (15.0)	B (15.2)
NYS Route 17M/Museum Village Road	U	A (7.9)	A (7.9)	A (7.9)	A (7.9)
NYS Route 17M, EB		A (7.5)	A (7.6)	A (7.6)	A (7.6)
NYS Route 17M, WB		B (13.3)	C (15.2)	C (15.7)	C (15.9)
Museum Village Road, NB		C (17.2)	C (22.0)	D (23.6)	D (24.7)
NYS Route 208/Museum Village Road	U	F (97.3)	--	--	--
Museum Village Road, EB		A (10.4)	--	--	--
NYS Route 208, NB					
NYS Route 208/Museum Village Road	S	--	D (51.9)	D (51.8)	D (51.8)
Museum Village Road, EB		--	D (43.4)	D (43.5)	D (43.4)
		--	B (13.4)	B (11.2)	B (12.1)
NYS Route 208, NB		--	A (2.9)	A (2.9)	A (2.9)
		--	B (11.2)	B (10.6)	B (11.0)
NYS Route 208, SB		--	A (4.8)	A (4.6)	A (4.7)
Overall			B (12.8)	B (12.2)	B (12.4)
Craigville Road/ Old Mansion Road	U	A (9.4)	A (9.7)	B (10.1)	B (10.2)
Old Mansion Road, WB		A (7.7)	A (7.7)	A (7.7)	A (7.8)
Craigville Road, SB					

<sup>6</sup>U = Unsignalized intersection | S = Signalized intersection | EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches | L, T, R = Left-turn, Through, and/or Right-turn movements | X (Y.Y) = Level of service (Average delay in seconds per vehicle)

Table 4B – Level of Service Summary<sup>7</sup>

Intersection	Control	Weekday PM Peak Hour			
		2022 Existing	2024 No-Build	2024 Build	2029 Build
NYS Route 17M/Craigville Road	U				
NYS Route 17M EB      LT Craigville Road SB      LR		A (8.0) B (11.5)	A (8.1) B (12.6)	A (8.4) B (14.4)	A (8.4) B (14.7)
NYS Route 17 Northbound Exit 128 Off-Ramp/Craigville Road	U				
NYS Route 17 Northbound Exit 128 Off-Ramp EB      LR		A (9.3)	A (9.8)	B (10.7)	B (10.7)
Craigville Road/Site Driveway	U				
Site Driveway EB      L R Craigville Road NB      TL		-- --	-- --	B (10.4) A (9.2) A (7.7)	B (10.4) B (9.2) A (7.7)
NYS Route 17M/NYS Route 17 Southbound Exit 127 On-Ramp	U				
NYS Route 17M EB      LR		A (8.6)	A (9.2)	A (9.3)	A (9.4)
NYS Route 17M/Kings Hwy/Lehigh Ave <sup>8</sup>	S				
NYS Route 17M EB      T R NYS Route 17M WB      LTR Kings Highway NB      L TR Lehigh Avenue SB      LTR		D (37.3) C (37.1) D (44.5) D (48.7) D (37.7) D (36.6)	D (38.8) C (37.8) D (47.5) E (55.7) D (39.9) D (36.5)	D (39.2) D (37.8) D (54.1) E (55.7) D (39.4) D (36.4)	D (39.3) C (37.9) E (27.1) E (56.8) D (39.6) D (36.5)
NYS Route 17M/NYS Route 94/Academy Ave	S				
NYS Route 17M EB      L T TR NYS Route 17M WB      L TR NYS Route 94 NB      L T R Academy Ave SB      L TR		D (38.4) C (27.4) C (28.1) C (30.6) C (24.5) F (88.7) B (19.1) C (21.3) D (45.6) C (29.8)	D (40.7) C (29.6) C (30.3) D (35.4) C (25.2) F (121.9) C (20.6) C (23.7) D (47.5) C (31.8)	D (40.9) C (29.9) C (30.5) D (36.0) C (25.2) F (124.6) C (20.8) C (24.5) D (47.7) C (32.1)	D (41.3) C (30.3) C (31.0) D (38.1) C (25.6) F (139.9) C (21.2) C (25.2) D (47.9) C (33.0)
Overall		C (34.2)	D (39.4)	D (39.9)	D (42.0)
NYS Route 94/NYS Route 17 Northbound Exit 126 On/Off-Ramp	S				
NYS Route 94 EB      L T NYS Route 94 WB      T TR NYS Route 17 Northbound Exit 126 On/Off-Ramp      LT NB      R		A (9.9) A (6.0) B (12.4) B (15.4) C (20.5) B (18.8)	B (13.3) A (7.7) B (15.1) C (20.9) C (25.7) B (19.8)	B (13.4) A (7.8) B (15.1) C (21.1) C (26.0) B (19.9)	B (13.9) A (8.0) B (15.3) C (22.1) C (27.1) C (20.3)
Overall		B (12.6)	B (16.2)	B (16.8)	B (16.9)

<sup>7</sup> U = Unsignalized intersection | S = Signalized intersection | EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches | L, T, R = Left-turn, Through, and/or Right-turn movements | X (Y.Y) = Level of service (Average delay in seconds per vehicle)

<sup>8</sup> There is no overall LOS provided for this intersection as it was evaluated as two separate intersections due to its operation as a clustered intersection.

**Table 4B cont. – Level of Service Summary<sup>9</sup>**

Intersection	Control	Weekday PM Peak Hour			
		2022 Existing	2024 No-Build	2024 Build	2029 Build
NYS Route 94/NYS Route 17 Southbound Exit 126 On/Off-Ramp	S	B (16.4)	B (17.4)	B (17.6)	B (18.1)
		B (13.0)	B (13.8)	B (13.9)	B (14.0)
NYS Route 94 WB		A (9.4)	B (10.5)	B (10.6)	B (10.8)
		A (7.4)	A (9.7)	A (9.7)	B (10.0)
NYS Route 17 Southbound Exit 126 Off-Ramp SB		B (18.6)	C (20.1)	C (20.1)	C (20.4)
		B (16.4)	B (18.8)	B (18.6)	B (18.8)
Overall		B (13.6)	B (15.0)	B (15.0)	B (15.3)
NYS Route 17M/Museum Village Road	U	A (8.3)	A (8.4)	A (8.4)	A (8.5)
		A (7.7)	A (7.7)	A (7.7)	A (7.7)
NYS Route 17M, WB		B (14.6)	C (17.9)	C (19.4)	C (19.9)
Museum Village Road, NB		E (37.7)	F (91.3)	F (128)	F (146.2)
Museum Village Road, SB					
NYS Route 208/Museum Village Road	U	F (300+)	--	--	--
		A (8.8)	--	--	--
Museum Village Road, EB					
NYS Route 208, NB	LR				
NYS Route 208/Museum Village Road	S	--	D (51.5)	D (51.7)	D (51.8)
		--	D (47.4)	D (47.5)	D (47.5)
Museum Village Road, EB		--	A (8.5)	A (8.3)	A (8.5)
		--	B (16.7)	B (17.3)	B (18.4)
NYS Route 208, NB		--	B (12.1)	B (12.0)	B (12.2)
NYS Route 208, SB		--	A (7.9)	A (7.9)	A (7.9)
Overall			C (20.0)	C (20.3)	C (20.9)
Craigville Road/ Old Mansion Road	U	B (11.1)	B (11.3)	B (11.5)	B (11.7)
		A (7.7)	A (7.8)	A (7.8)	A (7.8)
Old Mansion Road, WB					
Craigville Road, SB	LT				

<sup>9</sup> U = Unsignalized intersection | S = Signalized intersection | EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches | L, T, R = Left-turn, Through, and/or Right-turn movements | X (Y.Y) = Level of service (Average delay in seconds per vehicle)

Table 4C – Level of Service Summary<sup>10</sup>

Intersection	Control	Saturday Midday Peak Hour			
		2022 Existing	2024 No-Build	2024 Build	2029 Build
NYS Route 17M/Craigville Road	U				
NYS Route 17M EB      LT Craigville Road SB      LR		A (7.7) B (10.4)	A (7.8) B (10.9)	A (7.8) B (11)	A (7.8) B (11.1)
NYS Route 17 Northbound Exit 128 Off-Ramp/Craigville Road	U				
NYS Route 17 Northbound Exit 128 Off-Ramp EB      LR		A (8.6)	A (8.7)	A (8.9)	A (8.9)
Craigville Road/Site Driveway	U				
Site Driveway EB      L R Craigville Road NB      TL		-- --	-- --	A (9.1) A (8.5) A (7.3)	A (9.1) A (8.5) A (7.3)
NYS Route 17M/NYS Route 17 Southbound Exit 127 On-Ramp	U				
NYS Route 17M EB      LR		A (8.0)	A (8.1)	A (8.1)	A (8.1)
NYS Route 17M/Kings Hwy/Lehigh Ave <sup>11</sup>	S				
NYS Route 17M EB      T R NYS Route 17M WB      LTR Kings Highway NB      L TR Lehigh Avenue SB      LTR		C (33.1) C (31.4) C (33.1) D (40.2) D (28.6) D (37.6)	C (34.6) C (32.2) C (34.8) D (45.3) C (30.7) D (37.6)	D (34.8) C (32.3) D (35.0) D (45.8) C (30.9) D (37.5)	C (34.8) C (32.3) A (6.4) D (48.6) C (32.0) D (38.0)
NYS Route 17M/NYS Route 94/Academy Ave	S				
NYS Route 17M EB      L T TR NYS Route 17M WB      L TR NYS Route 94 NB      L T R Academy Ave SB      L TR		D (38.9) C (28.7) C (29.3) C (32.4) C (28.6) F (105.2) B (17.6) C (20.3) D (48.9) C (32.2)	D (40.6) C (30.3) C (31.2) C (34.5) C (29.6) F (133.5) B (18.6) C (22.4) D (50.2) D (36.0)	D (40.6) C (30.3) C (31.2) C (34.5) C (29.4) F (133.8) B (18.6) C (22.6) D (50.2) D (36.0)	D (41.3) C (31.4) C (32.3) D (35.6) C (29.9) F (152.1) B (19.1) C (33.3) D (50.6) D (37.5)
Overall		D (37.4)	D (41.6)	D (41.6)	D (45.0)
NYS Route 94/NYS Route 17 Northbound Exit 126 On/Off-Ramp	S				
NYS Route 94 EB      L T NYS Route 94 WB      T TR NYS Route 17 Northbound Exit 126 On/Off-Ramp      LT NB      R		A (6.5) A (5.0) B (10.3) B (11.2) B (15.9) B (17.6)	A (7.5) A (5.8) B (11.8) B (12.4) B (18.0) B (17.3)	A (7.5) A (5.8) B (11.7) B (12.4) B (18.1) B (17.3)	A (7.6) A (5.9) B (11.8) B (12.4) B (18.1) B (17.5)
Overall		A (9.5)	B (10.7)	B (10.7)	B (10.8)

<sup>10</sup> U = Unsignalized intersection | S = Signalized intersection | EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches | L, T, R = Left-turn, Through, and/or Right-turn movements | X (Y.Y) = Level of service (Average delay in seconds per vehicle)

<sup>11</sup> There is no overall LOS provided for this intersection as it was evaluated as two separate intersections due to its operation as a clustered intersection.

Table 4C cont. – Level of Service Summary<sup>12</sup>

Intersection	Control	Saturday Midday Peak Hour			
		2022 Existing	2024 No-Build	2024 Build	2029 Build
NYS Route 94/NYS Route 17 Southbound Exit 126 On/Off-Ramp	S	B (15.3)	B (16.0)	B (16.1)	B (16.2)
NYS Route 94 EB		B (13.9)	B (14.6)	B (14.6)	B (14.7)
NYS Route 94 WB		A (8.9)	A (9.7)	A (9.9)	B (10.0)
NYS Route 17 Southbound Exit 126 Off-Ramp SB		A (6.4)	A (7.3)	A (7.3)	A (7.4)
NYS Route 17 Southbound Exit 126 Off-Ramp SB		B (16.5)	B (18.0)	B (18.1)	B (18.3)
NYS Route 17 Southbound Exit 126 Off-Ramp SB		B (13.6)	B (15.4)	B (15.4)	B (15.6)
Overall		B (12.6)	B (13.4)	B (13.5)	B (13.6)
NYS Route 17M/Museum Village Road	U	A (7.7)	A (7.8)	A (7.8)	A (7.8)
NYS Route 17M, EB		A (7.7)	A (7.7)	A (7.7)	A (7.7)
NYS Route 17M, WB		B (12.0)	B (13.3)	B (13.5)	B (13.6)
Museum Village Road, NB		C (15.0)	C (18.7)	C (19.2)	C (19.8)
NYS Route 208/Museum Village Road	U	D (33.0)	--	--	--
Museum Village Road, EB		A (8.6)	--	--	--
NYS Route 208, NB	LT				
NYS Route 208/Museum Village Road	S	--	D (48.1)	D (48.1)	D (48.2)
Museum Village Road, EB		--	D (44.0)	D (43.9)	D (43.9)
NYS Route 208, NB		--	A (6.3)	A (6.3)	A (6.5)
NYS Route 208, SB		--	A (4.0)	A (4.0)	A (4.1)
NYS Route 208, SB		--	A (9.4)	A (9.4)	A (9.5)
NYS Route 208, SB		--	A (6.2)	A (6.3)	A (6.3)
Overall			B (13.2)	B (13.3)	B (13.3)
Craigville Road/ Old Mansion Road	U	A (8.9)	A (8.9)	A (9)	A (9)
Old Mansion Road, WB		A (7.4)	A (7.4)	A (7.4)	A (7.4)

The impact of the project can be described by comparing the analysis of the No-Build and Build operating conditions. The following observations are evident from this analysis:

- NYS Route 17M/Craigville Road:** The level of service analysis indicates that in the weekday AM peak hour Build condition, the approaches will operate at acceptable LOS C or better commensurate to the No-Build condition. In the weekday PM peak hour Build condition, the southbound Craigville Road approach will experience an increase in delay from the No-Build condition of nine seconds resulting in an LOS C, which is still considered an acceptable level of service. The NYS Route 17M approach will maintain a LOS of A.
- NYS Route 17 Northbound Exit 128 Off-Ramp/Craigville Road:** The level of service analysis indicates that the NYS Route 17 Exit 128 Off-Ramp approach will experience a two-to-three-second increase in delay between the No-Build and Build conditions. Regardless of the increase in delay, the approach will continue to operate at an acceptable LOS B.

<sup>12</sup>U = Unsignalized intersection | S = Signalized intersection | EB, WB, NB, SB = Eastbound, Westbound, Northbound, and Southbound intersection approaches | L, T, R = Left-turn, Through, and/or Right-turn movements | X (Y.Y) = Level of service (Average delay in seconds per vehicle)

- **Craigville Road/Site Driveway:** The level of service analysis indicates that Site Driveway will operate at a LOS B under stop control for the weekday AM and weekday PM peak hours, and LOS A for the Saturday midday peak hour. It is recommended that the Site Driveway operates with stop control enforced by a “STOP” (R1-1) sign and white stop bar.
- **NYS Route 17M/NYS Route 17 Southbound Exit 127 On-Ramp:** The level of service analysis indicates that the eastbound and westbound NYS Route 17M approaches will continue to operate at LOS A in the Build condition with less than a one second increase in delay for all study periods between the No-Build and Build conditions.
- **NYS Route 17M/Kings Highway/Lehigh Avenue:** The level of service analysis indicates that in the weekday AM peak hour Build condition when the majority of employees and trucks are en route to the site, the eastbound NYS Route 17M and northbound Kings Highway approaches experience an increase in delay compared to the No-Build condition. This effect is reversed in the weekday PM peak hour Build condition when the majority of employees and trucks are departing the site, thus the westbound NYS Route 17M approach experiences an increase of ten seconds in delay, and a change in LOS from LOS D to LOS E. There are no other changes in LOS. It should be noted that the proposed project will account for only three to four percent increase in total volume experienced at the intersection between the No-Build and Build conditions.
- **NYS Route 17M/NYS Route 94/Academy Avenue:** The level of service analysis indicates that in the Build condition the intersection will operate at levels of service commensurate to those in the No-Build condition. Notably, the northbound NYS Route 94 left-turn approach is experiencing and will continue to experience a LOS of F for the PM peak hour over the years. The proposed project does not add any traffic to this approach. Furthermore, an optimization of the signal timings would result in a decrease in delay. Any proposed signal timing changes are subject to review and approval by the NYSDOT.
- **NYS Route 94/NYS Route 17 Northbound Exit 126 On-/Off-Ramp:** The level of service analysis indicates that in the weekday AM, weekday PM, and Saturday midday peak hours, the approaches and the overall intersection will operate at an acceptable LOS C or better.
- **NYS Route 94/NYS Route 17 Southbound Exit 126 On-/Off-Ramp:** The level of service analysis indicates that in the weekday AM, weekday PM, and Saturday midday peak hours the approaches and the overall intersection will operate at an acceptable LOS C or better.
- **NYS Route 17M/Museum Village Road:** The level of service analysis indicates that in the AM and Saturday peak hours the southbound approach will operate at an LOS C in the No-Build condition and continue to do so in the Build condition. The PM peak hour will operate at an LOS F in the No-Build condition and will increase for the Build condition due to the additional eastbound traffic on NYS Route 17M.
- **Museum Village Road/NYS Route 208:** The Existing conditions analysis for this intersection considered its current unsignalized configuration. As noted in Tables 4A-4C, the intersection currently operates with capacity constraints. As part of the South Blooming Grove Business Park project, the intersection would be reconfigured to provide exclusive turn lanes and signalization. The No-Build and Build condition analyses consider these improvements. The level of service analyses of those conditions indicate that in the weekday AM, weekday PM, and Saturday midday peak hours the overall intersection will operate at an acceptable LOS C or better.
- **Craigville Road/Old Mansion Road:** The level of service analysis indicates that the stop-controlled approach will operate at an LOS B or better for all time periods.

#### Qualitative Assessment of Other Off-Site Intersections

In accordance with the requirements of the DEIS Scoping Document, CM conducted a qualitative assessment of the following off-site intersections:

- NYS Route 17/NYS Route 208 Interchange
- NYS Route 17M/NYS Route 208 Interchange
- NYS Route 208/N Main Street

As shown on the Project Peak Hour Site-Generated Trip Assignments Figure under Attachment F, the intersections listed above will experience a maximum of eight site-generated trips during any study peak hour. Of these site-generated trips, none will be truck trips. Based on ITE and NYSDOT guidance, a quantitative intersection analysis should be completed if 100 or more site-generated trips are entering the intersection on any one approach. Given the number of site-generated trips entering the aforementioned intersections, it is unnecessary to conduct a quantitative analysis.

#### Traffic Signal Warrant Analysis at Craigville Road/NYS Route 17M Intersection

An analysis was conducted to determine if the No-Build or Build traffic volumes warrant the installation of a traffic signal at the intersection of Craigville Road and NYS Route 17M. The traffic conditions and physical characteristics of the intersection were correlated to the signal warrant criteria contained in the 2009 National *Manual on Uniform Traffic Control Devices* (National MUTCD), published by the Federal Highway Administration (FHWA). The National MUTCD specifies the minimum criteria that must be met in order for a traffic signal to be justified. The satisfaction of a signal warrant in itself is not necessarily justification for the installation of a traffic signal. Other engineering and operational factors must be considered. The National MUTCD contains nine warrants, three of which were determined to be applicable and were evaluated in detail:

- Warrant 1 – Eight-Hour Vehicular Volume – This warrant is satisfied if for any eight hours of an average day, the traffic volumes for Condition A or Condition B specified in Table 4C-1 of the National MUTCD are met for the main arterial and the higher volume side road approach to the intersection.
- Warrant 2 – Four-Hour Vehicular Volume - This warrant is met when for any four hours of an average day, points plotted on the graph presented on Figure 4C-2 of the National MUTCD fall above the appropriate curve.
- Warrant 3 – Peak Hour - This warrant is met when for any one hour of an average day, points plotted on the graph presented on Figure 4C-4 of the National MUTCD fall above the appropriate curve.

Table 5 summarizes the warrant analyses based on the No Build traffic volumes at the intersection for Warrants 1, 2, and 3. Table 6 summarizes the warrant analyses based on the Build traffic volumes associated with the proposed development. A “Yes” under the “Signal Warrants Met?” column indicates that the criteria are satisfied for that hour. The detailed evaluation for both scenarios for Warrants 1, 2, and 3 is included under Attachment G.

**Table 5 – Traffic Signal Warrant Analysis of NYS Route 17M/Craigville Road (No-Build)**

Time Begin (1-hour period)	No-Build Volumes		Signal Warrants Met?			
	NYS Route 17M	Craigville Road	#1		#2	#3
			Cond. A	Cond. B		
6:00 AM	143	24	No	No	No	No
7:00 AM	337	42	No	No	No	No
8:00 AM	402	40	No	No	No	No
9:00 AM	378	43	No	No	No	No
10:00 AM	393	39	No	No	No	No
11:00 AM	400	45	No	No	No	No
12:00 PM	414	51	No	No	No	No
1:00 PM	442	54	No	No	No	No
2:00 PM	480	63	No	No	No	No
3:00 PM	513	72	No	No	No	No
4:00 PM	542	88	No	Yes	No	No
5:00 PM	530	88	No	Yes	No	No
6:00 PM	363	51	No	No	No	No
7:00 PM	239	36	No	No	No	No
8:00 PM	162	30	No	No	No	No
Required Volumes	One Lane Major Street		350	525	See Figure 4C-2	See Figure 4C-4
	One Lane Minor Street		105	53		
Overall Warrant Met?			No	No	No	No

**Table 6 – Traffic Signal Warrant Analysis of NYS Route 17M/Craigville Road (Build 2024)**

Time Begin (1-hour period)	Build Volumes		Signal Warrants Met?			
	NYS Route 17M	Craigville Road	#1		#2	#3
			Cond. A	Cond. B		
6:00 AM	168	36	No	No	No	No
7:00 AM	363	65	No	No	No	No
8:00 AM	429	63	No	No	No	No
9:00 AM	414	68	No	No	No	No
10:00 AM	422	71	No	No	No	No
11:00 AM	437	81	No	No	No	No
12:00 PM	454	78	No	No	No	No
1:00 PM	472	80	No	No	No	No
2:00 PM	512	87	No	No	No	No
3:00 PM	558	111	Yes	Yes	Yes	No
4:00 PM	579	120	Yes	Yes	Yes	No
5:00 PM	562	117	Yes	Yes	Yes	No
6:00 PM	381	67	No	No	No	No
7:00 PM	246	41	No	No	No	No
8:00 PM	168	34	No	No	No	No
Required Volumes	One Lane Major Street		350	525	See Figure 4C-2	See Figure 4C-4
	One Lane Minor Street		105	53		
Overall Warrant Met?			No	No	No	No

Table 5 indicates that Warrants 1, 2, and 3 are not met at the intersection for the No-build conditions. Table 6 indicates that Warrant 1 is not met under Condition A in the Build conditions as well as both Warrants 2 and 3. The level of service analysis indicates that the Craigville Road approach operates at an acceptable LOS B or better in the No-Build and Build conditions as an unsignalized intersection.

#### 4.0 Sight Distance Analysis

The available intersection sight distance and stopping sight distance was measured at the following intersections:

- Craigville Road/Site Driveway
- Craigville Road/NYS Route 17 Northbound Exit 128 Off Ramp
- Craigville Road/NYS Route 17M

The measurements conducted at these intersections are depicted in the Exhibit 9.

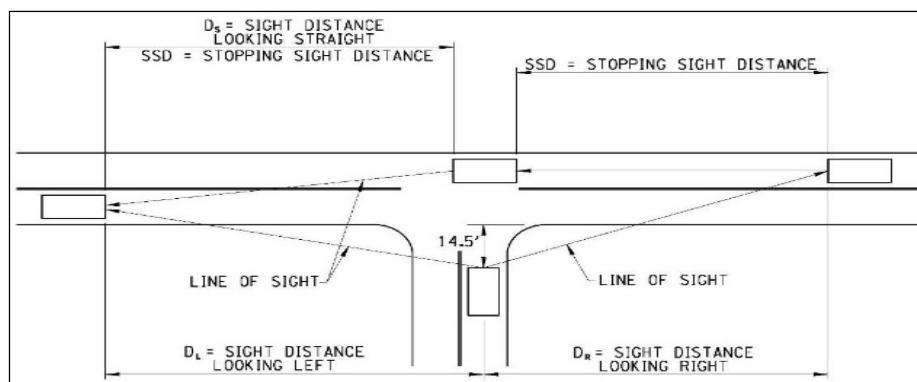


Exhibit 9 – Generic Intersection and Stopping Sight Distance Measurements

#### Site Driveway/Craigville Road

On September 18, 2020, CM conducted a sight distance analysis based on a preliminary driveway location for the proposed project on the north side of Craigville Road (CR 51) based on the Site Plan prepared by Arden Consulting Engineers. Along the subject site's frontage, Craigville Road does not have a posted speed limit; therefore, the statutory speed limit is 55 miles per hour. The analysis indicated that the preliminary driveway location would not provide sufficient intersection sight distance for vehicles turning from (exiting) the driveway onto Craigville Road. Specifically, the sight distance looking left (north) from the site driveway was limited by the Orange-Heritage Trail overpass. Guidelines found in AASHTO's *A Policy on Geometric Design of Highways and Streets*, 2018 and NYSDOT design guidance (EB 17-007) for a passenger vehicle and the posted speed limit recommend 610 feet of sight distance for an operating speed of 55 mph. CM coordinated a field visit with representatives of the OCDPW which has jurisdiction over Craigville Road, on October 6, 2020. The OCDPW representatives confirmed that intersection sight distance must be met for the site driveway and provided the following criteria:

- Driver's eye height = 3.5 feet
- Operating speed is equal to the posted speed limit. Because Craigville Rd (CR 51) does not have a posted speed limit, the statutory speed limit is 55 mph.

Based on these discussions and the field measurements from the September 18, 2020 field visit, the driveway was relocated to provide at least 610 feet of sight distance looking left and right from the driveway, satisfying the guidelines for AASHTO and NYSDOT for a speed of 55 mph. The available sight distances at the intersection of Craigville Road and the Site Driveway are shown in Table 8 and compared to the guidelines presented in AASHTO's *A Policy on Geometric Design of Highways and Streets*, 2018 and NYSDOT design guidance (EB 17-007).

**Table 8 – Sight Distance Summary (Feet) for Site Driveway/Craigville Road**

Intersection		Intersection Sight Distance <sup>1</sup>				Stopping Sight Distance <sup>2</sup>	
		Right Turn from Site Driveway (D <sub>L</sub> )	Left Turn from Site Driveway		Left Turn from Craigville Road (D <sub>S</sub> )	SSD <sub>NB</sub>	SSD <sub>SB</sub>
			Looking Left (D <sub>L</sub> )	Looking Right (D <sub>R</sub> )			
Site Driveway/Craigville Road	Available	610	610	+1100	525	+1100	500
	Recommended <sup>3</sup>	530	610	610	445	455	455

<sup>1</sup> Intersection sight distance is measured at an eye height of 3.5-ft and object height of 3.5-ft.<sup>2</sup> SSD<sub>NB, SB</sub> = Stopping sight distance measured for a 2-foot object located in the path of vehicles traveling northbound and southbound on Craigville Road.<sup>3</sup> Speed Limit = 55-mph

The analysis indicates that the available intersection and stopping sight distances at the Site Driveway on Craigville Road will meet or exceed the AASHTO and NYSDOT guidelines for 55-mph. It is recommended that vegetation along the subject site's frontage with Craigville Road be cleared. Furthermore, the OCDPW has provided a letter approving the site driveway location, which is included under Attachment H.

#### NYS Route 17 Northbound Exit 128 Off Ramp/Craigville Road

Per the DEIS Scoping Document, a sight distance evaluation was conducted at the NYS Route 17 Northbound Exit 128 Off Ramp/Craigville Road intersection. The available sight distances at the intersection of are shown in Table 9 and compared to the guidelines presented in AASHTO's *A Policy on Geometric Design of Highways and Streets*, 2018 and NYSDOT design guidance (EB 17-007).

**Table 9 – Sight Distance Summary (Feet) for NYS Route 17 NB Exit Off-Ramp/Craigville Road**

Intersection		Intersection Sight Distance <sup>1</sup>				Stopping Sight Distance <sup>2</sup>	
		Right Turn from Off-Ramp (D <sub>L</sub> )	Left Turn from Off-Ramp		Left Turn from Craigville Rd (D <sub>S</sub> )	SSD <sub>NB</sub>	SSD <sub>SB</sub>
			Looking Left (D <sub>L</sub> )	Looking Right (D <sub>R</sub> )			
NYS Route 17 NB Exit 128 Off-Ramp/Craigville Road	Available	610	610	550	N/A	N/A	N/A
	Recommended <sup>3</sup>	530	610	610	445	455	455

<sup>1</sup> Intersection sight distance is measured at an eye height of 3.5-ft and object height of 3.5-ft.<sup>2</sup> SSD<sub>NB, SB</sub> = Stopping sight distance measured for a 2-foot object located in the path of vehicles traveling northbound and southbound on Craigville Road.<sup>3</sup> Speed Limit = 55-mph

The analysis indicates that the available intersection sight distance looking left (north) meets the AASHTO and NYSDOT guidelines for the posted speed limit, 55-mph. However, the available intersection sight distance looking right (south) falls short of the recommended guideline by 60-ft. This is due to the crest vertical curve on Craigville road south of the intersection. It should be noted that the intersection is not critically limited as the available intersection sight distance looking right (south) exceeds the recommended stopping sight distance.

#### Craigville Road/NYS Route 17M

Per the DEIS Scoping Document, a sight distance evaluation was conducted at the Craigville Road/NYS Route 17M intersection. The available sight distances at the intersection of Craigville Road and NYS Route 17M are shown in Table 10 and compared to the guidelines presented in AASHTO's *A Policy on Geometric Design of Highways and Streets*, 2018 and NYSDOT design guidance (EB 17-007).

**Table 10 – Sight Distance Summary (Feet) for Craigville Road/NYS Route 17M**

Intersection	Intersection Sight Distance <sup>1</sup>				Stopping Sight Distance <sup>2</sup>	
	Right Turn from Craigville Rd (D <sub>L</sub> )	Left Turn from Craigville Rd		Left Turn from NYS Route 17M (D <sub>S</sub> )	SSD <sub>EB</sub>	SSD <sub>WB</sub>
		Looking Left (D <sub>L</sub> )	Looking Right (D <sub>R</sub> )			
Craigville Road/NYS Route 17M	Available	610	610	675	610	565
	Recommended <sup>3</sup>	530	610	610	445	455

<sup>1</sup> Intersection sight distance is measured at an eye height of 3.5-ft and object height of 3.5-ft.

<sup>2</sup> SSD<sub>EB</sub>, <sub>WB</sub> = Stopping sight distance measured for a 2-foot object located in the path of vehicles traveling eastbound and westbound on NYS Route 17M.

<sup>3</sup> Speed Limit = 55-mph

The analysis indicates that the available intersection and stopping sight distances at the Craigville Road/NYS Route 17M intersection meet or exceed the AASHTO and NYSDOT guidelines for the posted speed limit, 55-mph.

## 5.0 Site Access, Circulation, Maintenance, and Parking

CM reviewed the site access, circulation and parking layout as shown on the Site Plan prepared by Arden Consulting Engineers, PLLC. As discussed, access to the subject site will be provided via one full-movement driveway on Craigville Road. The access driveway will provide one ingress lane and two egress lanes for exclusive left-turn and right-turn lanes. The ingress and egress lanes will be 13-foot-wide near Craigville Road and then widen to 15-foot-wide travel lanes. The width of the proposed driveway allows for vehicles (passenger vehicles, trucks, and fire trucks) to temporarily by-pass vehicles that may become inoperable within a travel lane along the driveway length. At an internal intersection, passenger vehicles are directed to the south side of the building where there are 349 parking spaces; tractor trailers are directed to the north side of the building where there are 88 loading docks and 60 trailer parking spaces provided.

Within the passenger vehicle parking area, there is a 24-foot-wide drive aisle provided for two-way circulation. Within the tractor trailer loading dock and trailer parking area, there is a 75-foot-wide drive aisle provided for vehicles to maneuver in and out of the loading docks and trailer parking spaces. On-site truck turning diagrams for a WB-67 and Fire Truck are depicted on Sheets 50-53 of the Site Plan Prepared by Arden Consulting Engineers, PLLC. Road and driveway maintenance will fall under the purview of the owner/operators of the development, who will contract with private maintenance services for the handling of snow removal and de-icing of roadways and pedestrian walkways. As shown on Sheets 12-17 of the Site plan prepared by Arden Consulting Engineers, PLLC, the snow storage associated with these operations will be located on the westerly and southerly portions of the subject site.

Based on the current code, the proposed project, which is comprised of 515,888 square feet of warehouse space and 46,562 square feet of supporting office space, is required to provide 1,798 parking spaces. CM notes that the project is proposing a text change to the Town of Blooming Grove Zoning Code Section 235-23.C – “Off-Street Parking Requirements” specific to the “Warehouse; Distribution” land use. To justify the proposed text change, several resources were studied in order to develop a recommended parking requirement and an alternative to the current Town of Blooming Grove Zoning Code. These resources included three other warehouse developments in Orange County, the parking requirements of a nearby municipality, and the Institute of Transportation Engineers’ (ITE) *Parking Generation Manual*, 5<sup>th</sup> Edition.

## 5.1 Parking Supply for Other Warehouse Developments

In the Town of Montgomery, three similar developments have proposed parking supply rates ranging from 1 space per 2,111 square feet (SF) and 1 space per 787SF. For context, the Town of Montgomery Zoning Code 235-12.4 for "Truck Terminal, wholesaler, warehouse, or distribution center" states that there should be "sufficient parking for all trucks, truck trailers and truck tractors stored or being serviced at any one period of time, plus required parking for office area, plus two (2) spaces per three (3) employees on duty or on the premises at any one time."

The following are the three developments examined as part of this discussion:

- The *Galaxy Limited* development consists of a 240,000-square-foot warehouse inclusive of a 12,000-square-foot office space and anticipates 162 employees. In adherence with the code, the site proposes 60 spaces for the office use (one (1) space per 200 square feet) and 108 parking spaces for the warehouse use which equates to 0.47 spaces/1,000SF or 1 space/2,111SF.
- The *915 Route 17K, LLC* development includes 133,690 square feet of leasable floor area for a warehouse use. In adherence with the code at the time of review, the site proposes 110 parking spaces for the warehouse use, which equates to 0.75 spaces/1,000SF or 1 space/1,215SF.<sup>13</sup>
- The *Project Sailfish* development includes a 1,010,866 square feet of leasable floor area for a warehouse use and anticipates 629 employees during its peak shift. While the site would meet the parking requirements with 420 parking spaces, the site proposes 1,060 parking spaces, which equates to 1 space/954SF or 1.05 spaces/1,000SF. The site also proposes 225 trailer parking spaces; if these are included in the total number of spaces the rates are 1.27 spaces/1,000SF or 1 space/787SF.

Applying the parking rates of any of these Montgomery-based developments to the proposed warehouse project in Blooming Grove would yield a lower parking requirement as summarized in Table 11.

**Table 11 – Summary of Parking Requirements based on Other Developments' Rates**

	<i>Galaxy Limited 1 space/2,111SF</i>	<i>915 Route 17K, LLC 1 space/1,215SF</i>	<i>Project Sailfish 1 space/787SF</i>
515,888 SF * Other Development Rate	245	425	656
Proposed Office Use per 1 space/600SF <sup>1</sup>	78	78	78
<b>Total</b>	<b>323</b>	<b>503</b>	<b>734</b>

<sup>1</sup>The Town of Blooming Grove zoning code requirement for the accessory office use would not be changed.

As Table 11 shows, based on the Project Sailfish parking rates, which are the most demanding, the subject development would require 1,064 fewer parking spaces than the current Blooming Grove Town Code requirement.

## 5.2 Parking Requirements of Other Municipalities

The Town of Wallkill is a nearby municipality whose code requires less parking for warehouse uses than the Town of Blooming Grove code. The Town of Wallkill Zoning Code 249-12.F – Off-Street Parking for a Warehouse requires "one (1) space per 1,500SF of gross floor area for the first 10,000SF, plus one (1) per each additional 5,000SF, plus one (1) per 300 SF of office area." Applying the parking requirement of the Town of Wallkill to the proposed warehouse project in Blooming Grove would also yield a lower parking requirement. Table 12 summarizes this lower parking requirement.

<sup>13</sup> At the time of submittal to Town of Montgomery Planning Board, the Zoning Code 100-30 referred to the ITE Parking Generation 4<sup>th</sup> Edition for Peak Parking Demand Rates.

**Table 12 – Summary of Parking Requirements based on Other Municipality's Rates**

		<i>Town of Wallkill 1 space/1,500SF + 1 space/Add. 5,000SF</i>
515,888 SF * Other Development Rate		108
Proposed Office Use per 1 space/300SF <sup>1</sup>		78
<b>Total</b>		<b>186</b>

<sup>1</sup>The Town of Blooming Grove zoning code requirement for an office use would not be changed.

As Table 12 shows, the same development in the Town of Wallkill would require 1,612 fewer parking spaces.

### 5.3 Parking Demand per the Institute of Transportation Engineers

The Institute of Transportation Engineers' (ITE) *Parking Generation Manual*, 5<sup>th</sup> Edition, is the industry standard used for estimating parking demands for proposed land uses based on data collected at similar uses. The manual presents information for 121 land uses, including warehouses, allowing engineers and analysts to forecast the parking demand for a specific development site. Based on a study of 31 warehouse sites, the ITE calculated an average rate and a 95-percent confidence interval of rates for peak parking demand. These rates are applied to the proposed warehouse and the results are summarized in Table 13.

**Table 13 – Summary of Peak Parking Demand Based on ITE**

	<i>Average Rate 0.39 spaces/1,000SF or 1 space/2,564SF</i>	<i>95% Confidence Interval Rate 0.47 spaces/1,000SF or 1 space/2,128SF<sup>1</sup></i>
Warehouse – 518,888 SF	202	243
Proposed Office Use 1 space/600SF <sup>2</sup>	78	78
<b>Total</b>	<b>280</b>	<b>321</b>

<sup>1</sup>The range of rates in the 95% Confidence interval were 1 space/3,226SF to 1 space/2,128SF. The higher rate is used to be conservative.

<sup>2</sup>The Town of Blooming Grove zoning code requirement for an office use would not be changed.

As Table 13 shows, with the addition of the proposed office use parking requirements based on Town of Blooming Grove zoning code, the ITE average peak parking demand is 1,598 parking spaces fewer than what is required by the Town of Blooming Grove Zoning Code. Conservatively, when this calculation is based on the highest rate in the 95-percent confidence interval, the peak parking demand is still 1,477 parking spaces less than the current requirement.

A review of the studied material indicates that the Town of Blooming Grove Zoning Code parking requirements for a warehouse use are excessive and would result in approximately **1,500 unused** parking spaces when considering the industry-standard ITE data. Constructing surplus impervious surfaces (i.e., parking spaces) to this extent is environmentally concerning and wasteful. CM recommends that the parking requirements for warehouses, specifically the Craigville Road Logistics Warehouse, be determined based on a rate of 1 space per 2,000SF or 0.5 spaces per 1,000SF. This rate is more demanding than both ITE peak parking demand rates and is similar to the Town of Wallkill progressive rate methodology. Based on this proposed rate, the subject development would require 258 parking spaces for the warehouse use and 78 parking spaces for the supporting office space, which is based on the current Town requirement. As the site plan currently proposes 349 vehicle parking spaces inclusive of 14 ADA-accessible spaces, 88 loading docks, and 60 trailer parking spaces, there would still be a reasonable reserve of off-street parking spaces to address unforeseen/unusual periods of demand. Still, the proposed 349 parking spaces is 1,462 spaces fewer than the current Blooming Grove Zoning Code requirement, allowing for significantly more pervious/landscaped area on the subject site.

## 6.0 Mitigation

### Motor Vehicle Collisions

As discussed in **Section 2.0 Existing Conditions – Motor Vehicle Collision Analysis**, the following intersections exhibited intersection collision rates above the statewide average for similar type intersections:

- NYS Route 17M/Craigville Road
- NYS Route 17M and NYS Route 94/Academy Avenue
- NYS Route 17M/Museum Village Road
- NYS Route 208/Museum Village Road

In regards to the NYS Route 208/Museum Village Road intersections, a signal is proposed as part of the South Blooming Grove Business Park development. Based on the industry-standard MUTCD guidelines a new signal is not warranted at the intersection of NYS Route 17M/Craigville Road. Consideration by NYSDOT, which has jurisdiction over the intersection, can still be given to a traffic signal at this intersection as a means to address the pre-existing conditions of pedestrian connectivity to Goosepond Park and the intersection crash rate. According to the NYSDOT Post Implementation Evaluation System (PIES) Reduction Factor Report, installation of a new red/yellow/green signal has a collision reduction factor of 32% for all accident types.

In regards the NYS Route 17M/Museum Village Road intersection, CM notes that the Museum Village Road approaches intersect NYS Route 17M at a skewed angle. Furthermore, the horizontal curvature of NYS Route 17M presents difficult sight lines for drivers on Museum Village Road. Given the history of collisions at the intersection and their jurisdiction of the roadway, CM recommends that the NYSDOT investigate mitigation measures to reduce collisions at this intersection.

### Truck Movements at Off-Site Intersections

As previously discussed, CM prepared truck turning diagrams at intersections where trucks associated with the proposed development are expected to enter. The diagrams indicate that the existing configurations for the majority of intersections allow a WB-67 to conduct the anticipated movements. However, based on the diagram shown in Figure 11A, the shoulder on the northeast corner of the intersection of Craigville Road and NYS Route 17M may need to be widened and reinforced. A five-foot widening is shown on NYS Route 17M, which is under the jurisdiction of the NYSDOT, in Figure 11A-1 and is recommended for consideration by the Department.

### Goosepond Parking & Trailhead

The Goosepond Parking and Trailhead are located directly south of the Craigville Rd/NYS Route 17M intersection on the east and west sides of NYS Route 17M, respectively. There are currently no pedestrian safety countermeasures connecting the approximately 6,500-square-foot parking area to the trail over NYS Route 17M. As previously noted, the roadway is under the jurisdiction of the NYSDOT, which means that any improvements will be subject to review and approval by the Department. A review of the New York State Pedestrian Safety Action Plan (NYSPSAP) indicates for a roadway with a posted speed limit above 50-mph, implementation of measures to reduce operational speeds should be considered.<sup>14</sup> Such measures typically consist of geometric modifications to the roadway. Roadway redesign of the NYS Route 17M is beyond a reasonable scope for a project of this nature. However, Appendix C of the NYSPSAP indicates that Basic Treatment Package C is applicable if speed reduction is not feasible. Therefore, CM recommends that W11-15A “Trail Crossing” warning sign with supplemental W16-2 “600 FT” plaques be considered for implementation by the NYSDOT on the approaching lane roadside approximately 600-ft in each direction from the trailhead. Additionally, CM recommends that a W11-2 “Pedestrian Crossing” warning signs with supplemental W11-15P “Trail X-ING” plaques be considered for implementation by the NYSDOT on either side of NYS Route 17M at the trailhead.

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<sup>14</sup> NYSPSAP Appendix A Table 4

## 7.0 Construction Traffic

At the time of this report, the proposed development is expecting 195,081 cubic yards (CY) of cut. Assuming the standard dump truck permitted on public roadways has an average capacity of 17 CY and the target duration of the cut operation is three months (75 working days assuming six days/week), the project would generate 153 truckloads per day, which equates to 306 truck trips per day (153 entering | 153 exiting). Assuming an eight-hour workday, the site would generate 38-40 total truck trips per hour. This hourly volume of traffic is approximately 50-percent lower than the weekday AM and weekday PM peak hour traffic volumes generated by the proposed development, but is approximately 40-percent higher than the Saturday peak hour traffic volumes generated by the proposed development. As previously stated, it is the intention of the project to contain the operations resulting in these truck trips to a three-month period.

## 8.0 Conclusion

The subject site is comprised of two parcels designated as Section 54, Block 1, Lot 50.1 and Section 52, Block 5, Lot 11 as shown on the Orange County Tax Map and is currently utilized for agriculture. The proposed commercial development consists of constructing a new one-story 562,450-square-foot building located on the west side of Craigville Road. The site will be accessed via one full-movement driveway on Craigville Road approximately 610 feet south of the Orange Heritage Trail Overpass. The following is noted regarding the proposed project:

- The proposed development will generate 91 total trips in the weekday AM peak hour, 97 total trips in the weekday PM peak hour, and 28 total trips in the Saturday midday peak hour. It is important to note that there is no “pass-by” component of the traffic associated with the proposed development.
- The study herein considered traffic volumes generated by other pending developments in the Town of Chester, Town of Monroe, and Town of Blooming Grove.
- The proposed development is not expected to have a significant adverse impact at the majority of off-site intersections. Where impacts are anticipated, mitigation is recommended as detailed in **Section 6.0 Mitigation**.
- A signal warrant analysis of the Craigville Road/NYS Route 17M intersection indicates that a signal is not warranted.
- Sight distance analyses of the intersections requested in the DEIS Scoping Document found that no intersections are critically limited.
- A review of parking provisions at other warehouse developments and parking requirements for other municipalities, indicates that the parking requirements for the proposed development are excessive and would result in unused pavement. The research conducted justifies a text change to the Town of Blooming Grove Zoning Code 235-23.C – “Off-Street Parking Requirements” specific to the “Warehouse; Distribution” land use to bring the parking requirement more in line with the actual parking need.

Please do not hesitate to call our office if you have any questions or comments, or require additional information.

Respectfully submitted,

Creighton Manning Engineering, LLP



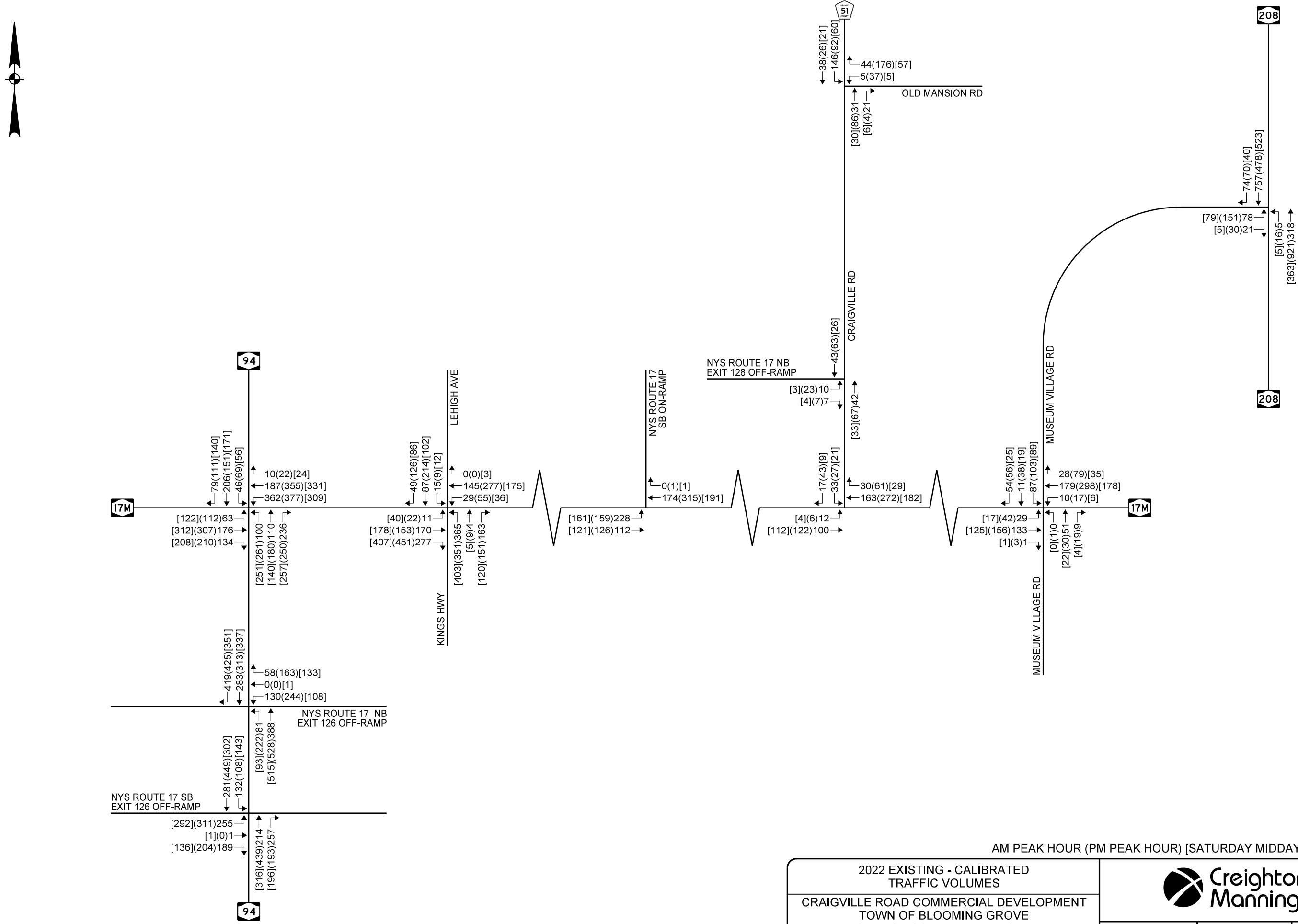
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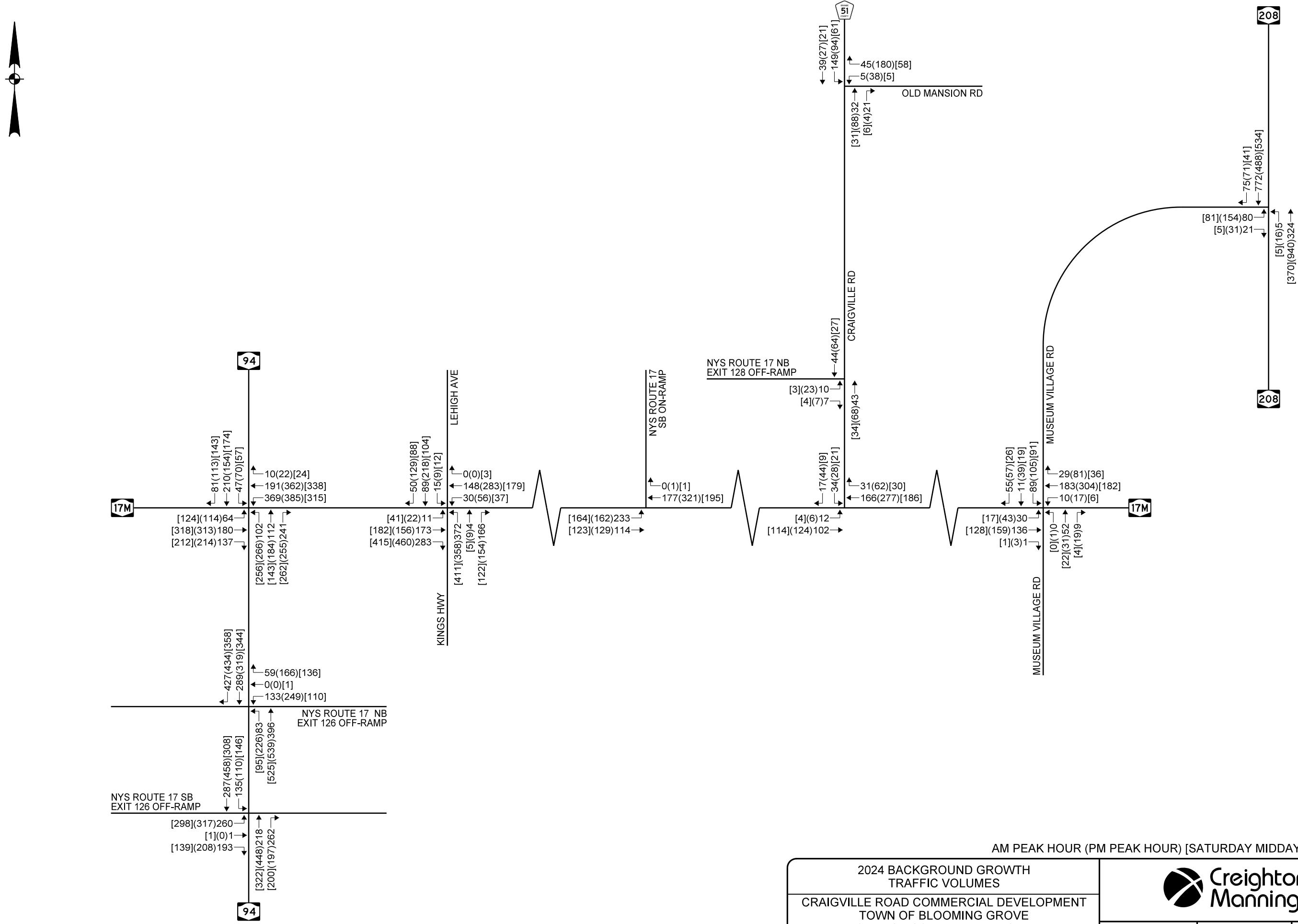
Associate

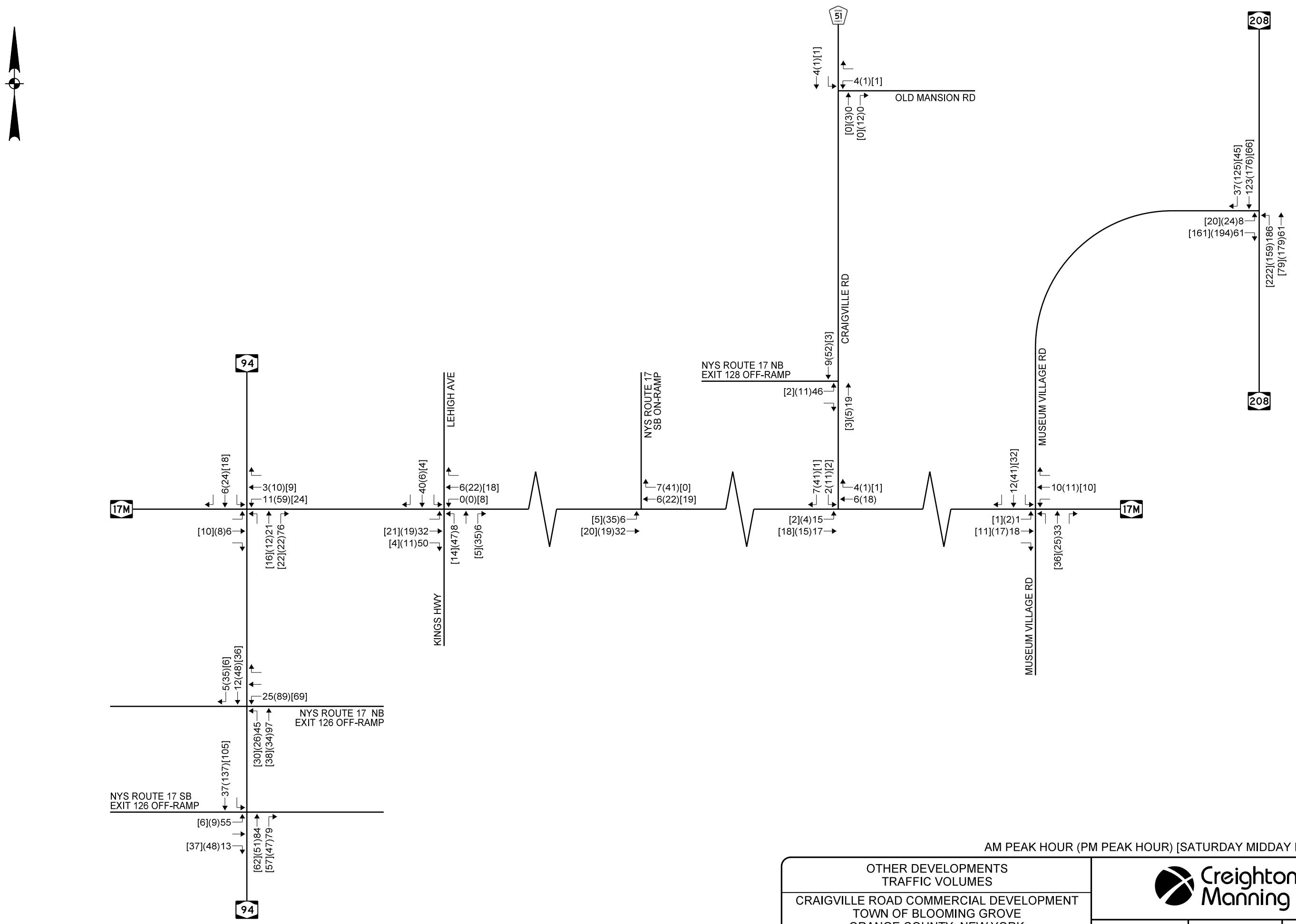


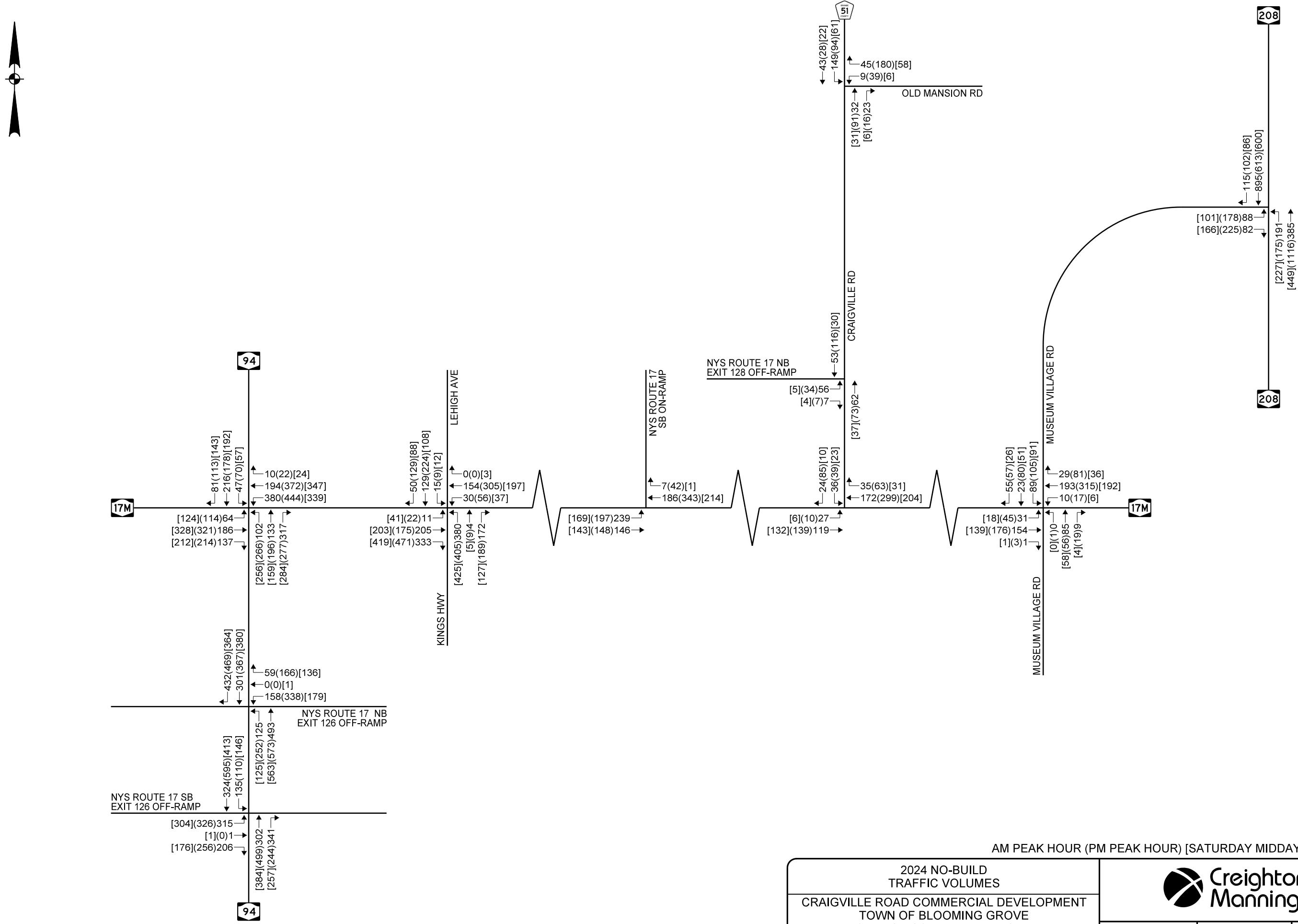
Starke W. Hipp, PE

Project Engineer





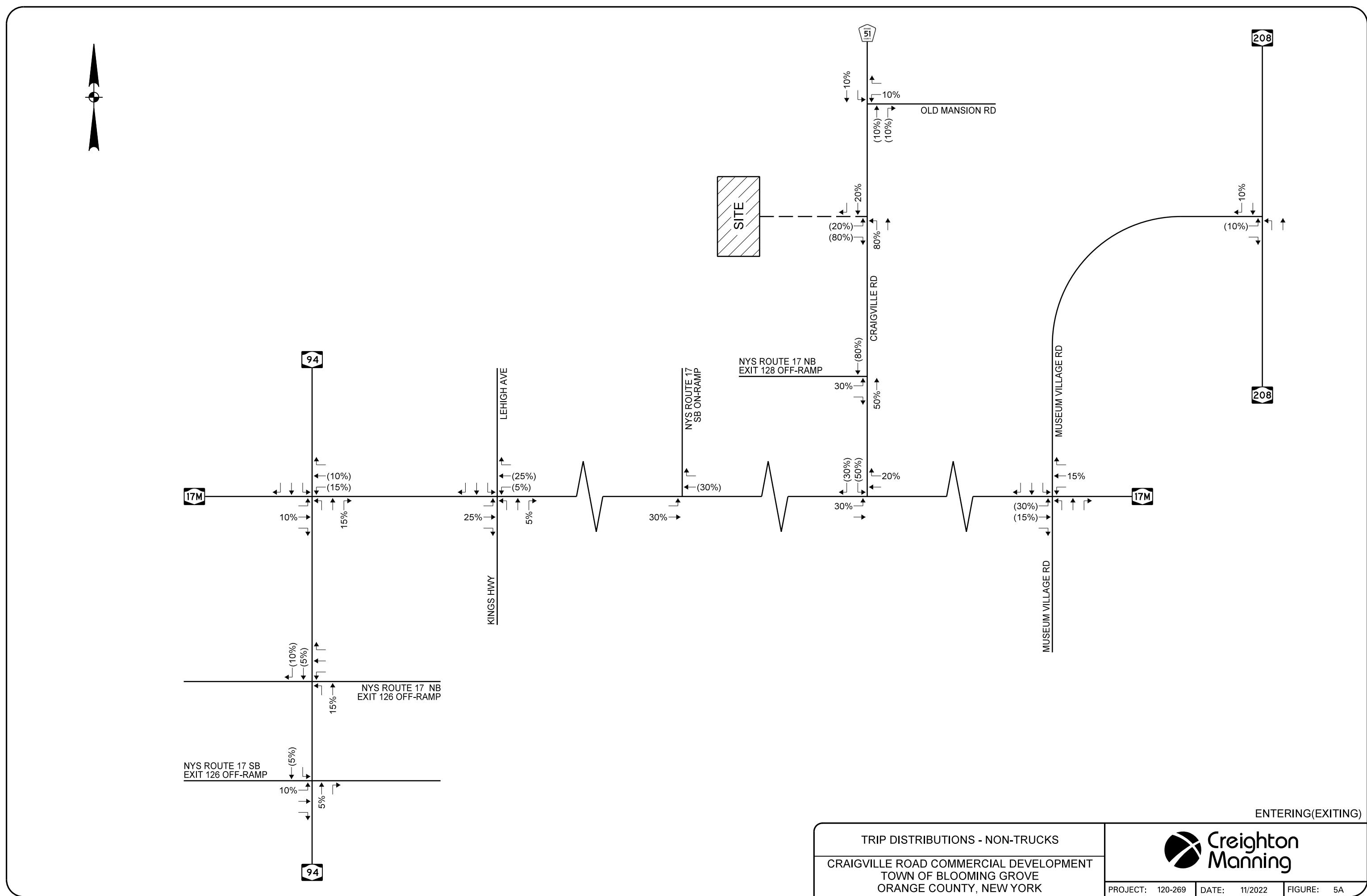


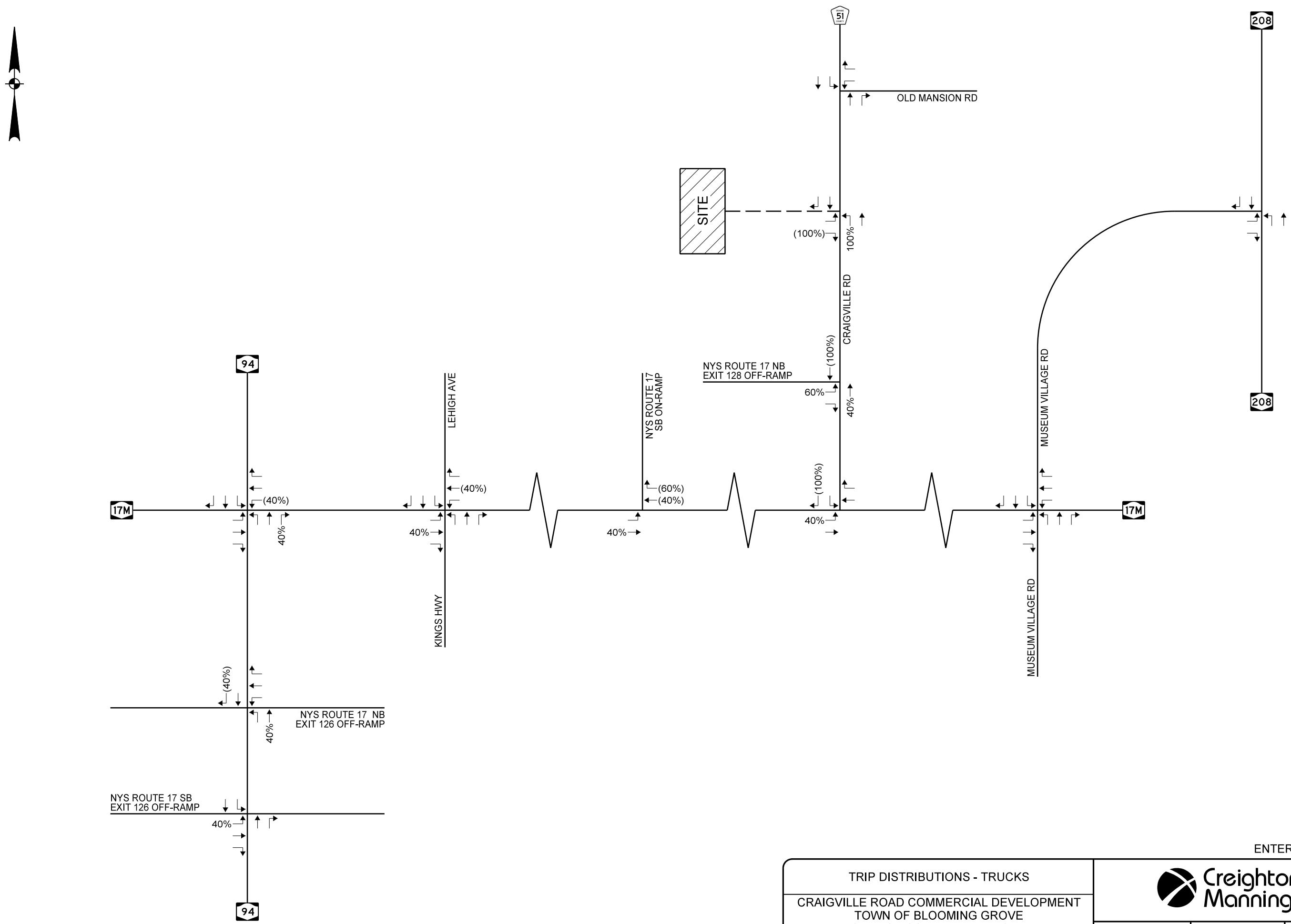


AM PEAK HOUR (PM PEAK HOUR) [SATURDAY MIDDAY PEAK HOUR]

2024 NO-BUILD TRAFFIC VOLUMES
CRAIGVILLE ROAD COMMERCIAL DEVELOPMENT TOWN OF BLOOMING GROVE ORANGE COUNTY, NEW YORK





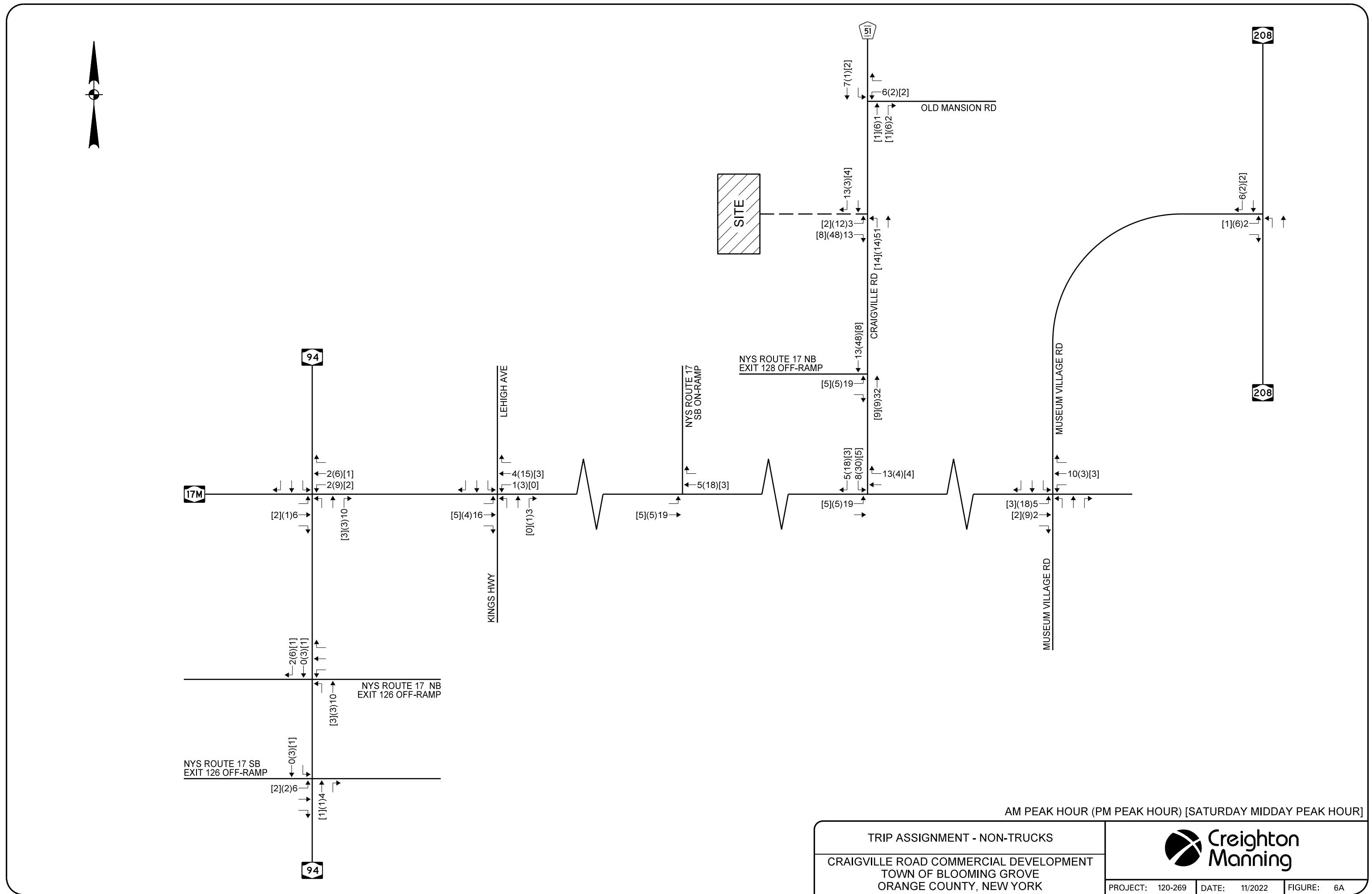


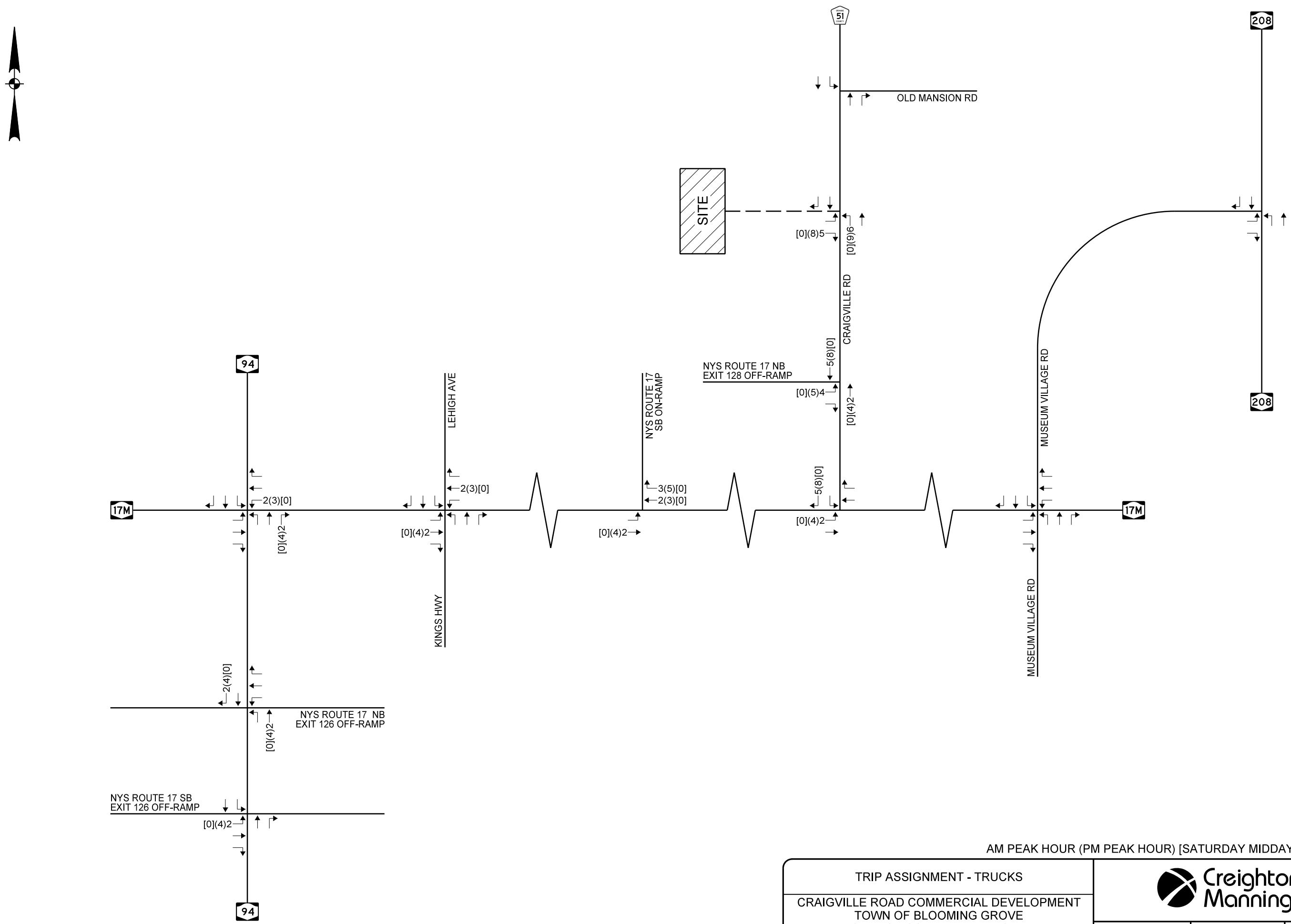
ENTERING(EXITING)

TRIP DISTRIBUTIONS - TRUCKS  
 CRAIGVILLE ROAD COMMERCIAL DEVELOPMENT  
 TOWN OF BLOOMING GROVE  
 ORANGE COUNTY, NEW YORK



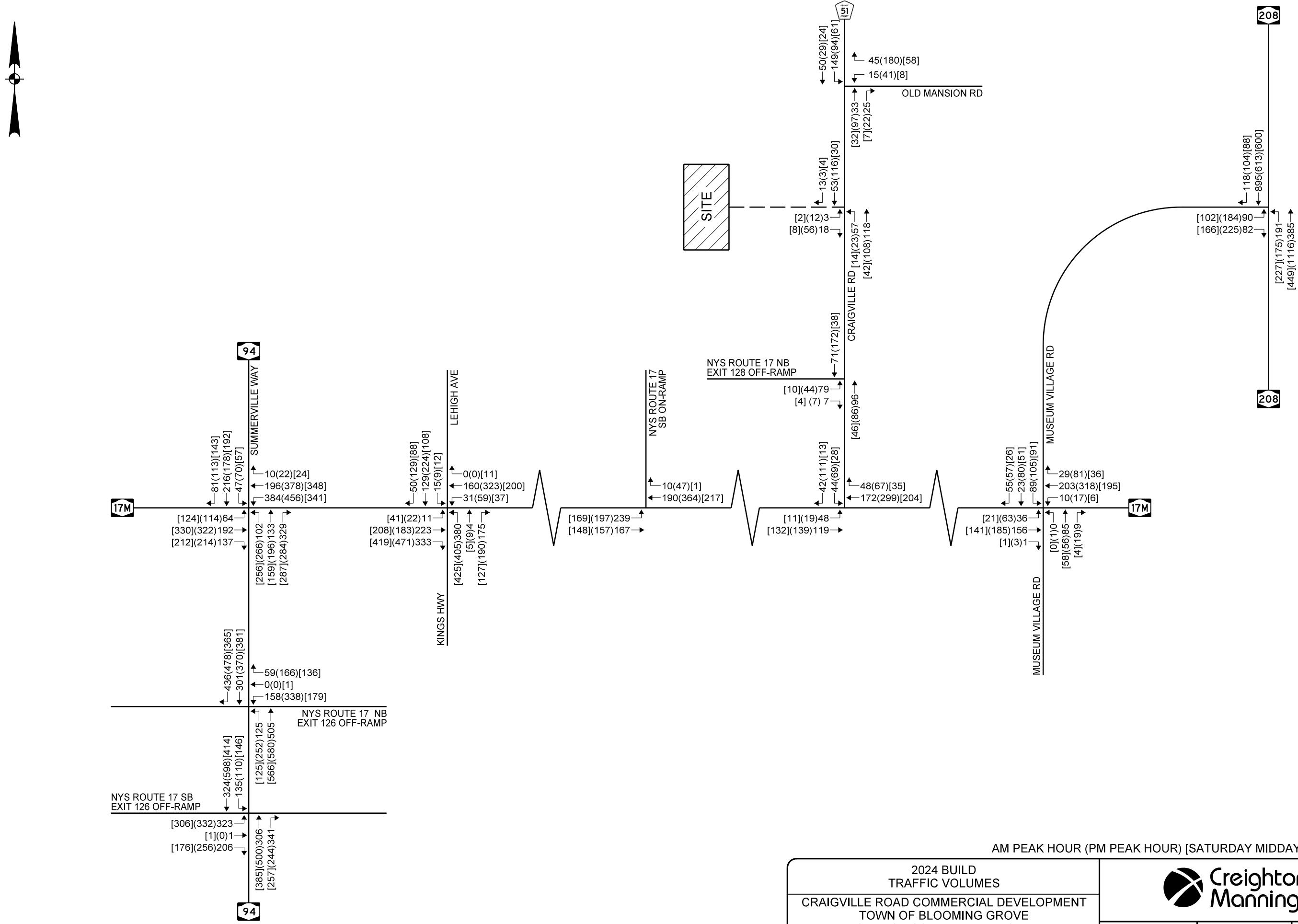
PROJECT: 120-269 DATE: 11/2022 FIGURE: 5B

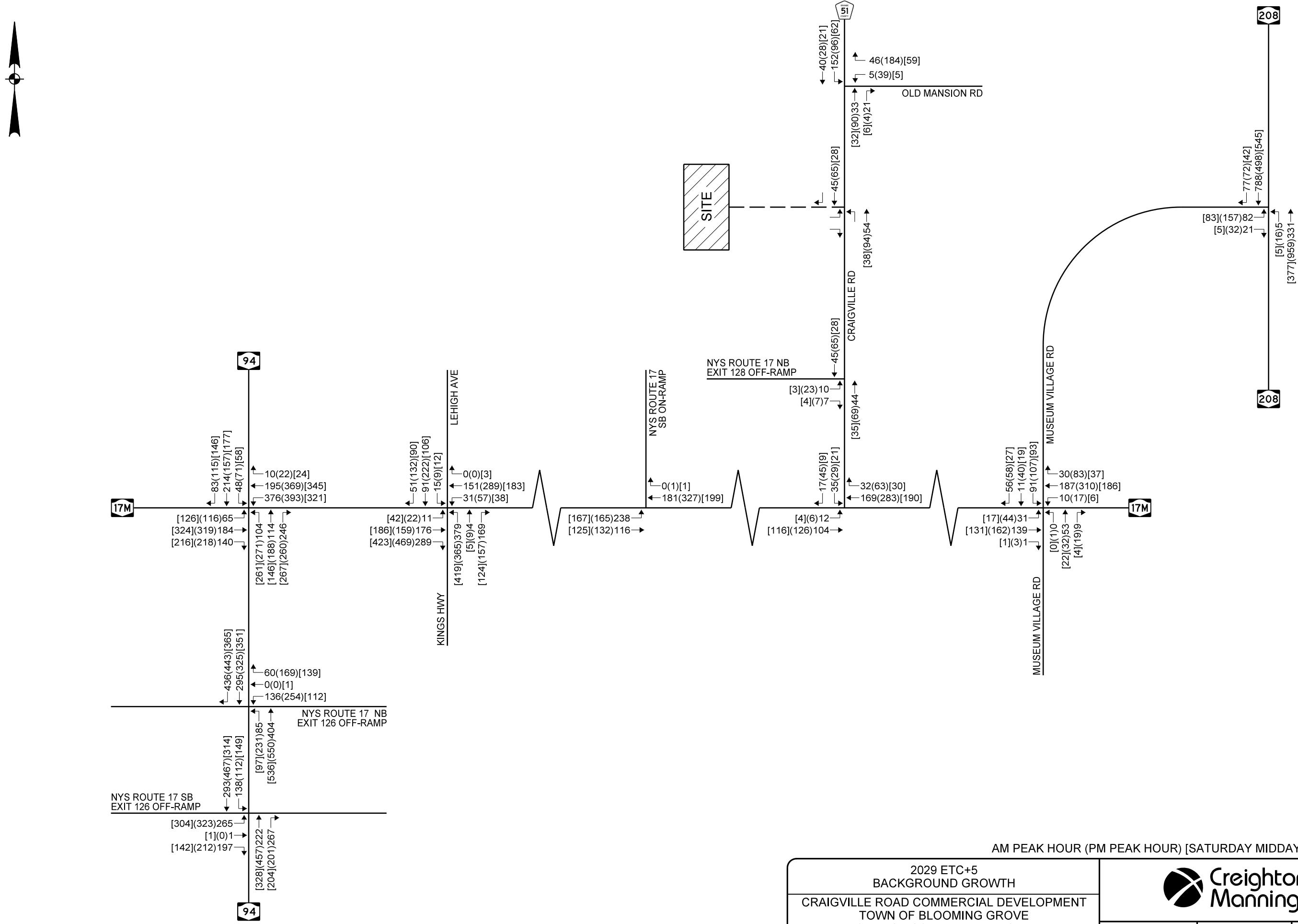




AM PEAK HOUR (PM PEAK HOUR) [SATURDAY MIDDAY PEAK HOUR]

TRIP ASSIGNMENT - TRUCKS	
CRAIGVILLE ROAD COMMERCIAL DEVELOPMENT TOWN OF BLOOMING GROVE ORANGE COUNTY, NEW YORK	PROJECT: 120-269 DATE: 11/2022 FIGURE: 6B



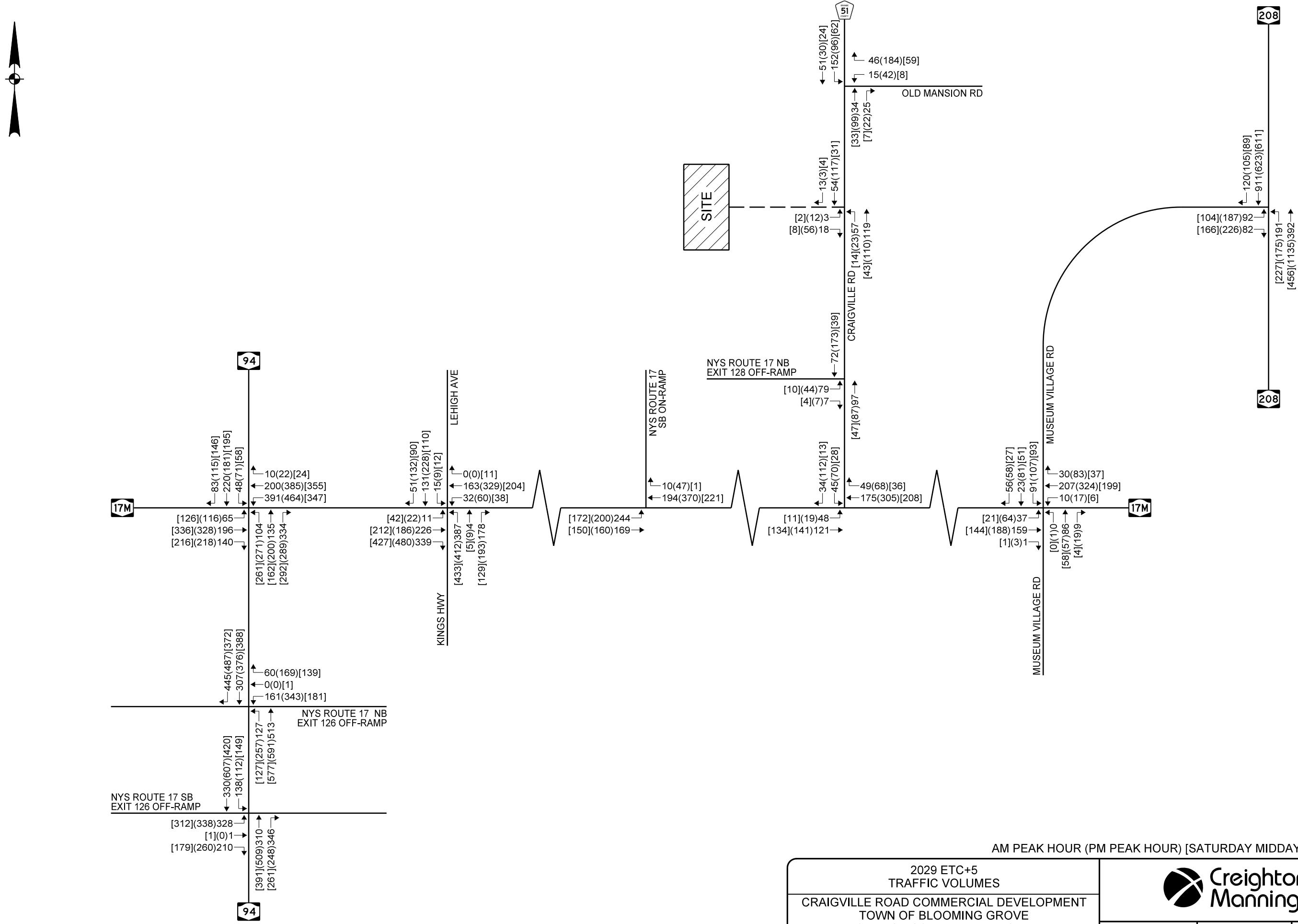


AM PEAK HOUR (PM PEAK HOUR) [SATURDAY MIDDAY PEAK HOUR]

2029 ETC+5 BACKGROUND GROWTH
CRAIGVILLE ROAD COMMERCIAL DEVELOPMENT TOWN OF BLOOMING GROVE ORANGE COUNTY, NEW YORK

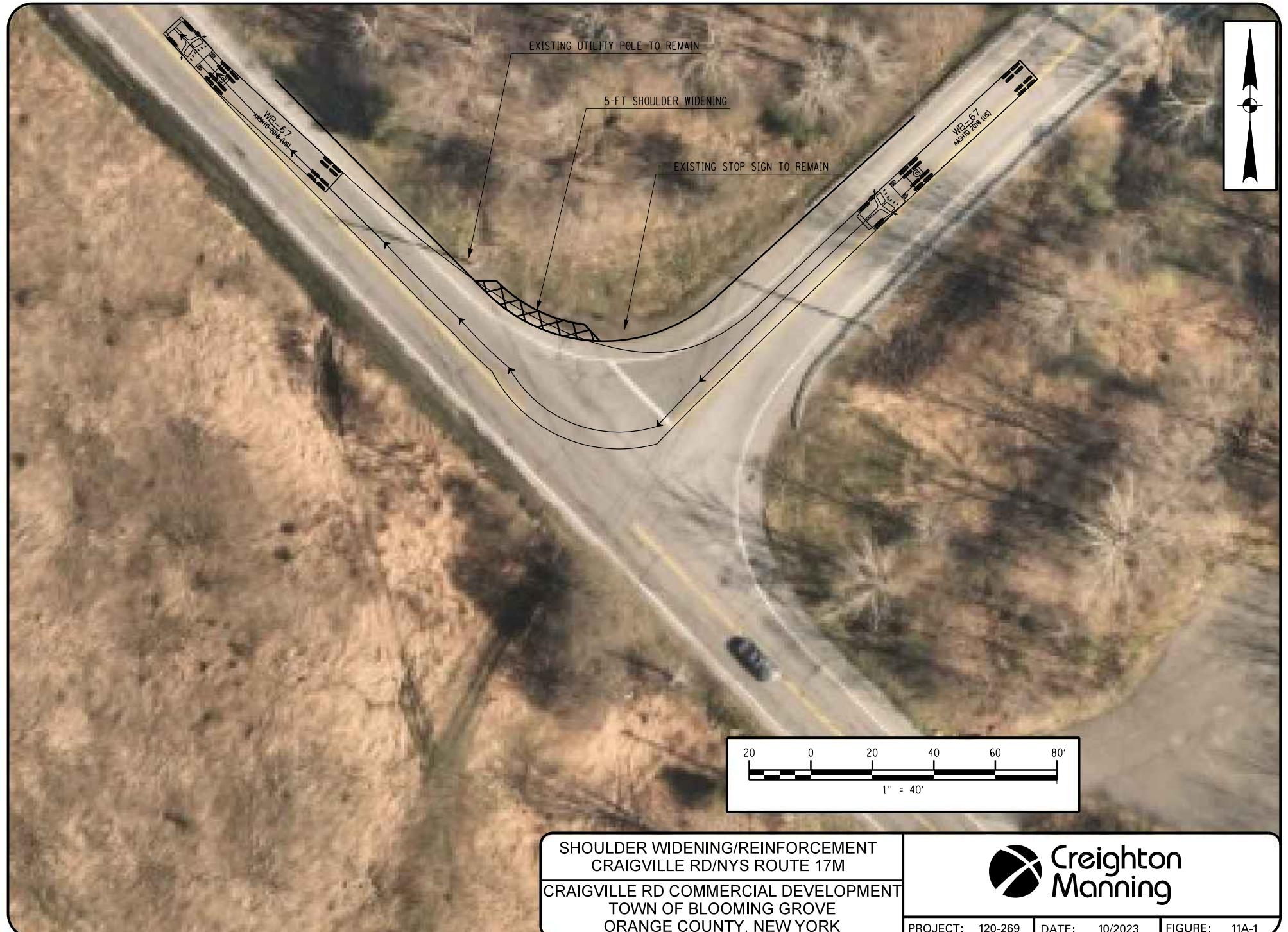
**Creighton  
Manning**

PROJECT: 120-269 DATE: 11/2022 FIGURE: 8





















**ATTACHMENT A**  
**COLLISION SUMMARY SHEETS AND TE-213**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK

## Accident Summary Sheet

Location: Craigville Rd & 17M  
 Period Covered: 3/01/2019-2/28/2022  
 Date: 3/25/2022

City: Blooming Grove  
 County: Orange

Time of Day			Weather		
	#	%	#	%	
0600-1000	1	17	Clear	3	50
1000-1600	3	50	Cloudy	3	50
1600-1900	0	0	Rain/Snow	0	0
1900-2400	1	17	Sleet/Hail/Freezing Rain	0	0
2400-0600	1	17	Fog/Smog/Smoke	0	0
Uknown	0	0	Other/Unknown	0	0
Total	6	100.00%	Total	6	100.00%
Light Condition			Time of Year		
	#	%	#	%	
Daylight	3	50	Winter (Dec-Feb)	2	33
Dawn	1	17	Spring (Mar-May)	2	33
Dusk	0	0	Summer (Jun-Aug)	1	17
Dark Lighted	0	0	Fall (Sep-Nov)	1	17
Dark Unlighted	2	33	Total	6	100.00%
Unknown	0	0			
Total	6	100.00%			
Accident Type			Roadway Characteristics		
	#	%	#	%	
Overtaking	0	0	Straight & Level	4	67
Rear End	0	0	Straight & Grade	0	0
Right Angle	2	33	Straight & Hillcrest	0	0
Left Turn	1	17	Curve & Level	1	17
Sideswipe	0	0	Curve & Grade	1	17
Run Off Road	1	17	Curve & Hillcrest	0	0
Fixed Object	0	0	Unknown	0	0
Pedestrian	0	0	Total	6	100.00%
Bicycle	0	0			
Animal	2	33			
Right Turn	0	0			
Head On	0	0			
Other	0	0			
Total	6	100.00%			
Accident Severity			Roadway Surface Condition		
	#	%	#	%	
Fatal	0	0	Dry	6	100
Serious Injury	0	0	Wet	0	0
Other Injury	1	17	Muddy	0	0
Prop damage Only	5	83	Snow/Ice	0	0
Unknown	0	0	Slush	0	0
Total	6	100.00%	Unknown	0	0
			Total	6	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.			ROUTE NO. or STREET NAME Craigville Rd.										COUNTY: Orange MUNICIPALITY : Blooming Grove BY: LSC DATE: 3/25/22			
			AT INTERSECTION WITH / OR BETWEEN: 17M													
NO. OF MONTHS 36		LIGHT CONDITIONS (LC)				ROADWAY CHARACTER (RC)					ROADWAY SURFACE CONDITION (RSC)		WEATHER (WEA)			
Begin Date: 3/01/2019 End Date: 2/28/2022		1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted				1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest					1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other		1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other			
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION			
1	38826132	4/16/2021	14:03	2	PDO	1	1	1	2	07, YY	17M83013138	RIGHT ANGLE	Unit #1 was making a left turn onto State Route 17M from Craigville Rd in a southern direction. Unit 1 failed to yield the right of way of unit 2 traveling west on State Route 17M. This caused both vehicles to collide with each other. Operator of Unit #1 stated she stopped and another vehicle came west and made the turn onto Craigville Rd. She proceeded to make the turn and didn't see the truck. Operator of Unit #2 stated he was traveling west behind another vehicle. The other vehicle turned right onto Criagville Rd and Unit #1 pulled out in front of him.			
2	38782612	3/10/2021	13:30	2	INJURY	1	1	1	1	07, YY	17M83013138	LEFT TURN (WITH OTHER CAR)				
3	38156081	10/26/2019	03:15	1	PDO	5	5	1	1	61, YY	17M83013138	DEER	Vehicle #1 traveling West on State Route 17M when a deer entered the roadway in Vehicle 1's path. Vehicle 1 collided with deer.			
4	38957331	7/16/2021	23:55	1	PDO	5	1	1	2	17, 69	17M83013138	RAN OFF ROAD ONLY	Vehicle #1 traveling South on Craigville Road. Vehicle #1 proceeded into intersection across both East and Westbound lanes of State Route 17M and off the roadway into a field off the Southern side of State Route 17M. Operator of Vehicle #1 stated he did not see the stop sign because he was driving with his lights off. Operator of Vehicle #1 stated he attempted to back out of the field but got stuck. Tickets Issued: Driver of vehicle number (1) tickets: Ticket Number: C632DCQ569 Violation: 5111A Ticket Number: C632DCQ5HT Violation: 5091 Ticket Number: C632DCQ5JW Violation: 1172A Ticket Number: C632DCQ5MX Violation: 3752A1;			
5	37915685	6/5/2019	15:15	2	PDO	1	1	1	2	07, YY	17M83013139	RIGHT ANGLE	V1 South on Craigville Rd. Town of Blooming Grove at stop sign. V2 West on State Route 17M. V1 fails to yield right of V2 and pulls out in front of V2. Collision occurs on State Route 17M causing the above stated damage.			
6	38308697	12/26/2019	06:40	1	PDO	2	4	1	1	YY	17M83013138	DEER				

## Accident Summary Sheet

Location:	Craigville Rd & Old Mansion Rd.	City:	Blooming Grove
Period Covered:	3/01/2019-2/28/2022	County	Orange
Date:	3/28/2022		

Time of Day			Weather		
	#	%		#	%
0600-1000	1	33	Clear	2	67
1000-1600	1	33	Cloudy	0	0
1600-1900	1	33	Rain/Snow	1	33
1900-2400	0	0	Sleet/Hail/Freezing F	0	0
2400-0600	0	0	Fog/Smog/Smoke	0	0
Unknown	0	0	Other/Unknown	0	0
Total	3	100.00%	Total	3	100.00%

Light Condition			Time of Year		
	#	%		#	%
Daylight	2	67	Winter (Dec-Feb)	0	0
Dawn	1	33	Spring (Mar-May)	1	33
Dusk	0	0	Summer (Jun-Aug)	2	67
Dark Lighted	0	0	Fall (Sep-Nov)	0	0
Dark Unlighted	0	0	Total	3	100.00%
Unknown	0	0			
Total	3	100.00%			

Accident Type			Roadway Characteristics		
	#	%		#	%
Overtaking	0	0	Straight & Level	1	33
Rear End	1	33	Straight & Grade	1	33
Right Angle	1	33	Straight & Hillcrest	0	0
Left Turn	0	0	Curve & Level	0	0
Sideswipe	0	0	Curve & Grade	1	33
Run Off Road	0	0	Curve & Hillcrest	0	0
Fixed Object	0	0	Unknown	0	0
Pedestrian	0	0	Total	3	100.00%
Bicycle	0	0			
Animal	1	33			
Right Turn	0	0			
Head On	0	0			
Other	0	0			
Total	3	100.00%			

Accident Severity			Roadway Surface Condition		
	#	%		#	%
Fatal	0	0	Dry	1	33
Serious Injury	1	33	Wet	2	67
Other Injury	1	33	Muddy	0	0
Prop damage Only	1	33	Snow/Ice	0	0
Unknown	0	0	Slush	0	0
Total	3	100.00%	Unknown	0	0
			Total	3	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.			ROUTE NO. or STREET NAME: Craigville Rd.										COUNTY: Orange MUNICIPALITY : Blooming Grove BY: LSC DATE: 3/28/22			
			AT INTERSECTION WITH / OR BETWEEN: Old Mansion Rd.													
NO. OF MONTHS		LIGHT CONDITIONS (LC)				ROADWAY CHARACTER (RC)					ROADWAY SURFACE CONDITION (RSC)		WEATHER (WEA)			
Begin Date: 3/01/2019 End Date: 2/28/2022		1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted				1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest					1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other		1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other			
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION			
1	38472100	6/29/2020	17:30	2	PDO	1	5	2	3	09, YY		REAR END				
2	38055481	8/22/2019	12:20	2	INJURY	1	2	1	1	04, YY		RIGHT ANGLE	Vehicle #1 traveled west on Old Mansion Rd and stopped at stop sign at intersection of Old Mansion Rd and Craigville Rd. Vehicle #2 traveled south on Craigville Rd and turned left onto Old Mansion Rd. Vehicle #1 collided into vehicle #2. NOTE: Operator of vehicle #1 stated that he didn't see vehicle #2 due to the size and color over his vehicle.			
3	38416514	5/19/2020	05:17	1	NR	2	1	1	1	61, YY		DEER	Vehicle #1 traveling northbound on Craigville Road collided with a deer.			

## Accident Summary Sheet

Location:	NYS17M/NYS17 On-Ramp	City:	Chester
Period Covered:	3/01/2019-2/28/2022	County:	Orange
Date:	4/15/2022		

Time of Day		Weather	
#	%	#	%
0600-1000	0	Clear	1
1000-1600	1	Cloudy	1
1600-1900	1	Rain/Snow	1
1900-2400	1	Sleet/Hail/Freezing Rain	0
2400-0600	0	Fog/Smog/Smoke	0
Uknown	0	Other/Unknown	0
Total	3	Total	3
	100.00%		100.00%
Light Condition		Time of Year	
#	%	#	%
Daylight	1	Winter (Dec-Feb)	1
Dawn	0	Spring (Mar-May)	1
Dusk	0	Summer (Jun-Aug)	0
Dark Lighted	0	Fall (Sep-Nov)	1
Dark Unlighted	2	Total	3
Unknown	0		100.00%
Total	3		
	100.00%		
Accident Type		Roadway Characteristics	
#	%	#	%
Overtaking	0	Straight & Level	2
Rear End	2	Straight & Grade	1
Right Angle	0	Straight & Hillcrest	0
Left Turn	0	Curve & Level	0
Sideswipe	0	Curve & Grade	0
Run Off Road	0	Curve & Hillcrest	0
Fixed Object	0	Unknown	0
Pedestrian	0	Total	3
Bicycle	0		100.00%
Animal	1		
Right Turn	0		
Head On	0		
Other	0		
Total	3		
	100.00%		
Accident Severity		Roadway Surface Condition	
#	%	#	%
Fatal	0	Dry	1
Serious Injury	1	Wet	1
Other Injury	0	Muddy	1
Prop damage Only	2	Snow/Ice	0
Unknown	0	Slush	0
Total	3	Unknown	0
	100.00%	Total	3
	100.00%		100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.			ROUTE NO. or STREET NAME: 17M										COUNTY: Orange MUNICIPALITY: Chester BY: LSC DATE: 4/15/22	
			AT INTERSECTION WITH / OR BETWEEN: NY17 SB On-Ramp											
NO. OF MONTHS 36		LIGHT CONDITIONS (LC)			ROADWAY CHARACTER (RC)					ROADWAY SURFACE CONDITION (RSC)		WEATHER (WEA)		
Begin Date: 3/01/2019 End Date: 2/28/2022			1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted			1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest					1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other		1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other	
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION	
1	37862419	4/24/2019	15:10	2	PDO	1	1	1	1	05, 09, YY, ZZ	17M83013124	REAR END	Vehicle 1(Dombrowski) stated he was stopped in the eastbound lane at the intersection of 17m and the 127 on ramp to 17east. waiting to turn left. Vehicle 2(Odell) traveling eastbound on 17m could not stop in time and struck the rear of vehicle 1 causing damage. Odell was cited for driving too closely.	
2	38681648	12/22/2020	16:55	1	PDO	5	1	1	2	61, YY	17M83013124	DEER	V1 was traveling N/W on State Hwy 17M nearest the 127 on ramp. A deer entered the roadway and the operator struck the deer with the front of her vehicle. Deer fur observed on the vehicle.	
3	39065664	10/16/2021	19:25	2	INJURY	5	2	2	3	09, 19, YY	17M83013124	REAR END	V1 and V2 traveling east on State Hwy 17m when V1 slowed to make left turn onto the 127E on ramp. V2 failed to brake resulting in a rear end collision. The operators of both vehicles complained of minor injuries. V1 had pain/discomfort in her back and neck and the operator of V2 had wrist pain. An ambulance responded as a precaution and checked on both subjects. Both parties refused transport to the hospital and signed waivers for the ambulance.	

## Accident Summary Sheet

Location:	17M & Lehigh Ave/Kings Highway	City:	Chester
Period Covered:	3/1/2019-2/28/2022	County:	Orange
Date:	4/15/2022		

Time of Day			Weather		
	#	%		#	%
0600-1000	1	50	Clear	1	50
1000-1600	0	0	Cloudy	0	0
1600-1900	0	0	Rain/Snow	1	50
1900-2400	1	50	Sleet/Hail/Freezing Rain	0	0
2400-0600	0	0	Fog/Smog/Smoke	0	0
Uknown	0	0	Other/Unknown	0	0
Total	2	100.00%	Total	2	100.00%
Light Condition			Time of Year		
	#	%		#	%
Daylight	0	0	Winter (Dec-Feb)	1	50
Dawn	0	0	Spring (Mar-May)	0	0
Dusk	0	0	Summer (Jun-Aug)	1	50
Dark Lighted	0	0	Fall (Sep-Nov)	0	0
Dark Unlighted	2	100	Total	2	100.00%
Unknown	0	0			
Total	2	100.00%			
Accident Type			Roadway Characteristics		
	#	%		#	%
Overtaking	0	0	Straight & Level	1	50
Rear End	2	100	Straight & Grade	1	50
Right Angle	0	0	Straight & Hillcrest	0	0
Left Turn	0	0	Curve & Level	0	0
Sideswipe	0	0	Curve & Grade	0	0
Run Off Road	0	0	Curve & Hillcrest	0	0
Fixed Object	0	0	Unknown	0	0
Pedestrian	0	0	Total	2	100.00%
Bicycle	0	0			
Animal	0	0			
Right Turn	0	0			
Head On	0	0			
Other	0	0			
Total	2	100.00%			
Accident Severity			Roadway Surface Condition		
	#	%		#	%
Fatal	0	0	Dry	1	50
Serious Injury	1	50	Wet	0	0
Other Injury	0	0	Muddy	1	50
Prop damage Only	1	50	Snow/Ice	0	0
Unknown	0	0	Slush	0	0
Total	2	100.00%	Unknown	0	0
			Total	2	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.			ROUTE NO. or STREET NAME: 17M										COUNTY : Orange MUNICIPALITY : Chester BY: LSC DATE: 4/15/2022	
			AT INTERSECTION WITH / OR BETWEEN: Lehigh Ave/Kings Highway											
NO. OF MONTHS 36		LIGHT CONDITIONS (LC)				ROADWAY CHARACTER (RC)					ROADWAY SURFACE CONDITION (RSC)			WEATHER (WEA)
Begin Date: 3/01/2019 End Date: 2/28/2022		1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted				1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest					1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other			1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION	
1	38521875	8/1/2020	22:25	2	INJURY	5	2	1	1	09, YY		REAR END	Driver of vehicle 1 states that he was slowing down at the intersection of Lehigh and Brookside Ave when he was struck in the rear by vehicle 2, causing two small indentations from V2s license plate onto V1s rear bumper. Driver of V2 states that V1 stopped suddenly and she did not have time to stop before bumping into the back of V1. V2 sustained no damage to the vehicle, only a slightly bent license plate. Passenger in V1 had recent previous back surgery that appeared to be agitated by the accident and was transported to Garnet Health Medical Center for evaluation.	
2	39162175	12/22/2021	06:09	2	PDO	5	1	2	3	03, YY	17M83013113	REAR END	OPV1 states he was stopped at the red traffic signal on 17M and attempted to back up subsequently striking V2. OPV2 states he was stopped at the red traffic signal on 17M when V1 began traveling in reverse subsequently striking V2.	

## Accident Summary Sheet

Location: NYS 17M & Summerville Wa City: Chester  
 Period Covered: 3/01/2019-2/28/2022 County: Orange  
 Date: 4/13/2022

Time of Day		Weather			
	%	#	%		
0600-1000	1	5	Clear	11	52
1000-1600	9	43	Cloudy	7	33
1600-1900	4	19	Rain/Snow	2	10
1900-2400	5	24	Sleet/Hail/Freezing R	0	0
2400-0600	2	10	Fog/Smog/Smoke	0	0
Unknown	0	0	Other/Unknown	1	5
Total	21	5	Total	21	100.00%
Light Condition		Time of Year			
	#	%	#	%	
Daylight	11	52	Winter (Dec-Feb)	6	29
Dawn	1	5	Spring (Mar-May)	2	10
Dusk	0	0	Summer (Jun-Aug)	5	24
Dark Lighted	8	38	Fall (Sep-Nov)	8	38
Dark Unlighted	0	0	Total	21	100.00%
Unknown	1	5			
Total	21	100.00%			
Accident Type		Roadway Characteristics			
	#	%	#	%	
Overtaking	2	10	Straight & Level	14	67
Rear End	8	38	Straight & Grade	6	29
Right Angle	3	14	Straight & Hillcrest	0	0
Left Turn	4	19	Curve & Level	0	0
Sideswipe	1	5	Curve & Grade	0	0
Run Off Road	0	0	Curve & Hillcrest	0	0
Fixed Object	0	0	Unknown	1	5
Pedestrian	0	0	Total	21	100.00%
Bicycle	0	0			
Animal	0	0			
Right Turn	1	5			
Head On	1	5			
Other	1	5			
Total	21	100.00%			
Accident Severity		Roadway Surface Cond			
	#	%	#	%	
Fatal	0	0	Dry	14	67
Serious Injury	4	19	Wet	4	19
Other Injury	3	14	Muddy	0	0
Prop damage Only	14	67	Snow/Ice	1	5
Unknown	0	0	Slush	0	0
Total	21	100.00%	Unknown	2	10
			Total	21	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.		ROUTE NO. or STREET NAME: NYS 17M										COUNTY: Orange MUNICIPALITY: Chester BY: LSC DATE: 4/13/22	
		AT INTERSECTION WITH / OR BETWEEN: NYS94/Summerville Way/Academy Ave											
NO. OF MONTHS 36		LIGHT CONDITIONS (LC)			ROADWAY CHARACTER (RC)				ROADWAY SURFACE CONDITION (RSC)		WEATHER (WEA)		
Begin Date: 3/01/2019 End Date: 2/28/2022		1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted			1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest				1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other		1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other		
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
1	38588931	10/8/2020	19:50	2	PDO	4	1	1	1	17, YY	17M83013107	LEFT TURN (AGAINST OTHER CAR)	Driver of vehicle 1 states she was in the center lane on Summerville Way and saw the green light to go straight and made a left turn onto Brookside Ave and she did not see vehicle 2 when she struck V2 with V1's front passenger side bumper/head lamp. Driver of V2 states that he was on Academy Ave and had the green light so was making the right turn onto Brookside Ave when V1 came out of nowhere and struck him in the driver side front side scratching to the driver side rear door. Witness states that he observed V1 make a left turn from the center lane on Summerville Way, when the light was green to go straight but there was a no left turn red arrow, and he observed both vehicles making the turns as stated above.
2	38080568	9/12/2019	12:00	3	PDO	1	2	1	1	05, 09, YY	17M83013107	OTHER	
3	38291980	1/20/2020	13:41	2	INJURY	1	1	1	2	04, YY	17M83013107	REAR END	Operator of V1 stated that she was stopped at the red light on Brookside Ave(SR17M), awaiting to turn left onto Academy Ave(SR94) when V2 struck the rear of V1. Operator of V2 stated that she was traveling behind V1 and did not realize that the light was red or that V1 was stopped and V2 struck the rear of V1. Operator of V1 advised that she felt slightly dizzy from the impact. Operator of V2 did not complain of any pain. EMS responded and checked both operators. Both operators refused medical treatment.
4	37919543	5/24/2019	17:30	2	PDO	Z	Z	Z	Z	XX	94 83011148	UNKNOWN	
5	38707383	12/8/2020	13:20	2	NR	1	1	1	1	03, YY	17M83013107	REAR END	Operator of V1 advised that she was leaving the parking lot of the CVS, in a northerly direction, when V2 backed out of a parking space and struck the plastic grille on the front of V1 with the rear bumper area of V2. V2 proceeded to leave the scene of the auto accident. The operator of V1 advised that V2 was described as a Dark colored SUV, bearing an unknown registration. Operator of V1 advised that she was uninjured. V1 was operated from the scene.

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
6	38614571	10/22/2020	14:40	2	PDO	1	1	1	1	09, YY	94 83011148	REAR END	Operator of V1 stated that he was stopped in traffic due to a red light on Academy Ave. The light turned green and he went forward, when he got onto Summerville Way operator of V2 came out of her vehicle and stated that he had hit her vehicle, he didn't know he hit her. Operator of V2 stated that she was stopped in traffic at a red light on Academy Ave, when the light turned green the vehicle in front of her did not move straight away, so she couldn't move. That is when V1 struck her vehicle in the back bumper and trunk. Operator of V2 then moved forward and the operator of V1 struck her two more times in the same spot. When operator of V2 was stopped in traffic on Summerville Way she got out of her vehicle and told operator of V1 that he had struck her 3 times, he stated he did not know. V1 sustained no damage to its front bumper. V2 sustain damage to its trunk. No injures reported and no vehicles towed.
7	38596807	10/14/2020	15:05	2	INJURY	1	1	1	1	09, YY		REAR END	V1 was slowing to make a right hand turn into the Shell parking lot, when V2 struck V1 in the middle rear bumper area with the right headlight/ bumper area of V2 causing damage to both vehicles. V2 was towed from the scene by Freemans Towing. The operator of V1 advised that she was experiencing mild neck pain but did not require EMS. V1 was operated from the scene. The operator of V2 reported no injuries.
8	39120564	11/23/2021	19:07	2	INJURY	4	2	1	2	05, 07, YY	17M83013107	LEFT TURN (AGAINST OTHER CAR)	
9	38944275	7/16/2021	17:40	2	PDO	1	2	2	3	66, YY	94 83011148	REAR END	Driver of V1 states that she was approaching the light on Summerville Way (SR94), intersection Brookside Ave, when the light turned yellow she applied her brakes and came to a stop, and was then struck in the rear by V2. Driver of V2 states that he was behind V1 and believed V1 was going to go through the yellow light when V1 came to a stop he applied his brakes but due to the wet pavement slid into V1.V1 sustained minor damage on the rear bumper, and a cracked passenger side brake lamp cover.V2 sustained a minor bend in the center of his hood, as well as a cracked driver side head lamp.
10	38520947	8/17/2020	13:50	2	PDO	1	1	1	2	09, YY	94 83011148	REAR END	Operator of vehicle #1, Doty, states that she was stopped in traffic for the red traffic signal on Route 94, at Brookside Avenue, when she was suddenly struck from behind vehicle #2. Doty states that operator vehicle #2 apologized to her and asked her to pull in to the Mcdonalds parking lot so that they could exchange information. Doty states that when she pulled in to the Mcdonalds parking lot, vehicle #2 kept going and she then lost sight of vehicle #2. No injuries reported. Vehicle operated from location.
11	38680829	12/14/2020	18:23	2	PDO	4	1	2	2	07, YY	17M83013107	LEFT TURN (AGAINST OTHER CAR)	Vehicle 2 was traveling east on Brookside Ave through the intersection and was struck in the front of the vehicle causing damage. Vehicle 1 was traveling North turning a left onto Academy Ave from Brookside and failed to yield the right of way. Vehicle 1 has damage to the rear right panel and tire.

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
12	38945406	7/25/2021	22:30	2	PDO	4	1	2	3	66, 69	17M83013107	OVERTAKING	BOTH V#1 AND V#2 WERE BOTH STOPPING AT THE RED LIGHT ON BROOKSIDE AVE AT THE SAME TIME. DUE TO HEAVY RAIN AND SLIPPERY PAVEMENT BOTH VEHICLES SIDE SWIPED EACH OTHER AT THE LIGHT. V#1 SUSTAINED HEAVY DAMAGE TO THE PASSENGER SIDE DOOR, AND V#2 (TOW TRUCK) DID NOT SUSTAIN ANY DAMAGE. BOTH VEHICLES DRIVEN FROM THE SCENE.
13	38981948	8/14/2021	16:05	2	PDO	1	1	1	1	07, 20, YY	17M83013107	RIGHT ANGLE	The operator of V1 states he was leaving McDonalds parking lot in a northerly direction and stopped in between two vehicles in the center lane due to traffic. The operator of V1 further advised that he honked his horn and proceeded to the third lane towards the Shell parking lot. At this time V2 was traveling in a westerly direction and struck the passenger side front bumper area of V1 with the driver side front bumper area of V2. The operator of V2 states that she was traveling in a westerly direction on Brookside Ave when V1 pulled out in front of her vehicle. V1 was towed by freeman&apos;s. V2 was operated from the scene. No reported injuries on scene.
14	38565794	8/29/2020	14:10	2	NR	1	1	1	1	07, YY		LEFT TURN (AGAINST OTHER CAR)	V1 was on Academy Ave, traveling in a Southerly direction. V1 was approaching the stop light in the left hand turning lane when V1 was struck by V2 in the left front headlight area of V1. Operator of V1 advised that the operator of V2 left the scene on an Orange and White colored motorcycle, traveling in a southerly direction from Academy Ave and onto Summerville Way. No injuries at the location. V1 operated from the scene. The operator of V2 was later identified via video footage from the shell gas station. The operator of V2 returned to the same gas station wearing the same clothing, and operating the same motorcycle. V2&apos;s registration did not come back to the same motorcycle and the operator of V2 did flee from law enforcement on the motorcycle prior to Officer&apos;s being able to obtain the VIN # of V2. The operator of V2 was later apprehended by Police and cited for the above mentioned vehicle and traffic law infractions. Police were unable to locate or identify V2. Tickets Issued: Driver of vehicle number (2) tickets: Ticket Number: STA2C8XZDD Violation: 6001A Ticket Number: STA2C8XZZW Violation: 5112A2 Ticket Number: STA2C8Z02G Violation: 5091 Ticket Number: STA2C8Z03R Violation: 4101 Ticket Number: STA2C8Z05K Violation: 1143;
15	39102213	11/7/2021	01:00	1	PDO	4	2	Z	1	XX	94 83011148	OTHER BARRIER	
16	38106702	9/27/2019	00:00	2	PDO	4	2	1	1	07, YY	17M83013107	RIGHT TURN (AGAINST OTHER CAR)	The accident occurred in a police vehicle owned/operated by the Village of Chester Police while responding to an emergency. V-1 traveling west, approaches an intersection with a red traffic signal. V-2 is traveling northwest and enters the intersection. V-1 slows to make a right turn and collides with V-2.

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
17	38680834	11/19/2020	19:20	2	PDO	4	1	1	1	07, YY	17M83013107	RIGHT ANGLE	V1 traveling NE on Brookside Ave, making a left turn on to Academy Ave, failed to yield to V2 traveling westbound on Brookside Ave. V1 collided into V2 causing damage. V1 sustained driver side front damage and V2 sustained driver side front quarter panel and front driver side wheel damage. Passenger of V2 sustained minor injuries but refused medical attention. No vehicles towed from scene.
18	38335137	2/9/2020	07:00	2	INJURY	2	2	4	2	17, YY	17M83013107	RIGHT ANGLE	Driver of vehicle 1 states that she was driving Eastbound on Brookside Ave and came up to the light at Academy Ave, which had a red light and green right turn arrow, the light then turned green for her to go straight. When V1 entered the intersection she was struck in the driver side front by vehicle 2 which driver of V1 states came from the opposite side of the light and made a left turn. Driver of V1 stated she was not injured, but felt sore in her chest area. Driver of v2 appeared to be disoriented, possibly in shock, stating that she had the green light to go and she made the left turn to go to Summerville Way. Driver states her wrists hurt from the airbag deployment, and was experiencing pain in her right hip. P/O Guarneiri did view video from the Mobil Gas Station, which shows the intersection, and shows that V1 had a green light to go straight Eastbound, which would mean that V2 would have red arrows at the time of accident, which occurred at exactly 0700 camera time.
19	38769489	2/23/2021	15:43	2	NR	1	1	2	2	13, YY	17M83013107	OVERTAKING	Driver of V1 stated that both vehicles were traveling West Bound on Brookside Avenue, when they were both at the intersection to make a left Southbound on to Summerville Way, when they both collided with each other causing damage to V1's driver side mirror, and V2 sustained minor damage to the passenger side mirror. V2 was on the farthest left lane to turn left, and V1 was on the right lane beside V2 to turn left. * Driver of V2 confirmed the telling of the incident.*
20	38310660	1/17/2020	20:04	2	PDO	4	1	1	1	09, YY	94 83011148	REAR END	
21	38450835	5/19/2020	12:43	2	PDO	1	1	1	2	04, YY		REAR END	Operator of V2 states that he was stopped at the red light on Academy Ave when V1 struck the rear of V2. Operator of V2 states that he observed Operator of V1's head down, and believed that the operator may have been on the cell phone. Operator of V2 states that he was traveling South on Academy Ave(SR94) and was distracted due to personal reasons and thought that the light was green and that V2 was moving. Operator of V1 denied being on his cell phone. No reported injuries at the time of the incident.

## Accident Summary Sheet

Location:	NY Rt. 94 & Rt. 17M N/bound Off/c	City:	Chester
Period Covered:	3/01/2019-2/28/2022	County:	Orange
Date:	4/20/2022		

Time of Day		Weather			
#	%	#	%		
0600-1000	1	14	Clear	5	71
1000-1600	3	43	Cloudy	2	29
1600-1900	2	29	Rain/Snow	0	0
1900-2400	1	14	Sleet/Hail/Freezing Rain	0	0
2400-0600	0	0	Fog/Smog/Smoke	0	0
Uknown	0	0	Other/Unknown	0	0
Total	7	100.00%	Total	7	100.00%

Light Condition		Time of Year			
#	%	#	%		
Daylight	5	71	Winter (Dec-Feb)	1	14
Dawn	0	0	Spring (Mar-May)	2	29
Dusk	1	14	Summer (Jun-Aug)	2	29
Dark Lighted	1	14	Fall (Sep-Nov)	2	29
Dark Unlighted	0	0	Total	7	100.00%
Unknown	0	0			
Total	7	100.00%			

Accident Type		Roadway Characteristics			
#	%	#	%		
Overtaking	0	0	Straight & Level	3	43
Rear End	3	43	Straight & Grade	4	57
Right Angle	2	29	Straight & Hillcrest	0	0
Left Turn	2	29	Curve & Level	0	0
Sideswipe	0	0	Curve & Grade	0	0
Run Off Road	0	0	Curve & Hillcrest	0	0
Fixed Object	0	0	Unknown	0	0
Pedestrian	0	0	Total	7	100.00%
Bicycle	0	0			
Animal	0	0			
Right Turn	0	0			
Head On	0	0			
Other	0	0			
Total	7	100.00%			

Accident Severity		Roadway Surface Condition			
#	%	#	%		
Fatal	0	0	Dry	5	71
Serious Injury	1	14	Wet	2	29
Other Injury	1	14	Muddy	0	0
Prop damage Only	5	71	Snow/Ice	0	0
Unknown	0	0	Slush	0	0
Total	7	100.00%	Unknown	0	0
			Total	7	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.			ROUTE NO. or STREET NAME: NYS Route 94 & 17										COUNTY: Orange MUNICIPALITY: Chester BY: LSC DATE: 4/20/22	
			AT INTERSECTION WITH / OR BETWEEN: Exit 126 Northbound Off/On-Ramp											
NO. OF MONTHS 36		LIGHT CONDITIONS (LC)				ROADWAY CHARACTER (RC)					ROADWAY SURFACE CONDITION (RSC)			WEATHER (WEA)
Begin Date:03/01/2019 End Date: 2/28/2022		1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted				1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest					1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other			1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION	
1	38922192	7/6/2021	14:40	2	PDO	1	2	1	1	09, YY		REAR END	Operator of V2 states he is stopped at a red light at the intersection of State Route 17 west exit 126 off ramp and Summerville Way. Operator of V1 states he is slowing down to stop for the red light, is unable to brake in time and rear ends V2. V2 is out of state NJ Farmers Insurance Policy #193207770	
2	38473926	5/24/2020	16:20	2	PDO	1	1	1	1	17, 26, YY	94 83011146	LEFT TURN (AGAINST OTHER CAR)	Operator of V1 advised that she was making the left turn on a green light when she was struck by V2. The operator of V2 advised that he had gone through the red light while traveling in Northerly direction on Summerville Way in an effort to avoid being struck by a Black SUV on the driver's side of V2. The Black SUV was not on scene when units arrived. Both vehicles were towed from the scene due to damage to V1's Driver side rear wheel and damage to V2's Driver side front wheel. EMS responded to the scene. Both parties refused Medical Attention. No injuries sustained by either operator.	
3	38267329	1/7/2020	16:24	2	PDO	3	1	1	1	07, 17, YY	94 83011146	RIGHT ANGLE	OP OF V-2 INITIATING LEFT TURN FROM EXIT 126 OFF RAMP ONTO STATE RT 94 T/CHESTER. OP OF V-1 TRAVELING S/BOUND ON STATE RT 94 DISREGARDS TRAFFIC CONTROL DEVICE SUBSEQUENTLY STRIKING V-2 AT A RIGHT ANGLE. NO INJURIES TO REPORT, NO TOW NEEDED.	
4	38860519	5/19/2021	09:00	2	NR	1	2	1	1	09, YY	94 83011146	REAR END	V1 behind V2 in turning lane, North in traffic at light on State Route 94. Op V1 states that her foot came off the brake and her vehicle rolled into V2. V2 states he was waiting to turn onto State Route 17 West ramp when his vehicle was struck.	

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
5	39075013	10/18/2021	15:37	2	PDO	1	2	1	1	04, 07, YY	94 83011146	LEFT TURN (AGAINST OTHER CAR)	Operator of V1 was in the left turning lane on Summerville way, while having a steady green light he made the left onto S/R 17 West on ramp. As he was turning V2 was traveling West on Summerville Way had a steady green light and going straight when V1 and V2 collided into each other. Operator of V1 stated someone honked at him, he got distracted and that's why he didn't see V2 going straight. V1 sustained damage to its passenger rear quarter panel when it collided with V2, as well as, damage to its driver side rear bumper and driver side taillight, from striking the guardrail. V2 sustained damage to its center and passenger side front bumper. No injuries reported on scene. V2 was towed by Pat's towing to Pat's towing yard. V1 was towed by Pat's towing to operator of V1's residence.
6	38204523	11/28/2019	13:47	2	PDO	1	2	1	2	02, 09, YY	94 83011146	REAR END	V2 comes to a stop on State Route 17W off ramp at red light. V1 following to closely rear ends V2.
7	38922569	6/19/2021	23:20	2	INJURY	4	1	2	2	17, YY	94 83011146	RIGHT ANGLE	Driver of V1 advised she was going straight through a green light and V2 pulled out in front of her. Driver of V2 advised he had the green light and V1 struck his vehicle. Driver of vehicle 2 was transported to hospital with chest pain.

## Accident Summary Sheet

Location:	NY Rt. 94 & Rt. 17M S/bound Off/C	City:	Chester
Period Covered:	2/28/2019-2/28/2022	County:	Orange
Date:	4/20/2022		

Time of Day			Weather		
0600-1000	3	43	Clear	6	86
1000-1600	1	14	Cloudy	1	14
1600-1900	2	29	Rain/Snow	0	0
1900-2400	1	14	Sleet/Hail/Freezing Rain	0	0
2400-0600	0	0	Fog/Smog/Smoke	0	0
Uknown	0	0	Other/Unknown	0	0
Total	7	100.00%	Total	7	100.00%
Light Condition			Time of Year		
#	%		#	%	
Daylight	5	71	Winter (Dec-Feb)	0	0
Dawn	0	0	Spring (Mar-May)	2	29
Dusk	0	0	Summer (Jun-Aug)	3	43
Dark Lighted	2	29	Fall (Sep-Nov)	2	29
Dark Unlighted	0	0	Total	7	100.00%
Unknown	0	0			
Total	7	100.00%			
Accident Type			Roadway Characteristics		
#	%		#	%	
Overtaking	0	0	Straight & Level	4	57
Rear End	2	29	Straight & Grade	2	29
Right Angle	0	0	Straight & Hillcrest	1	14
Left Turn	3	43	Curve & Level	0	0
Sideswipe	1	14	Curve & Grade	0	0
Run Off Road	0	0	Curve & Hillcrest	0	0
Fixed Object	0	0	Unknown	0	0
Pedestrian	0	0	Total	7	100.00%
Bicycle	0	0			
Animal	0	0			
Right Turn	0	0			
Head On	0	0			
Other	1	14			
Total	7	100.00%			
Accident Severity			Roadway Surface Condition		
#	%		#	%	
Fatal	0	0	Dry	7	100
Serious Injury	3	43	Wet	0	0
Other Injury	0	0	Muddy	0	0
Prop damage Only	4	57	Snow/Ice	0	0
Unknown	0	0	Slush	0	0
Total	7	100.00%	Unknown	0	0
			Total	7	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269  P.I.N..  INVENTORY NO.		ROUTE NO. or STREET NAME: NYS Route 94 & 17											COUNTY: Orange MUNICIPALITY: Chester BY: LSC DATE: 4/20/22			
		AT INTERSECTION WITH / OR BETWEEN: Exit 126 Southbound Off/on-Ramp														
NO. OF MONTHS: 36		LIGHT CONDITIONS (LC)			ROADWAY CHARACTER (RC)				ROADWAY SURFACE CONDITION (RSC)		WEATHER (WEA)					
Begin Date: 3/01/2019  End Date: 2/28/2022		1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted			1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest				1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other		1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other					
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION			
1	38407247	4/17/2020	06:40	2	PDO	1	2	1	1	04, YY		OTHER	Operator of V1 bearing NY reg 6HF319 was stopped at the red light on State Route 17 Exit 126 East off ramp and released the brake getting ready to move forward, when his vehicle rolled back and struck V2 in its front bumper. V2 operator bearing NY Reg EUR8231 stated she was stopped in traffic at the red light on State Route 17 Exit 126 East off ramp when V1 rolled back and struck the front bumper of V2. No injuries reported, no vehicles towed.			
2	38534748	8/18/2020	06:30	2	INJURY	1	1	1	1	07, YY	94 83011146	LEFT TURN (AGAINST OTHER CAR)				
3	38539923	8/18/2020	06:35	2	PDO	1	1	1	1	05, 64, YY		SIDESWIPE	V1 was making a left hand turn in a northerly direction from Exit 126 East Off Ramp onto Summerville Way. V1 trailer side swiped TC645 in the left front wheel well area with the left rear wheel area of V1's Enclosed Trailer, while avoiding another two car PDAA in which New York State Police was handling on SR 94. Trailer Registration NY BT87370. TC 645 was parked facing a southerly direction, completely in the left hand turn lane, at the accident scene with all emergency lights activated and displayed. No injuries as a result of the MVA. Both vehicles operated from the scene.			
4	38059294	9/2/2019	14:35	2	PDO	1	2	1	2	09, YY		REAR END	V1 and V2 East on SR 17 Exit 126 off ramp in the Town of Chester. V2 stopped at red light, V1 following too closely strikes V2. V1 self Insurer P.V. Holding Corp.			

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
5	38955308	7/28/2021	17:12	2	INJURY	1	1	1	1	04, YY	94 83011146	REAR END	Driver of vehicle 1 states that he was facing Northbound waiting at the green light on Summerville way at the intersection of 126 East Bound on ramp due to heavy traffic, he was not able to cross the intersection, when he was struck in the rear by driver of vehicle 2. Driver of vehicle 2 states that he was traveling Northbound on Summerville Way (SR 94) and saw the traffic light at the intersection of the 126 East Bound on ramp was green but did not realize driver of vehicle 1 was stopped due to traffic and struck the back of vehicle 1. Vehicle 1 sustained heavy damage to the rear bumper and trunk. Vehicle 2 sustained dents to the front bumper and the license plate bracket fell off.
6	39116553	11/16/2021	17:39	2	INJURY	4	3	1	1	17, YY		LEFT TURN (AGAINST OTHER CAR)	Operator of V1 stated that she was going West on Summerville Way and V1 and V2 collided. Operator of V1 stated that she couldn't remember what color light she had. Operator of V2 stated that he was stopped at the red light on the S/R 17 exit 126 East off ramp. When the light turned green he attempted to make a left turn onto Summerville Way, that's when V1 and V2 collided. Operator of V2 stated that when both vehicles collided V1 continued to travel causing damage to the rear passenger quarter panel. Operator of V1 complained of chest pain from the seat belt and airbag. She also had swelling on her left leg and calf. Operator of V2 complained of left shoulder and head pain. Passenger of V2, Linda Byrne, complained of chest pain. All subjects were evaluated by EMS 279, all RMA&apos;d. V1 sustained major damage to its front bumper and rear passenger quarter panel. Driver steering wheel airbag deployed. V2 sustained major damage to its driver side front bumper. Both vehicles towed by Freeman&apos;s Auto.
7	38840758	4/27/2021	20:12	2	PDO	4	1	1	1	07, YY	94 83011146	LEFT TURN (AGAINST OTHER CAR)	Operator of V1 stated that he was in the left turn only lane on Summerville Way. He had a steady green light; he noticed a V2 going straight on Summerville Way and did not think that they were that close. As he was turning left on to S/R 17 Exit 126 East on ramp he stated that he was already 3/4 into the turn and V2 sped up and struck his vehicle. Operator of V2 stated that she was going straight on Summerville Way and saw V1 starting to turn. Then she noticed that the nose of the vehicle went down as if they were braking, then still make the left turn. Operator of V2 stated that they could not brake in time, and struck V1. V1 passenger side airbags deployed, and sustained major damage to the entire passenger side of the vehicle. V2 sustained major damage to its front bumper on the passenger side and front passenger quarter panel. EMS on scene and evaluated everyone on scene. All RMA&apos;d. Both vehicles towed by Freeman&apos;s towing.

## Accident Summary Sheet

Location: NYS 17 & Museum Village Road  
 Period Covered: 3/1/2019-2/28/2022  
 Date: 4/14/2022

City: Blooming Grove  
 County: Orange

Time of Day			Weather		
	#	%		#	%
0600-1000	4	33	Clear	6	50
1000-1600	3	25	Cloudy	2	17
1600-1900	3	25	Rain/Snow	3	25
1900-2400	2	17	Sleet/Hail/Freezing Rain	0	0
2400-0600	0	0	Fog/Smog/Smoke	0	0
Uknown	0	0	Other/Unknown	1	8
Total	12	100.00%	Total	12	100.00%
Light Condition			Time of Year		
	#	%		#	%
Daylight	9	75	Winter (Dec-Feb)	2	17
Dawn	0	0	Spring (Mar-May)	5	42
Dusk	0	0	Summer (Jun-Aug)	2	17
Dark Lighted	0	0	Fall (Sep-Nov)	3	25
Dark Unlighted	2	17	Total	12	100.00%
Unknown	1	8			
Total	12	100.00%			
Accident Type			Roadway Characteristics		
	#	%		#	%
Overtaking	0	0	Straight & Level	3	25
Rear End	0	0	Straight & Grade	1	8
Right Angle	4	33	Straight & Hillcrest	0	0
Left Turn	4	33	Curve & Level	0	0
Sideswipe	0	0	Curve & Grade	7	58
Run Off Road	1	8	Curve & Hillcrest	0	0
Fixed Object	1	8	Unknown	1	8
Pedestrian	0	0	Total	12	100.00%
Bicycle	0	0			
Animal	2	17			
Right Turn	0	0			
Head On	0	0			
Other	0	0			
Total	12	100.00%			
Accident Severity			Roadway Surface Condition		
	#	%		#	%
Fatal	0	0	Dry	7	58
Serious Injury	7	58	Wet	3	25
Other Injury	1	8	Muddy	0	0
Prop damage Only	4	33	Snow/Ice	0	0
Unknown	0	0	Slush	1	8
Total	12	100.00%	Unknown	1	8
			Total	12	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.			ROUTE NO. or STREET NAME: NYS Route 17M										COUNTY: Orange MUNICIPALITY : Blooming Grove BY: LSC DATE: 4/14/22		
			AT INTERSECTION WITH / OR BETWEEN: Museum Village Rd												
NO. OF MONTHS 36		LIGHT CONDITIONS (LC)			ROADWAY CHARACTER (RC)				ROADWAY SURFACE CONDITION (RSC)			WEATHER (WEA)			
Begin Date: 3/01/2019 End Date: 2/28/2022			1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted			1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest				1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other			1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other		
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION		
1	38999710	8/31/2021	11:49	2	PDO	1	5	1	1	07, YY		LEFT TURN (AGAINST OTHER CAR)	OP OF V-2 STATES HE IS TRAVELING W/B ON SR 17M WHEN V-1 EXITS MUSEUM VILLAGE ROAD AND STRIKES V-2.OP OF V-1 STATES SHE EXITS MUSEUM VILLAGE ROAD AND STRIKES V-2.		
2	39021190	9/17/2021	09:05	2	PDO	1	5	2	3	07, 69, YY	17M83013152	LEFT TURN (AGAINST OTHER CAR)	Vehicle #1 was attempting to make a left turn from Museum Village Road onto State Route 17m in a southern direction as Vehicle #2 was traveling North on State Route 17M. Vehicle #1 entered the roadway colliding with Vehicle #2. Vehicle #1 then spun out and collided with a guide rail at the intersection of Museum Village Road and State Route 17M before coming to rest. Operator of Vehicle #1 states he was stopped at the stop sign and did not see Vehicle #2 coming and thought it was safe to enter the roadway, upon entering the roadway he saw Vehicle #2 and could not stop in time. Operator of Vehicle #2 states she was traveling North on State Route 17M when Vehicle #1 pulled out in front of her, causing the collision.		
3	38048188	5/12/2019	08:22	2	INJURY	1	5	2	3	07, XX	17M83013152	RIGHT ANGLE			
4	38204903	10/31/2019	07:30	1	PDO	Z	Z	Z	Z	XX	17M83013152	DEER			
5	38880967	6/5/2021	17:10	2	INJURY	1	2	1	1	02, 07, YY	17M83013152	RIGHT ANGLE			
6	38188590	11/10/2019	18:45	1	INJURY	5	1	1	1	08, YY		GUIDE RAIL	Vehicle #1 was traveling South bound on Museum Village Rd and collided into the guard rail, and rode the rail for approximately 200 feet. Vehicle #1 then rolled over and landed in a pond/swamp.		

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
7	37797814	3/18/2019	09:11	2	INJURY	1	5	1	1	07, XX	17M83013152	LEFT TURN (AGAINST OTHER CAR)	Vehicle #1 facing southbound on Museum Village Road, stopped at the Museum Village Road stop sign, waiting to cross over Route 17m intersection. Vehicle #1 entered the intersection of Route 17m, failing to yield the right of way to vehicle #2 that was traveling westbound on Route 17m, causing the collision. Operator of vehicle #2 stated that the operator of vehicle #1 drove to the right of a truck that was stopped waiting to make a left turn from Museum Village Road onto Route 17m. Operator of vehicle #2 stated that vehicle #1 pulled out in front of her vehicle. Operator of vehicle #1 stated that he looked left and then right and the roadway was clear prior to him entering the intersection. Operator of vehicle #1 stated that vehicle #2 was traveling fast.
8	39213436	1/17/2022	12:35	1	INJURY	1	5	5	2	26, 66	17M83013152	SNOW EMBANKMENT	Unit 1 traveling Westbound on Route 17M approaching Museum Village Road. An uninvolved plow truck entered the roadway from a private driveway causing Unit 1 to swerve to avoid a collision. Unit 1 drove off the right side of the roadway and into a snow embankment before coming to rest front end down and nearly perpendicular to the ground. Operator of Unit 1 stated that he saw the plow but could not stop in time due to the road conditions. Witness stated that the plow truck was over the double yellow line and nearly blocking the whole roadway. Operator of plow truck issued citation for VTL 1143.
9	39228121	2/8/2022	12:09	2	PDO	1	5	2	2	07, YY	17M83013152	RIGHT ANGLE	Vehicle #1 was stopped at the Museum Village Road stop sign facing south, intersection of Route 17m. Vehicle #2 traveling westbound on Route 17m. Vehicle #1 made a left turn onto Route 17m crossing into the path of Vehicle #2, causing the collision. Operator of Vehicle #1 stated that he was stopped on Museum Village Road and made a left turn on Route 17m. Operator of Vehicle #1 stated that he looked and it was clear. Operator of Vehicle #1 stated he did not see Vehicle #2 until the last second. Operator of Vehicle #2 stated that she was traveling westbound on Route 17m when Vehicle #1 pulled out in front of her vehicle.
10	37878021	5/6/2019	15:47	2	INJURY	1	1	1	2	07, YY	17M83013152	RIGHT ANGLE	V2 was heading East on State Route 17m in the Town of Blooming Grove. V1 was heading NorthEast on Museum Village Road. V1 failed to notice V2 driving down State Route 17m and subsequently struck V2.
11	37920893	5/19/2019	18:58	2	INJURY	1	5	1	1	07, YY	17M83013152	LEFT TURN (AGAINST OTHER CAR)	
12	38402991	3/11/2020	23:47	1	NR	5	1	1	1	61, YY	17M83013152	DEER	Vehicle 1 traveling west on route 17m collided with a deer crossing the road.

## Accident Summary Sheet

Location:	NYS Route 208 and Museum Village	City:	South Blooming Grove
Period Covered:	3/1/2019-2/28/2022	County:	Orange
Date:	4/14/2022		

Time of Day		Weather			
#	%	#	%		
0600-1000	1	7	Clear	12	80
1000-1600	9	60	Cloudy	2	13
1600-1900	2	13	Rain/Snow	1	7
1900-2400	3	20	Sleet/Hail/Freezing Rain	0	0
2400-0600	0	0	Fog/Smog/Smoke	0	0
Uknown	0	0	Other/Unknown	0	0
Total	15	100.00%	Total	15	100.00%
Light Condition		Time of Year			
#	%	#	%		
Daylight	11	73	Winter (Dec-Feb)	4	27
Dawn	0	0	Spring (Mar-May)	3	20
Dusk	0	0	Summer (Jun-Aug)	4	27
Dark Lighted	2	13	Fall (Sep-Nov)	4	27
Dark Unlighted	2	13	Total	15	100.00%
Unknown	0	0			
Total	15	100.00%			
Accident Type		Roadway Characteristics			
#	%	#	%		
Overtaking	0	0	Straight & Level	8	53
Rear End	4	27	Straight & Grade	3	20
Right Angle	2	13	Straight & Hillcrest	0	0
Left Turn	7	47	Curve & Level	1	7
Sideswipe	0	0	Curve & Grade	3	20
Run Off Road	0	0	Curve & Hillcrest	0	0
Fixed Object	0	0	Unknown	0	0
Pedestrian	0	0	Total	15	100.00%
Bicycle	0	0			
Animal	0	0			
Right Turn	0	0			
Head On	0	0			
Other	2	13			
Total	15	100.00%			
Accident Severity		Roadway Surface Condition			
#	%	#	%		
Fatal	0	0	Dry	14	93
Serious Injury	7	47	Wet	1	7
Other Injury	0	0	Muddy	0	0
Prop damage Only	8	53	Snow/Ice	0	0
Unknown	0	0	Slush	0	0
Total	15	100.00%	Unknown	0	0
			Total	15	100.00%

## DETAILS OF ACCIDENT HISTORY FOR LOCATION (AS SHOWN ON CRASH DIAGRAM)

## DIAGRAM SHEET

STUDY NO. 120-269 P.I.N.. INVENTORY NO.			ROUTE NO. or STREET NAME NYS Route 208										COUNTY Orange MUNICIPALITY Blooming Grove BY LSC DATE 4/14/2022					
			AT INTERSECTION WITH / OR BETWEEN Museum Village Road															
NO. OF MONTHS 36		LIGHT CONDITIONS (LC)				ROADWAY CHARACTER (RC)					ROADWAY SURFACE CONDITION (RSC)		WEATHER (WEA)					
Begin Date: 3/01/2019 End Date: 2/28/2022			1. Daylight 2. Dawn 3. Dusk 4. Dark Road Lighted 5. Dark Road Unlighted				1. Straight & Level 2. Straight & Grade 3. Straight at Hillcrest 4. Curve & Level 5. Curve & Grade 6. Curve at Hillcrest					1. Dry 2. Wet 3. Muddy 4. Snow/Ice 5. Slush 10. Other		1. Clear 2. Cloudy 3. Rain 4. Snow 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke 10. Other				
NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION					
1	38469816	7/5/2020	11:45	2	PDO	1	5	1	1	07, 18, YY	208 83011013	LEFT TURN (AGAINST OTHER CAR)	Vehicle #1 traveling East on Museum Village Road stopped at the posted stop sign and proceeded into the intersection of Route 208 while attempting to make a left turn onto Route 208 North failing to yield the right of way to oncoming South bound traffic on Route 208. Vehicle #2 traveling South on Route 208 collided with Vehicle #1 while Vehicle #1 was turning.					
2	38764097	2/24/2021	10:19	2	INJURY	1	1	1	1	04, 09, YY	208 83011013	REAR END	Vehicle 1 was traveling northbound on State Route 208 behind vehicle 2 also traveling northbound. Vehicle 1 collided with the rear of vehicle 2. Operator of vehicle 1 stated she was not paying attention and did not see vehicle 2 slow down prior to her colliding with the rear of vehicle 2. Operator of vehicle 2 stated she slowed down for a vehicle in front of her making a left turn onto Museum Village Road prior to being hit in the rear by vehicle 1.					
3	38570992	9/27/2020	23:13	2	PDO	4	1	2	3	07, YY	208 83011013	LEFT TURN (WITH OTHER CAR)	V1 was traveling East on Museum Village Road. V2 was traveling South on Route 208. V1 entered Route 208, attempting a left turn, failing to yield for V2 resulting in a collision. Operator of V1 states that she observed a single light approaching from the left, but felt that it was far away. She further states that when she entered the roadway V2 was right there. Operator of V2 states that she was driving home and V1 just pulled out in front of her.					
4	38943318	7/23/2021	15:40	2	PDO	1	2	1	1	07, YY	208 83011013	LEFT TURN (AGAINST OTHER CAR)	Vehicle 2 was traveling South on Route 208. Vehicle 1 was on Museum Village Rd facing east at the stop sign at the intersection of Route 208 and Museum Village Rd. Vehicle 1, failing to yield the right way, attempted to perform a left hand turn onto Route 208, colliding with Vehicle 2. Operator of Vehicle 1 stated that the other car stopped to let him go so he thought it was safe. Operator of Vehicle 2 stated that he was just driving straight when the other car pulled out.					
5	39149653	12/7/2021	16:55	2	PDO	5	5	1	1	07, YY	208 83011013	RIGHT ANGLE						

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
6	38635617	10/28/2020	20:07	2	INJURY	5	1	1	1	09, 26, YY	208 83011013	REAR END	Vehicle 1 traveling Northbound on route 208 behind Vehicle 2. Vehicle 2 braked aggressively for an uninvolved motorist. Vehicle 1 struck Vehicle 2 in the drivers side rear bumper. Vehicle 1 coming to rest on Museum Village Rd, Vehicle 2 coming to rest on Route 208. Operator of Vehicle 1 states that another car, a silver SUV, slammed on the brakes all of a sudden to make the left onto Museum Village Rd. States further that he attempted to stop and steered to the left to avoid the collision but was unable to. Operator of Vehicle 2 states that a silver SUV braked suddenly and aggressively to turn onto Museum Village Rd. She was then struck by the other car. Witness states that a silver SUV slammed on its brakes and suddenly turned onto Museum Village Rd, causing Vehicle 1 to collide with Vehicle 2
7	38697821	11/24/2020	09:19	2	PDO	1	1	1	2	09, YY	208 83011013	REAR END	Vehicle #1 traveling northbound on Route 208 collided with the rear end of Vehicle #2. Operator of Vehicle #1 stated that he saw Vehicle #2 stopped, but was unable to stop in time and collided with the rear end. Operator of Vehicle #2 stated that he was stopped in traffic when he was struck from behind by Vehicle #1.
8	38315799	1/22/2020	15:40	2	INJURY	1	1	1	1	07, YY	208 83011013	LEFT TURN (AGAINST OTHER CAR)	Vehicle #1 was traveling East bound on Museum Village RD and attempted to make a left hand turn onto State Route 208, North bound. Vehicle #1 failed to yield the right of way to vehicle #2, who was traveling South bound on State Route 208. By doing so vehicle #2 collided into vehicle #1.
9	38970696	7/18/2021	15:09	2	PDO	1	1	1	1	07, YY	208 83011013	LEFT TURN (AGAINST OTHER CAR)	V2 was traveling southbound on Route 208 and approaching the intersection of Museum Village Road. V1 was traveling northbound on Route 208 and attempted to make a left turn onto Museum Village Road. V1 made the left turn directly in front of V2, causing the collision. Driver of V1 states he was making a left turn onto museum village road and V2 did not slow down for him to turn. Driver of V2 states he was driving southbound on route 208 when V1 made a turn right in front of him.
10	38221846	11/29/2019	13:03	2	PDO	1	2	1	1	18, 69, YY	208 83011013	RIGHT ANGLE	Vehicle #1 traveling east on Museum Village Rd and stopped at the stop sign. Vehicle #2 traveling south on Route 208. Vehicle #1 collided into vehicle #2. Operator of vehicle #1 stated traffic was backed up in both directions on Route 208. He stated he was motioned by the operator of an uninvolved vehicle that stopped in the northbound lane of Route 208 and waved him on to Route 208. Operator of vehicle #1 stated he was inching his way onto Rout 208 to go north. He was looking to his right and collided into vehicle #2. Operator of vehicle #1 stated that vehicle #2 drove in front of him going around him not yielding. Operator of vehicle #2 stated he was traveling south on Route 208 and vehicle #1 collided into his vehicle.
11	37922453	5/31/2019	13:58	2	INJURY	1	1	1	1	07, 21, YY	208 83011013	LEFT TURN (AGAINST OTHER CAR)	
12	38478785	7/12/2020	17:00	2	INJURY	1	5	1	1	09, YY	208 83011013	REAR END	Vehicle 1 traveling North on Route 208 behind Vehicle 2. Vehicle 2 was slowing in the roadway for an uninvolved vehicle performing a left hand turn onto Museum Village Rd. Vehicle 1 struck Vehicle 2 in the rear.

NO	CASE	DATE	TIME	# OF VEH	SEV	LC	RC	RSC	WEA	CONTRIB FACTORS	REF MKR	ACC TYPE	DESCRIPTION
13	38867533	5/12/2021	14:55	2	PDO	1	4	1	1	07, YY	208 83011013	LEFT TURN (AGAINST OTHER CAR)	Vehicle 2 was traveling South on Route 208. Vehicle 1 was stopped at the stop sign at the intersection of Route 208 and Museum Village Rd facing West. Vehicle 1, failing to yield the right of way, pulled out in front of Vehicle 2. Vehicle 2 collided with the front drivers side of Vehicle 1. Witness states that the Black car made a left turn onto Route 208 and the White truck was not able to stop in time.
14	38423404	5/15/2020	13:55	4	INJURY	1	1	1	1	09, YY	208 83011013	OTHER	Vehicles 1, 2, 3, and 4 traveling northbound on Route 208. Vehicle 4 slowed to a stop to make lefthand turn onto Museum Village Rd. Vehicle 3 and vehicle 2 slowed to a stop behind vehicle 4. Vehicle 1 failed to stop, colliding with the rear of vehicle 2, causing vehicle 2 to then collide with the rear of vehicle 3, causing vehicle 3 to then collide with the rear of vehicle 4. Operator of vehicle 4 stated that he was using his lefthand turn signal, and was slowing to make a lefthand turn onto Museum Village Rd. when he was rear ended. Operator of vehicle 3 stated that she was slowing down because the vehicle in front of her was making a lefthand turn, when she was rear ended. Operator of vehicle 2 stated that she was stopped in traffic for five seconds when she observed in her rear view mirror that the vehicle behind her was not slowing, and watched as it collided with her. Operator of vehicle 1 stated that the vehicle in front of him stopped suddenly without warning, and he could not stop quickly enough to avoid the collision. Witness 1: stated that he was traveling southbound on Route 208 when he observed the vehicle 4 slowing to make a lefthand turn onto Museum Village Rd. Michael stated that he observed vehicle 1 collide with the rear of vehicle 2 at a high rate of speed, which in turn caused vehicle 2 to collide with vehicle 3, and vehicle 3 to collide with vehicle 4. The operator and passenger of vehicle 2 were both transported to ORMC in BGVCAC 497 for complaints of neck pain. The operator of vehicle 3 was transported to ORMC in BGVCAC 498 for complaint of neck pain as well.
15	39191785	1/6/2022	20:34	3	INJURY	4	2	1	2	02, 07, 17, YY	208 83011013	OTHER	Unit 1 was traveling eastbound on Museum Village Road approaching State Route 208. Unit 1 while attempting to make a left hand turn onto State Route 208, in a northerly direction, failed to stop at the posted stop sign. Unit 1 drove into the path and collided with Unit 2 which was traveling north on State Route 208. Unit 2 after the initial collision continued in a northerly direction and collided with a tree before coming to rest off the right side of the roadway. Unit 1 after the initial collision continued in a northerly direction eventually crossing into the southbound lane and collided with Unit 3 which was traveling southbound on State Route 208. Both Unit 1 and Unit 3 came to rest facing south. Operator of unit 3 stated that he observed unit 1 fail to stop at the stop sign at the intersection and attempted to make a left hand turn and collide with unit 2. Operator of unit 2 appeared to be under the influence of alcohol at the time of the collision. Operator of unit 1 was unable to communicate thoroughly due to his injuries. Note: All occupants were transported by BGVCAC to Garnet Health Hospital.

**ATTACHMENT B**  
**CORRESPONDENCE WITH SCHOOL DISTRICTS**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK

## **Starke Hipp**

---

**From:** Matthew Kuhl <mkuhl@studentbusco.com>  
**Sent:** Monday, September 25, 2023 12:12 PM  
**To:** Starke Hipp  
**Cc:** Frank Filiciotto  
**Subject:** Re: 120-269 Craigville Rd; Request for School Bus Route Mapping - CUFSD

Understood, Starke.

As my colleague had stated, unfortunately, our route maps cannot be shared due to privacy concerns and for the security of the children and communities we service.

Thank You and have a good day

---

**From:** Starke Hipp <shipp@cmellp.com>  
**Sent:** Monday, September 25, 2023 11:49 AM  
**To:** Matthew Kuhl <mkuhl@studentbusco.com>  
**Cc:** Frank Filiciotto <ffiliciotto@cmellp.com>  
**Subject:** RE: 120-269 Craigville Rd; Request for School Bus Route Mapping - CUFSD

Hello Matt,

Sorry, I should have clarified that our study area includes parts of Chester, which is why I reached out. Specifically, our study includes NYS Route 17M from the eastern edge of Chester up to NYS Route 94 (a.k.a. Summerville Way/Academy Avenue) as well as the portion of NYS Route 94 (a.k.a. Summerville Way) between NYS Route 17M and Nucifora Boulevard/Lowes Driveway.

Thanks,

**Starke W. Hipp, PE**  
NY,NC  
Project Engineer

**Creighton Manning** | O: 914.800.9205 | C: 910.616.5608 | [www.cmellp.com](http://www.cmellp.com)

---

**From:** Matthew Kuhl <mkuhl@studentbusco.com>  
**Sent:** Monday, September 25, 2023 11:45 AM  
**To:** Starke Hipp <shipp@cmellp.com>  
**Cc:** Frank Filiciotto <ffiliciotto@cmellp.com>  
**Subject:** Re: 120-269 Craigville Rd; Request for School Bus Route Mapping - CUFSD

Good Morning Starke-

Our company does not service the Town of Blooming Grove. For that reason, I do not believe our route maps would be applicable to your study either way.

Matt

Sent from iPhone

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**From:** Starke Hipp <[shipp@cmellp.com](mailto:shipp@cmellp.com)>  
**Sent:** Monday, September 25, 2023 11:27:30 AM  
**To:** Matthew Kuhl <[mkuhl@studentbusco.com](mailto:mkuhl@studentbusco.com)>  
**Cc:** Frank Filiciotto <[ffiliciotto@cmellp.com](mailto:ffiliciotto@cmellp.com)>  
**Subject:** 120-269 Craigville Rd; Request for School Bus Route Mapping - CUFSD

Hello Matt

Earlier today I spoke with your colleague regarding a request for school bus route mapping. For background, my firm is preparing a traffic impact study for a project in Blooming Grove. That study is following a scope set forth by the Town of Blooming Grove Planning Board. The scope includes that we provide school bus route mapping within the study area. When I spoke with your colleague earlier today, they explained that your company will not be able to provide this information for confidentiality reasons. I fully understand this reasoning and I'm sure the Planning Board will feel the same way. That being said, I would appreciate it if you could respond here confirming that the requested information cannot be provided. We will include that confirmation in our records, so that Planning Board knows we attempted to adhere to their scope.

I appreciate you and your colleague for taking the time to help in progressing our study. If you have any questions, please do not hesitate to reach me here or via my office line listed below in the signature.

Best regards,

**Starke W. Hipp, PE**

NY,NC

Project Engineer

direct 914.800.9205  
office 914.800.9201  
cell 910.616.5608  
email [shipp@cmellp.com](mailto:shipp@cmellp.com)  
web [www.cmellp.com](http://www.cmellp.com)



145 Main Street, 3<sup>rd</sup> Floor | Ossining | New York | 10562

## **Starke Hipp**

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**From:** Perez, Raphy <rperez@wcsdk12.org>  
**Sent:** Tuesday, September 26, 2023 6:04 AM  
**To:** Starke Hipp  
**Cc:** Frank Filiciotto; jedwards@wcsdk12.org  
**Subject:** Re: 120-269 Craigville Rd; Request for School Bus Route Mapping - WCSD  
**Attachments:** image001.jpg

Good morning,

Jessica is correct. The information would include personal information that we would not be able to share. If you need anything further please let me know.

Thank you,  
Ralph Perez

On Mon, Sep 25, 2023 at 11:14 AM Starke Hipp <[shipp@cmellp.com](mailto:shipp@cmellp.com)> wrote:

**[EXTERNAL EMAIL]** This email originated outside of Washingtonville CSD. **DO NOT CLICK** links or attachments unless you recognize the sender and know the content is safe.

Hello Ralph,

Earlier today I spoke with your colleague, Jessica, regarding a request for school bus route mapping. For background, my firm is preparing a traffic impact study for a project in Blooming Grove. That study is following a scope set forth by the Town of Blooming Grove Planning Board. The scope includes that we provide school bus route mapping within the study area. When I spoke with Jessica earlier today, she explained that the district will not be able to provide this information for confidentiality reasons. I fully understand this reasoning and I'm sure the Planning Board will feel the same way. That being said, I would appreciate it if you could respond here confirming that the requested information cannot be provided. We will include that confirmation in our records, so that Planning Board knows we attempted to adhere to their scope.

I appreciate you and your colleague for taking the time to help in progressing our study. If you have any questions, please do not hesitate to reach me here or via my office line listed below in the signature.

Best regards,

**Starke W. Hipp, PE**

NY,NC

Project Engineer

## **Josh Koh**

---

**From:** Josh Koh  
**Sent:** Monday, October 2, 2023 11:17 AM  
**To:** Josh Koh  
**Subject:** FW: Roads buses travel on

**From:** Jessie Lazar <[jlazar@mw.k12.ny.us](mailto:jlazar@mw.k12.ny.us)>  
**Sent:** Thursday, September 28, 2023 9:20 AM  
**To:** Starke Hipp <[shipp@cmelip.com](mailto:shipp@cmelip.com)>  
**Subject:** Roads buses travel on

Good morning, I was able to get the information you are requesting. We travel on these roads between the hours of 5:45-9:15am - 1:15 6:00pm  
Craigville Rd between 17m & Oxford depo approximately 15 buses  
Museum Village Rd between 17m & Rt 208 approximately 20 buses  
Route 17m between Craigville Rd & Museum Village rd approximately 25 buses.  
If you have any further questions Please let me know.

--  
Warm regards,

Jessie Lazar  
Assistant Director of Transportation  
Monroe-Woodbury CSD  
845 460-6010



**ATTACHMENT C**  
**RAW ATR AND TMC DATA**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M  
**City:** Chester  
**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001  
**Date:** 3/10/2022

## Data - Total

NS/EW Streets:	Craigville Rd				Craigville Rd				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	0	0	0	5	0	2	0	2	16	0	0	0	21	4	0	50
7:15 AM	0	0	0	0	4	0	2	0	1	35	0	0	0	29	3	0	74
7:30 AM	0	0	0	0	9	0	3	0	7	20	0	0	0	32	4	0	75
7:45 AM	0	0	0	0	7	0	5	0	4	26	0	0	0	40	5	0	87
8:00 AM	0	0	0	0	9	0	6	0	3	21	0	0	0	33	4	0	76
8:15 AM	0	0	0	0	9	0	5	0	3	26	0	0	0	45	7	0	95
8:30 AM	0	0	0	0	8	0	1	0	0	27	0	0	0	45	8	0	89
8:45 AM	0	0	0	0	9	0	7	0	3	43	0	0	0	40	2	0	104
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	0	0	0	60	0	31	0	23	214	0	0	0	285	37	0	650
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>				35	0	19	0	9	117	0	0	0	163	21	0	TOTAL
<b>PEAK HR VOL :</b>	0	0	0	0		0.972	0.000	0.679	0.000	0.750	0.680	0.000	0.000	0.000	0.906	0.656	0.000
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.844				0.685				0.868				0.875

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0	0	0	0	0	1	0	0	0	1	0	0	0	1	0	0	TOTAL
NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU		
4:00 PM	0	0	0	0	15	0	5	0	0	29	0	0	0	62	15	0	126
4:15 PM	0	0	0	0	14	0	2	0	3	31	0	0	0	62	9	0	121
4:30 PM	0	0	0	0	8	0	4	0	1	20	0	0	0	42	19	0	94
4:45 PM	0	0	0	0	4	0	3	0	2	33	0	0	0	80	20	0	142
5:00 PM	0	0	0	0	10	0	19	0	1	29	0	0	0	78	10	0	147
5:15 PM	0	0	0	0	4	0	15	0	2	40	0	0	0	72	11	0	144
5:30 PM	0	0	0	0	6	0	4	0	2	25	0	0	0	57	8	0	102
5:45 PM	0	0	0	0	8	0	7	0	1	25	0	0	0	56	4	0	101
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	0	0	0	69	0	59	0	12	232	0	0	0	509	96	0	977
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>				24	0	41	0	7	127	0	0	0	287	49	0	TOTAL
<b>PEAK HR VOL :</b>	0	0	0	0		0.600	0.000	0.539	0.000	0.875	0.794	0.000	0.000	0.000	0.897	0.613	0.000
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.560				0.798				0.840				0.910

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Craigville Rd & NYS Rte 17M  
**City:** Chester  
**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001  
**Date:** 3/10/2022

## Data - Cars

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M  
**City:** Chester  
**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001  
**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	Craigville Rd				Craigville Rd				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	2	1	0	4
7:15 AM	0	0	0	0	1	0	0	0	0	3	0	0	0	2	0	0	6
7:30 AM	0	0	0	0	0	0	0	0	1	3	0	0	0	4	1	0	9
7:45 AM	0	0	0	0	1	0	0	0	0	1	0	0	0	2	1	0	5
8:00 AM	0	0	0	0	0	0	1	0	0	3	0	0	0	0	0	0	4
8:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	2	1	0	4
8:30 AM	0	0	0	0	1	0	1	0	0	2	0	0	0	8	2	0	14
8:45 AM	0	0	0	0	2	0	0	0	0	0	0	0	0	4	0	0	6
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	7	0	2	0	1	12	0	0	0	24	6	0	52
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>				4	0	2	0	0	5	0	0	0	14	3	0	<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0		0.500	0.000	0.500	0.000	0.000	0.417	0.000	0.000	0.000	0.438	0.375	0.000
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.750				0.417				0.425				0.500
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0	0	0	0	0	1	0	0	0	1	0	0	0	1	0	0	TOTAL
4:00 PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:15 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	1	0	0	3
4:30 PM	0	0	0	0	2	0	0	0	0	1	0	0	0	2	1	0	6
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	2	0	4
5:00 PM	0	0	0	0	1	0	0	0	0	1	0	0	0	4	0	0	6
5:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	2	0	0	3
5:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	2	0	0	3
5:45 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	1	0	0	3
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	5	0	0	0	0	7	0	0	0	17	4	0	33
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>				1	0	0	0	0	4	0	0	0	11	1	0	<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0		0.250	0.000	0.000	0.000	0.000	1.000	0.000	0.000	0.000	0.688	0.250	0.000
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.250				1.000				0.750				0.708

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M  
**City:** Chester  
**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001  
**Date:** 3/10/2022

Data - Bikes

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL	
	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU		
4:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	<b>TOTAL 0</b>

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M  
**City:** Chester

**Project ID:** 22-380005-001  
**Date:** 3/10/2022

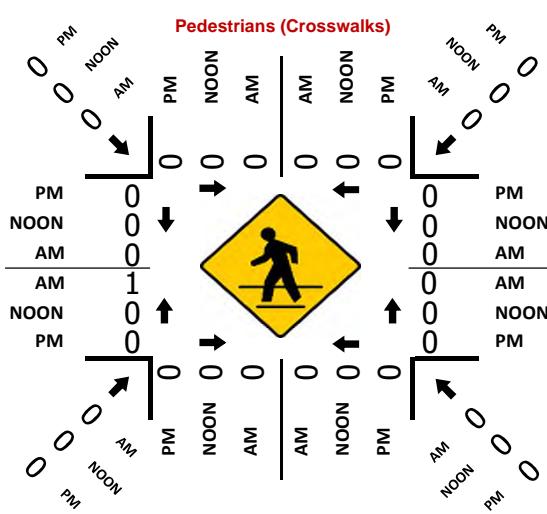
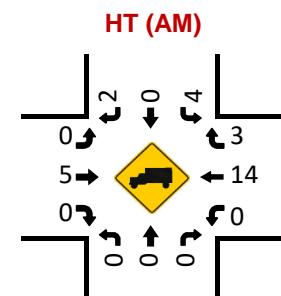
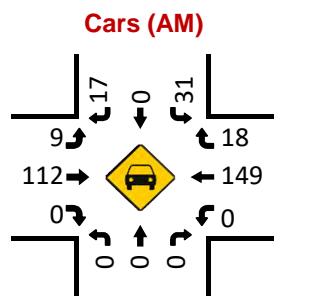
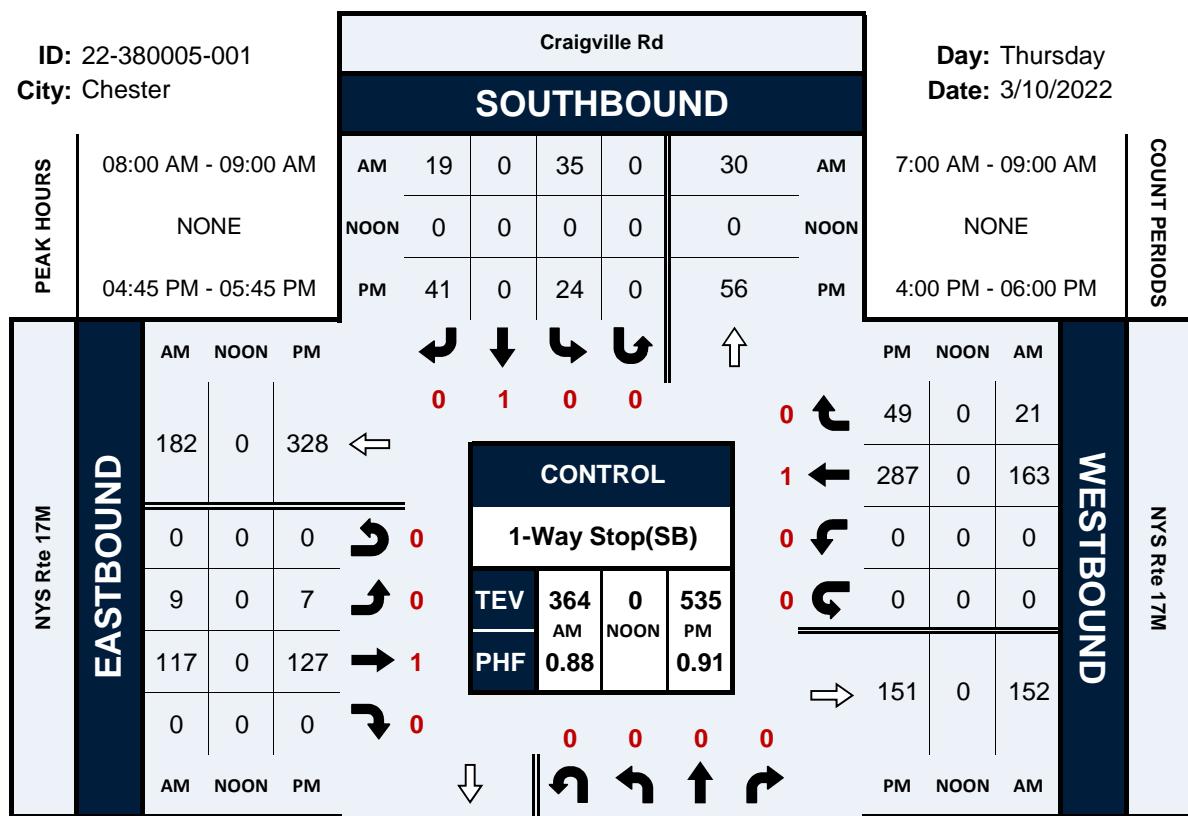
### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Craigville Rd		Craigville Rd		NYS Rte 17M		NYS Rte 17M		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	1	0	1
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	1	0	1
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	1	0	1
<b>PEAK HR FACTOR :</b>							0.250	0.250	0.250

<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									0

**Craigville Rd & NYS Rte 17M****Peak Hour Turning Movement Count**

**ID:** 22-380005-001  
**City:** Chester



National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd

**City:** Chester

**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002

Date: 3/10/2022

## Data - Total

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
4:00 PM	0 NL	18 NT	3 NR	0 NU	17 SL	13 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	4 WL	0 WT	35 WR	0 WU	90
4:15 PM	0 NL	14 NT	6 NR	0 NU	18 SL	15 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	2 WL	0 WT	30 WR	0 WU	85
4:30 PM	0 NL	23 NT	0 NR	0 NU	29 SL	8 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	2 WL	0 WT	40 WR	0 WU	102
4:45 PM	0 NL	26 NT	0 NR	0 NU	22 SL	6 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	3 WL	0 WT	38 WR	0 WU	95
5:00 PM	0 NL	16 NT	2 NR	0 NU	22 SL	8 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	20 WL	0 WT	60 WR	0 WU	128
5:15 PM	0 NL	21 NT	2 NR	0 NU	19 SL	4 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	12 WL	0 WT	38 WR	0 WU	96
5:30 PM	0 NL	10 NT	2 NR	0 NU	17 SL	7 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	4 WL	0 WT	39 WR	0 WU	79
5:45 PM	0 NL	9 NT	2 NR	0 NU	21 SL	7 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	9 WL	0 WT	24 WR	0 WU	72
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	137	17	0	165	68	0	0	0	0	0	0	56	0	304	0	747
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>				70.82%	29.18%	0.00%	0.00%	0	0	0	0	15.56%	0.00%	84.44%	0.00%	TOTAL
<b>PEAK HR VOL :</b>	0	86	4	0	92	26	0	0	0	0	0	0	37	0	176	0	421
<b>PEAK HR FACTOR :</b>	0.000	0.827	0.500	0.000	0.793	0.813	0.000	0.000	0.000	0.000	0.000	0.000	0.463	0.000	0.733	0.000	0.822
		0.865				0.797									0.666		

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd

**City:** Chester

**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002

Date: 3/10/2022

## Data - Cars

NS/EW Streets:		Craigville Rd				Craigville Rd				Old Mansion Rd				Old Mansion Rd				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	0	3	2	0	30	4	0	0	0	0	0	0	0	0	5	0	44	
7:15 AM	0	2	1	0	29	4	0	0	0	0	0	0	1	0	9	0	46	
7:30 AM	0	5	8	0	34	8	0	0	0	0	0	0	1	0	9	0	65	
7:45 AM	0	5	8	0	33	6	0	0	0	0	0	0	1	0	11	0	64	
8:00 AM	0	4	7	0	26	10	0	0	0	0	0	0	1	0	9	0	57	
8:15 AM	0	9	2	0	41	8	0	0	0	0	0	0	1	0	7	0	68	
8:30 AM	0	7	3	0	35	4	0	0	0	0	0	0	0	0	12	0	61	
8:45 AM	0	5	0	1	31	8	0	0	0	0	0	0	4	0	16	0	65	
<b>TOTAL VOLUMES :</b>		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>		0	40	31	1	259	52	0	0	0	0	0	9	0	78	0	470	
<b>PEAK HR :</b>		<b>08:00 AM - 09:00 AM</b>																
<b>PEAK HR VOL :</b>		0	25	12	1	133	30	0	0	0	0	0	6	0	44	0	TOTAL 251	
<b>PEAK HR FACTOR :</b>		0.000	0.694	0.429	0.250	0.811	0.750	0.000	0.000	0.000	0.000	0.000	0.375	0.000	0.688	0.000	0.923	
		</																

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd  
**City:** Chester  
**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002  
**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	Craigville Rd				Craigville Rd				Old Mansion Rd				Old Mansion Rd										
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND										
AM	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL						
7:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	1	0	3						
7:15 AM	0	1	0	0	2	1	0	0	0	0	0	0	0	0	0	3	7						
7:30 AM	0	2	0	0	1	2	0	0	0	0	0	0	0	0	0	2	7						
7:45 AM	0	2	1	0	2	0	0	0	0	0	0	0	1	0	1	0	7						
8:00 AM	0	1	0	0	5	1	0	0	0	0	0	0	0	0	0	0	7						
8:15 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	1	3						
8:30 AM	0	2	0	0	4	4	0	0	0	0	0	0	0	0	0	3	0						
8:45 AM	0	2	1	0	4	1	0	0	0	0	0	0	0	0	0	3	11						
<b>TOTAL VOLUMES :</b>	NL 0	NT 12	NR 2	NU 0	SL 18	ST 11	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 1	WT 0	WR 14	WU 0	<b>TOTAL</b> 58						
<b>APPROACH %'s :</b>	0.00%	85.71%	14.29%	0.00%	62.07%	37.93%	0.00%	0.00%	0	0	0	0	6.67%	0.00%	93.33%	0.00%							
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>																<b>TOTAL</b>						
<b>PEAK HR VOL :</b>	0	6	1	0	13 0.650	7	0	0	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	7 0.583	0 0.654							
<b>PEAK HR FACTOR :</b>	0.000	0.750	0.250	0.000		0.438	0.000	0.000															
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND										
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL						
4:00 PM	0	0	0	0	1	2	0	0	0	0	0	0	0	0	5	0	8						
4:15 PM	0	1	0	0	2	1	0	0	0	0	0	0	1	0	1	0	6						
4:30 PM	0	3	0	0	6	1	0	0	0	0	0	0	0	0	0	0	10						
4:45 PM	0	1	0	0	1	1	0	0	0	0	0	0	0	0	3	0	6						
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1						
5:15 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2	0	4						
5:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1						
5:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1						
<b>TOTAL VOLUMES :</b>	NL 0	NT 5	NR 1	NU 0	SL 13	ST 5	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 1	WT 0	WR 12	WU 0	<b>TOTAL</b> 37						
<b>APPROACH %'s :</b>	0.00%	83.33%	16.67%	0.00%	72.22%	27.78%	0.00%	0.00%	0	0	0	0	7.69%	0.00%	92.31%	0.00%							
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>						
<b>PEAK HR VOL :</b>	0	4	0	0	9 0.375	2	0	0	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	6 0.500	0 0.500	21 0.525							
<b>PEAK HR FACTOR :</b>	0.000	0.333	0.000	0.000		0.500	0.000	0.000															

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd  
**City:** Chester  
**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002  
**Date:** 3/10/2022

Data - Bikes

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL	
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU		
4:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>																		

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd  
**City:** Chester

**Project ID:** 22-380005-002  
**Date:** 3/10/2022

### Data - Pedestrians (Crosswalks)

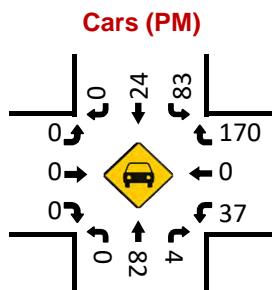
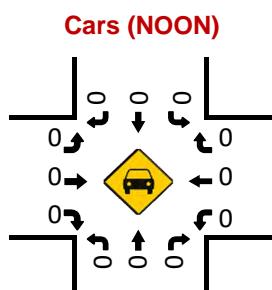
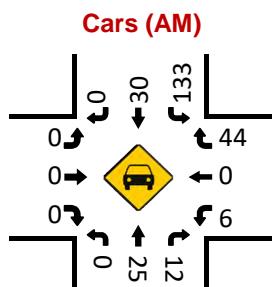
NS/EW Streets:	Craigville Rd		Craigville Rd		Old Mansion Rd		Old Mansion Rd		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

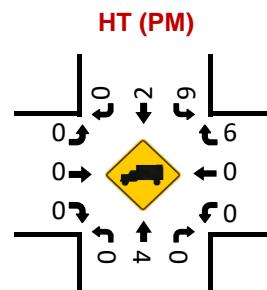
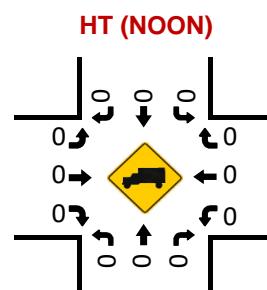
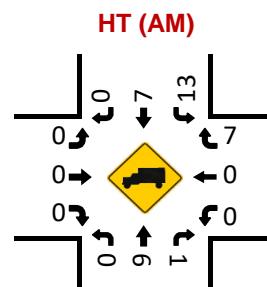
## **Craigville Rd & Old Mansion Rd**

# Peak Hour Turning Movement Count

**ID:** 22-380005-002  
**City:** Chester



The diagram illustrates the complex movement patterns of pedestrians across a crosswalk, represented by a central yellow diamond sign with a black walking figure. The surrounding grid shows arrows pointing in various directions (up, down, left, right) from each corner of the grid towards the central sign, representing the flow of pedestrian traffic throughout the week and day.



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-003

**Date:** 3/10/2022

## Data - Total

NS/EW Streets:	Craigville Rd				Craigville Rd				WB Exit 128 Off-Ramp				WB Exit 128 Off-Ramp				
<b>AM</b>	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				<b>TOTAL</b>
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	0	5	0	0	0	7	0	0	2	0	1	0	0	0	0	0	15
7:15 AM	0	3	0	0	0	4	0	0	0	0	1	0	0	0	0	0	8
7:30 AM	0	10	0	0	0	12	0	0	0	0	0	0	0	0	0	0	22
7:45 AM	0	9	0	0	0	10	0	0	0	0	3	0	0	0	0	0	22
8:00 AM	0	7	0	0	0	11	0	0	2	0	3	0	0	0	0	0	23
8:15 AM	0	7	0	0	0	9	0	0	2	0	0	0	0	0	0	0	18
8:30 AM	0	8	0	0	0	9	0	0	3	0	0	0	0	0	0	0	20
8:45 AM	0	5	0	0	0	14	0	0	2	0	2	0	0	0	0	0	23
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	54	0	0	0	76	0	0	11	0	10	0	0	0	0	0	151
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	33	0	0	0	42	0	0	4	0	6	0	0	0	0	0	85
<b>PEAK HR FACTOR :</b>	0.000	0.825	0.000	0.000	0.000	0.875	0.000	0.000	0.500	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.924
<b>PM</b>	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0	1	0	0	0	1	0	0	0	1	0	0	0	0	0	0	<b>TOTAL</b>
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	0	16	0	0	0	16	0	0	7	0	4	0	0	0	0	0	43
4:15 PM	0	12	0	0	0	19	0	0	4	0	2	0	0	0	0	0	37
4:30 PM	0	19	0	0	0	8	0	0	5	0	4	0	0	0	0	0	36
4:45 PM	0	23	0	0	0	9	0	0	5	0	0	0	0	0	0	0	37
5:00 PM	0	12	0	0	0	27	0	0	6	0	0	0	0	0	0	0	45
5:15 PM	0	13	0	0	0	16	0	0	7	0	3	0	0	0	0	0	39
5:30 PM	0	9	0	0	0	9	0	0	5	0	1	0	0	0	0	0	24
5:45 PM	0	6	0	0	0	14	0	0	3	0	1	0	0	0	0	0	24
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	110	0	0	0	118	0	0	42	0	15	0	0	0	0	0	285
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	67	0	0	0	60	0	0	23	0	7	0	0	0	0	0	157
<b>PEAK HR FACTOR :</b>	0.000	0.728	0.000	0.000	0.000	0.556	0.000	0.000	0.821	0.000	0.438	0.000	0.000	0.000	0.000	0.000	0.872

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

### **Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-003

Date: 3/10/2022

## Data - Cars

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	
4:00 PM	0	16	0	0	0	14	0	0	7	0	4	0	0	0	0	0	41
4:15 PM	0	11	0	0	0	17	0	0	4	0	2	0	0	0	0	0	34
4:30 PM	0	17	0	0	0	8	0	0	5	0	4	0	0	0	0	0	34
4:45 PM	0	22	0	0	0	9	0	0	5	0	0	0	0	0	0	0	36
5:00 PM	0	12	0	0	0	26	0	0	6	0	0	0	0	0	0	0	44
5:15 PM	0	13	0	0	0	16	0	0	7	0	3	0	0	0	0	0	39
5:30 PM	0	9	0	0	0	9	0	0	4	0	1	0	0	0	0	0	23
5:45 PM	0	6	0	0	0	14	0	0	3	0	1	0	0	0	0	0	24
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	106	0	0	0	113	0	0	41	0	15	0	0	0	0	0	275
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	0	64	0	0	0	59	0	0	23	0	7	0	0	0	0	0	153
PEAK HR FACTOR :	0.000	0.727	0.000	0.000	0.000	0.567	0.000	0.000	0.821	0.000	0.438	0.000	0.000	0.000	0.000	0.000	0.869

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

**Control:** 1-Way Stop(EB)

Project ID: 22-380005-003

Date: 3/10/2022

## Data - HT

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

**Control:** 1-Way Stop(EB)

Project ID: 22-380005-003

Date: 3/10/2022

Data - Bikes

NS/EW Streets:		Craigville Rd				Craigville Rd				WB Exit 128 Off-Ramp				WB Exit 128 Off-Ramp				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	<b>TOTAL 0</b>	

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL	
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU		
4:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	<b>TOTAL 0</b>

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & WB Exit 128 Off-Ramp      **Project ID:** 22-380005-003  
**City:** Chester      **Date:** 3/10/2022

## Data - Pedestrians (Crosswalks)

NS/EW Streets:	Craigville Rd		Craigville Rd		WB Exit 128 Off-Ramp		WB Exit 128 Off-Ramp		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	1	0	1
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	1	0	1
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	<b>0</b>
<b>PEAK HR FACTOR :</b>									

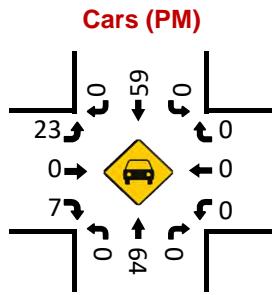
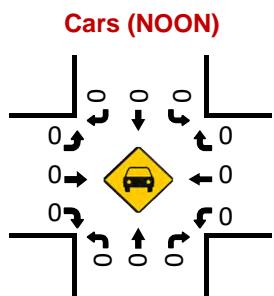
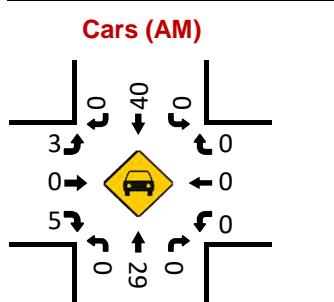
<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	<b>0</b>
<b>PEAK HR FACTOR :</b>									

# Craigville Rd & WB Exit 128 Off-Ramp

## Peak Hour Turning Movement Count

**ID:** 22-380005-003  
**City:** Chester

ID: 22-380005-003		Craigville Rd						Day: Thursday									
City: Chester		SOUTHBOUND						Date: 3/10/2022									
PEAK HOURS	07:30 AM - 08:30 AM			AM	0	42	0	0	37	AM	7:00 AM - 09:00 AM			COUNT PERIODS			
	NONE			NOON	0	0	0	0	0	NOON	NONE						
	04:30 PM - 05:30 PM			PM	0	60	0	0	90	PM	4:00 PM - 06:00 PM						
EASTBOUND	AM	NOON	PM														
	0	0	0														
	0	0	0														
	0	0	0														
	4	0	23														
	0	0	0														
	6	0	7														
WB Exit 128 Off-Ramp		CONTROL						WESTBOUND									
		1-Way Stop(EB)															
		TEV	85	0	157												
		AM	NOON	PM													
		PHF	0.92			0.87											



National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

### **Control: No Control**

Project ID: 22-380005-004

Date: 3/10/2022

## Data - Total

National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

### **Control: No Control**

**Project ID:** 22-380005-004

Date: 3/10/2022

## Data - Cars

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-004

**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	0	0	0	0	0	0	0	5	0	0	0	0	2	0	0	7
7:15 AM	0	0	0	0	0	0	0	0	5	3	0	0	0	2	0	0	10
7:30 AM	0	0	0	0	0	0	0	0	7	4	0	0	0	3	0	0	14
7:45 AM	0	0	0	0	0	0	0	0	9	2	0	0	0	3	0	0	14
8:00 AM	0	0	0	0	0	0	0	0	1	2	0	0	0	1	0	0	4
8:15 AM	0	0	0	0	0	0	0	0	4	1	0	0	0	2	0	0	7
8:30 AM	0	0	0	0	0	0	0	0	6	1	0	0	0	8	0	0	15
8:45 AM	0	0	0	0	0	0	0	0	5	0	0	0	0	5	0	0	10
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 42	ET 13	ER 0	EU 0	WL 0	WT 26	WR 0	WU 0	<b>TOTAL 81</b>
<b>APPROACH %'s :</b>																	
<b>PEAK HR :</b>	<b>07:15 AM - 08:15 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	22	11	0	0	0	9	0	0	<b>42</b>
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.611	0.688	0.000	0.000	0.000	0.750	0.000	0.750	0.750
<b>PM</b>	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 PM	0	0	0	0	0	0	0	0	4	0	0	0	0	0	1	0	5
4:15 PM	0	0	0	0	0	0	0	0	3	1	0	0	0	1	0	0	5
4:30 PM	0	0	0	0	0	0	0	0	2	0	0	0	0	3	0	0	5
4:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	3	0	0	4
5:00 PM	0	0	0	0	0	0	0	0	2	2	0	0	0	3	0	0	7
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
5:30 PM	0	0	0	0	0	0	0	0	3	1	0	0	0	1	0	0	5
5:45 PM	0	0	0	0	0	0	0	0	3	3	0	0	0	2	0	0	8
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 17	ET 8	ER 0	EU 0	WL 0	WT 16	WR 1	WU 0	<b>TOTAL 42</b>
<b>APPROACH %'s :</b>																	
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	5	4	0	0	0	10	0	0	<b>19</b>
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.417	0.500	0.000	0.000	0.000	0.833	0.000	0.833	0.679

National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

**Control:** No Control

Project ID: 22-380005-004

Date: 3/10/2022

Data - Bikes

NS/EW Streets:		NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M				NYS Rte 17M				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
		0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	TOTAL
7:00 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>		NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0
<b>PEAK HR :</b>		<b>07:15 AM - 08:15 AM</b>																<b>TOTAL 0</b>
<b>PEAK HR VOL :</b>		0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>
<b>PEAK HR FACTOR :</b>																		

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL	
	0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU		
4:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	<b>TOTAL 0</b>

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M WB

**City:** Chester

**Project ID:** 22-380005-004

**Date:** 3/10/2022

## Data - Pedestrians (Crosswalks)

NS/EW Streets:	NYS Rte 17M Exit 127 EB On-Ramp		NYS Rte 17M Exit 127 EB On-Ramp		NYS Rte 17M		NYS Rte 17M		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB 0	WB 0	EB 0	WB 0	NB 0	SB 0	NB 0	SB 0	<b>TOTAL 0</b>
<b>PEAK HR :</b>	<b>07:15 AM - 08:15 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0		0		0		0		<b>0</b>
<b>PEAK HR FACTOR :</b>									

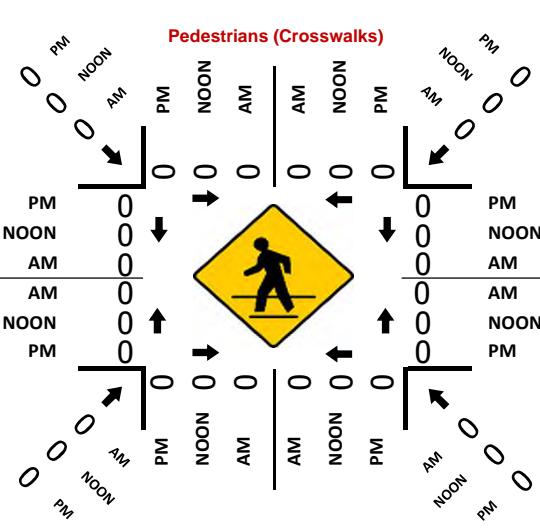
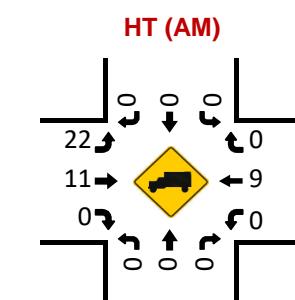
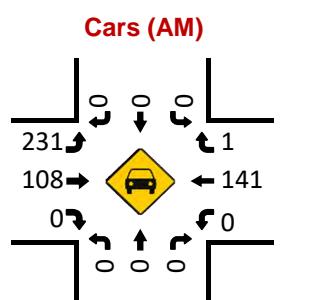
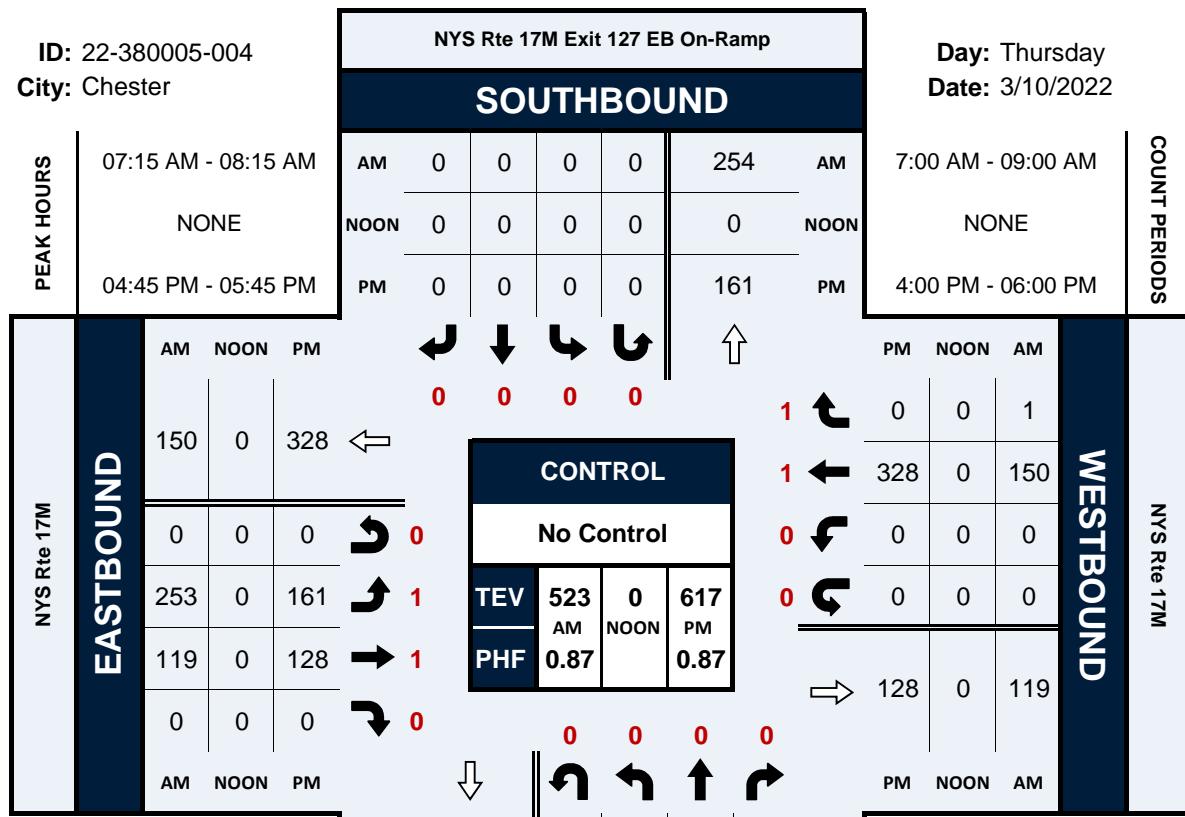
<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB 0	WB 0	EB 0	WB 0	NB 0	SB 0	NB 0	SB 0	<b>TOTAL 0</b>
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0		0		0		0		<b>0</b>
<b>PEAK HR FACTOR :</b>									

# NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

## Peak Hour Turning Movement Count

ID: 22-380005-004  
City: Chester

Day: Thursday  
Date: 3/10/2022



National Data & Surveying Services Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester

### **Control:** Signalized

**Project ID:** 22-380005-005

Date: 3/10/2022

## Data - Total

NS/EW Streets:		Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M			
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	65	0	58	0	1	21	16	0	8	35	43	0	1	25	0	0	273
7:15 AM	95	1	60	0	2	15	8	0	6	56	41	0	7	33	1	0	325
7:30 AM	87	1	38	0	3	19	11	0	0	44	63	0	2	28	0	0	296
7:45 AM	105	0	42	0	0	33	12	0	1	52	79	0	5	45	0	0	374
8:00 AM	95	1	39	0	3	19	13	0	3	39	57	0	7	31	0	0	307
8:15 AM	77	0	39	0	3	16	16	0	2	33	70	0	10	30	0	0	296
8:30 AM	88	3	43	0	0	19	8	0	5	46	71	0	7	39	0	0	329
8:45 AM	84	4	43	0	3	45	10	0	6	43	66	0	8	39	0	0	351
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	696	10	362	0	15	187	94	0	31	348	490	0	47	270	1	0	2551
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																TOTAL
<b>PEAK HR VOL :</b>	365	4	163	0	6	87	49	0	11	170	277	0	29	145	0	0	1306
<b>PEAK HR FACTOR :</b>	0.869	0.333	0.948	0.000	0.500	0.659	0.766	0.000	0.550	0.817	0.877	0.000	0.725	0.806	0.000	0.000	0.873
					0.905		0.789			0.867					0.870		

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
4:00 PM	94	1	37	0	2	46	31	0	8	25	84	0	10	57	1	0	396
4:15 PM	110	2	30	0	3	50	30	0	0	37	95	0	9	56	0	0	422
4:30 PM	94	1	33	0	0	49	36	0	3	32	140	0	8	45	0	0	441
4:45 PM	76	3	39	0	5	59	32	0	7	36	116	0	13	73	0	0	459
5:00 PM	91	3	48	0	0	42	29	0	4	47	96	0	13	80	0	0	453
5:15 PM	90	2	31	0	4	64	29	0	8	38	98	0	21	79	0	0	464
5:30 PM	92	5	25	0	1	55	27	0	6	40	99	0	14	47	0	0	411
5:45 PM	87	1	32	0	0	41	28	0	11	27	106	0	12	48	0	0	393
TOTAL VOLUMES :	NL 734	NT 18	NR 275	NU 0	SL 15	ST 406	SR 242	SU 0	EL 47	ET 282	ER 834	EU 0	WL 100	WT 485	WR 1	WU 0	TOTAL 3439
APPROACH %'s :	71.47%	1.75%	26.78%	0.00%	2.26%	61.24%	36.50%	0.00%	4.04%	24.25%	71.71%	0.00%	17.06%	82.76%	0.17%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	351	9	151	0	9	214	126	0	22	153	450	0	55	277	0	0	1817
PEAK HR FACTOR :	0.934	0.750	0.786	0.000	0.450	0.836	0.875	0.000	0.688	0.814	0.804	0.000	0.655	0.866	0.000	0.000	0.979
	0.900				0.899				0.893					0.830			

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-005

**Date:** 3/10/2022

## Data - Cars

NS/EW Streets:	Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
7:00 AM	62	0	53	0	1	16	10	0	6	33	37	0	1	21	0	0	240
7:15 AM	89	1	56	0	2	11	7	0	6	52	38	0	6	30	1	0	299
7:30 AM	83	1	33	0	3	14	9	0	0	38	59	0	2	26	0	0	268
7:45 AM	100	0	39	0	0	28	10	0	1	44	72	0	3	44	0	0	341
8:00 AM	90	0	37	0	3	17	10	0	2	38	50	0	7	31	0	0	285
8:15 AM	74	0	37	0	3	13	15	0	1	30	64	0	9	27	0	0	273
8:30 AM	82	3	40	0	0	15	8	0	5	45	70	0	6	35	0	0	309
8:45 AM	83	2	37	0	3	37	9	0	6	42	63	0	6	34	0	0	322
<b>TOTAL VOLUMES :</b>	NL 663	NT 7	NR 332	NU 0	SL 15	ST 151	SR 78	SU 0	EL 27	ET 322	ER 453	EU 0	WL 40	WT 248	WR 1	WU 0	TOTAL 2337
<b>APPROACH %'s:</b>	66.17%	0.70%	33.13%	0.00%	6.15%	61.89%	31.97%	0.00%	3.37%	40.15%	56.48%	0.00%	13.84%	85.81%	0.35%	0.00%	
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	346	3	153	0	6	73	43	0	9	157	256	0	25	137	0	0	1208
<b>PEAK HR FACTOR :</b>	0.865	0.250	0.956	0.000	0.500	0.652	0.717	0.000	0.450	0.872	0.889	0.000	0.694	0.778	0.000	0.000	0.886
	0.903				0.803				0.879				0.862				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
PM	0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
4:00 PM	90	1	34	0	2	43	30	0	8	25	79	0	10	54	1	0	377
4:15 PM	104	2	27	0	3	45	30	0	0	35	92	0	8	55	0	0	401
4:30 PM	88	1	31	0	0	45	35	0	3	31	134	0	8	43	0	0	419
4:45 PM	74	3	39	0	5	50	32	0	6	36	115	0	11	71	0	0	442
5:00 PM	88	3	45	0	0	39	28	0	4	43	95	0	13	75	0	0	433
5:15 PM	89	2	31	0	4	58	27	0	8	36	96	0	20	76	0	0	447
5:30 PM	87	4	23	0	1	54	27	0	6	39	99	0	13	47	0	0	400
5:45 PM	84	1	28	0	0	37	28	0	11	26	105	0	11	47	0	0	378
<b>TOTAL VOLUMES :</b>	NL 704	NT 17	NR 258	NU 0	SL 15	ST 371	SR 237	SU 0	EL 46	ET 271	ER 815	EU 0	WL 94	WT 468	WR 1	WU 0	TOTAL 3297
<b>APPROACH %'s:</b>	71.91%	1.74%	26.35%	0.00%	2.41%	59.55%	38.04%	0.00%	4.06%	23.94%	72.00%	0.00%	16.70%	83.13%	0.18%	0.00%	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	339	9	146	0	9	192	122	0	21	146	440	0	52	265	0	0	1741
<b>PEAK HR FACTOR :</b>	0.952	0.750	0.811	0.000	0.450	0.828	0.871	0.000	0.656	0.849	0.821	0.000	0.650	0.872	0.000	0.000	0.974
	0.908				0.907				0.903				0.826				

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester

#### **Control:** Signalized

**Project ID:** 22-380005-005

Date: 3/10/2022

## Data - HT

NS/EW Streets:		Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	3	0	5	0	0	5	6	0	2	2	6	0	0	4	0	0	0	33
7:15 AM	6	0	4	0	0	4	1	0	0	4	3	0	1	3	0	0	0	26
7:30 AM	4	0	5	0	0	5	2	0	0	6	4	0	0	2	0	0	0	28
7:45 AM	5	0	3	0	0	5	2	0	0	8	7	0	2	1	0	0	0	33
8:00 AM	5	1	2	0	0	2	3	0	1	1	7	0	0	0	0	0	0	22
8:15 AM	3	0	2	0	0	3	1	0	1	3	6	0	1	3	0	0	0	23
8:30 AM	6	0	3	0	0	4	0	0	0	1	1	0	1	4	0	0	0	20
8:45 AM	1	2	6	0	0	8	1	0	0	1	3	0	2	5	0	0	0	29
<b>TOTAL VOLUMES :</b>		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>		33	3	30	0	0	36	16	0	4	26	37	0	7	22	0	0	214
<b>PEAK HR :</b>		<b>07:45 AM - 08:45 AM</b>																TOTAL
<b>PEAK HR VOL :</b>		19	1	10	0	0	14	6	0	2	13	21	0	4	8	0	0	98
<b>PEAK HR FACTOR :</b>		0.792	0.250	0.833	0.000	0.000	0.700	0.500	0.000	0.500	0.406	0.750	0.000	0.500	0.500	0.000	0.000	0.742

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
4:00 PM	4 NL	0 NT	3 NR	0 NU	0 SL	3 ST	1 SR	0 SU	0 EL	0 ET	5 ER	0 EU	0 WL	3 WT	0 WR	0 WU	19
4:15 PM	6 NL	0 NT	3 NR	0 NU	0 SL	5 ST	0 SR	0 SU	0 EL	2 ET	3 ER	0 EU	1 WL	1 WT	0 WR	0 WU	21
4:30 PM	6 NL	0 NT	2 NR	0 NU	0 SL	4 ST	1 SR	0 SU	0 EL	1 ET	6 ER	0 EU	0 WL	2 WT	0 WR	0 WU	22
4:45 PM	2 NL	0 NT	0 NR	0 NU	0 SL	9 ST	0 SR	0 SU	1 EL	0 ET	1 ER	0 EU	2 WL	2 WT	0 WR	0 WU	17
5:00 PM	3 NL	0 NT	3 NR	0 NU	0 SL	3 ST	1 SR	0 SU	0 EL	4 ET	1 ER	0 EU	0 WL	5 WT	0 WR	0 WU	20
5:15 PM	1 NL	0 NT	0 NR	0 NU	0 SL	6 ST	2 SR	0 SU	0 EL	2 ET	2 ER	0 EU	1 WL	3 WT	0 WR	0 WU	17
5:30 PM	5 NL	1 NT	2 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	1 WL	0 WT	0 WR	0 WU	11
5:45 PM	3 NL	0 NT	4 NR	0 NU	0 SL	4 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	1 WL	1 WT	0 WR	0 WU	15
TOTAL VOLUMES :	NL 30	NT 1	NR 17	NU 0	SL 0	ST 35	SR 5	SU 0	EL 1	ET 11	ER 19	EU 0	WL 6	WT 17	WR 0	WU 0	TOTAL 142
APPROACH %'s :	62.50%	2.08%	35.42%	0.00%	0.00%	87.50%	12.50%	0.00%	3.23%	35.48%	61.29%	0.00%	26.09%	73.91%	0.00%	0.00%	
PEAK HR :	04:30 PM - 05:30 PM																TOTAL
PEAK HR VOL :	12 0.500	0 0.000	5 0.417	0 0.000	0 0.000	22 0.611	4 0.500	0 0.000	1 0.250	7 0.438	10 0.417	0 0.000	3 0.375	12 0.600	0 0.000	0 0.000	TOTAL 76
PEAK HR FACTOR :																	0.864

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-005

**Date:** 3/10/2022

## Data - Bikes

NS/EW Streets:	Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 0</b>	
<b>APPROACH %'s :</b>																		
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	0	1	1	0	0	1	0	0	0	1	1	0	0	1	0	0	TOTAL	
NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU			
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 0</b>	
<b>APPROACH %'s :</b>																		
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M  
**City:** Chester

**Project ID:** 22-380005-005  
**Date:** 3/10/2022

### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Kings Hwy/Lehigh Ave		Kings Hwy/Lehigh Ave		NYS Rte 17M		NYS Rte 17M		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

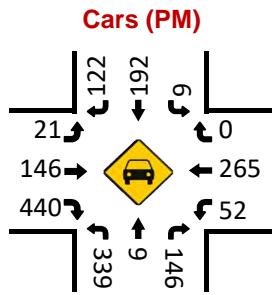
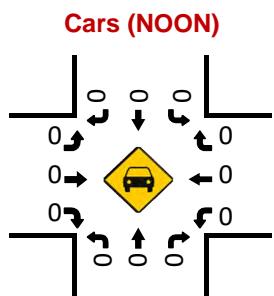
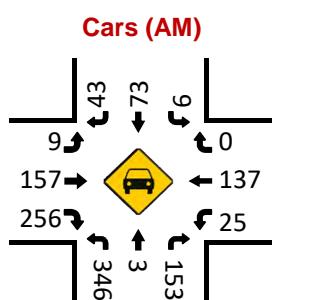
<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

# Kings Hwy/Lehigh Ave & NYS Rte 17M

## Peak Hour Turning Movement Count

**ID:** 22-380005-005  
**City:** Chester

PEAK HOURS			Kings Hwy/Lehigh Ave					COUNT PERIODS			
			SOUTHBOUND								
NYS Rte 17M	07:45 AM - 08:45 AM		AM	49	87	6	0	15	AM	7:00 AM - 09:00 AM	
	NONE		NOON	0	0	0	0	0	NOON	NONE	
	04:30 PM - 05:30 PM		PM	126	214	9	0	31	PM	4:00 PM - 06:00 PM	
EASTBOUND	AM	NOON	PM						PM	NOON	AM
	559	0	754		0	1	0	0	0	0	0
	0	0	0		0				277	0	145
	11	0	22		0				55	0	29
	170	0	153		1				0	0	0
	277	0	450		1	0	0	1	313	0	339
NYS Rte 17M			CONTROL					NYS Rte 17M			
			Signalized								
			TEV	1306	0	1817					
			PHF	AM 0.87	NOON	PM 0.98					
WESTBOUND								PM	NOON	AM	



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-006

**Date:** 3/10/2022

## Data - Total

NS/EW Streets:	Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M				
	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	TOTAL
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
7:00 AM	25	12	61	0	9	44	6	0	10	24	14	0	55	23	1	0	284
7:15 AM	23	19	55	0	9	42	10	0	5	29	39	0	79	30	0	0	340
7:30 AM	32	26	47	0	8	58	17	0	17	35	37	0	76	36	0	0	389
7:45 AM	23	22	61	0	6	67	15	0	13	35	33	0	97	56	0	0	428
8:00 AM	25	33	59	0	12	37	17	0	13	28	25	0	91	43	2	0	385
8:15 AM	25	25	42	0	14	55	22	0	18	47	38	0	74	33	5	0	398
8:30 AM	27	30	50	0	10	47	25	0	19	48	38	0	77	43	2	0	416
8:45 AM	33	22	58	0	16	52	25	0	8	31	34	0	71	42	4	0	396
<b>TOTAL VOLUMES :</b>	NL 213	NT 189	NR 433	NU 0	SL 84	ST 402	SR 137	SU 0	EL 103	ET 277	ER 258	EU 0	WL 620	WT 306	WR 14	WU 0	TOTAL 3036
<b>APPROACH %'s:</b>	25.51%	22.63%	51.86%	0.00%	13.48%	64.53%	21.99%	0.00%	16.14%	43.42%	40.44%	0.00%	65.96%	32.55%	1.49%	0.00%	
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	100	110	212	0	42	206	79	0	63	158	134	0	339	175	9	0	1627
<b>PEAK HR FACTOR :</b>	0.926	0.833	0.869	0.000		0.750	0.769	0.790	0.000	0.829	0.823	0.882	0.000	0.874	0.781	0.450	0.000
	0.902				0.898				0.845				0.855				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	TOTAL
4:00 PM	59	44	62	0	12	49	27	0	30	59	48	0	100	83	5	0	578
4:15 PM	53	50	58	0	17	43	27	0	17	64	41	0	72	85	3	0	530
4:30 PM	65	33	53	0	20	42	33	0	23	77	46	0	96	89	9	0	586
4:45 PM	65	53	62	0	19	42	20	0	32	80	62	0	58	73	1	0	567
5:00 PM	61	43	62	0	13	38	33	0	27	62	53	0	104	90	6	0	592
5:15 PM	69	51	70	0	16	29	25	0	30	85	49	0	91	77	4	0	596
5:30 PM	56	43	67	0	11	28	17	0	36	72	49	0	82	70	6	0	537
5:45 PM	65	41	57	0	20	40	27	0	18	55	37	0	58	68	5	0	491
<b>TOTAL VOLUMES :</b>	NL 493	NT 358	NR 491	NU 0	SL 128	ST 311	SR 209	SU 0	EL 213	ET 554	ER 385	EU 0	WL 661	WT 635	WR 39	WU 0	TOTAL 4477
<b>APPROACH %'s:</b>	36.74%	26.68%	36.59%	0.00%	19.75%	47.99%	32.25%	0.00%	18.49%	48.09%	33.42%	0.00%	49.51%	47.57%	2.92%	0.00%	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	260	180	247	0	68	151	111	0	112	304	210	0	349	329	20	0	2341
<b>PEAK HR FACTOR :</b>	0.942	0.849	0.882	0.000		0.850	0.899	0.841	0.000	0.875	0.894	0.847	0.000	0.839	0.914	0.556	0.000
	0.904				0.868				0.899				0.873				

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-006

**Date:** 3/10/2022

## Data - Cars

NS/EW Streets:	Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M				
	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	TOTAL
7:00 AM	18	11	51	0	8	39	6	0	9	22	13	0	51	22	1	0	251
7:15 AM	23	16	51	0	6	39	9	0	4	25	34	0	72	27	0	0	306
7:30 AM	29	26	46	0	8	54	15	0	16	35	36	0	72	33	0	0	370
7:45 AM	23	22	55	0	4	65	13	0	11	32	31	0	95	51	0	0	402
8:00 AM	23	32	52	0	10	34	14	0	13	27	23	0	84	39	2	0	353
8:15 AM	22	24	36	0	14	52	20	0	16	42	34	0	71	31	4	0	366
8:30 AM	26	28	50	0	10	35	23	0	17	48	35	0	74	40	1	0	387
8:45 AM	31	19	56	0	16	47	23	0	7	31	32	0	67	41	4	0	374
<b>TOTAL VOLUMES :</b>	NL 195	NT 178	NR 397	NU 0	SL 76	ST 365	SR 123	SU 0	EL 93	ET 262	ER 238	EU 0	WL 586	WT 284	WR 12	WU 0	TOTAL 2809
<b>APPROACH %'s:</b>	25.32%	23.12%	51.56%	0.00%	13.48%	64.72%	21.81%	0.00%	15.68%	44.18%	40.13%	0.00%	66.44%	32.20%	1.36%	0.00%	
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	94	106	193	0	38 0.679	186	70	0	57	149	123	0	324	161	7	0	1508
<b>PEAK HR FACTOR :</b>	0.904	0.828	0.877	0.000		0.715	0.761	0.000	0.838	0.776	0.879	0.000	0.853	0.789	0.438	0.000	0.938
				0.918													
				0.855													
				0.823													
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	TOTAL
4:00 PM	57	44	59	0	12	45	25	0	30	58	47	0	94	81	5	0	557
4:15 PM	50	49	56	0	17	43	26	0	17	60	41	0	67	84	2	0	512
4:30 PM	65	33	47	0	19	38	33	0	23	75	45	0	88	88	9	0	563
4:45 PM	62	52	61	0	19	41	20	0	32	80	55	0	54	72	1	0	549
5:00 PM	59	42	59	0	12	38	33	0	27	61	53	0	102	87	6	0	579
5:15 PM	68	51	70	0	15	29	25	0	30	84	48	0	86	76	4	0	586
5:30 PM	55	41	65	0	11	28	17	0	35	72	49	0	79	70	6	0	528
5:45 PM	65	41	56	0	20	39	27	0	18	55	37	0	55	67	5	0	485
<b>TOTAL VOLUMES :</b>	NL 481	NT 353	NR 473	NU 0	SL 125	ST 301	SR 206	SU 0	EL 212	ET 545	ER 375	EU 0	WL 625	WT 625	WR 38	WU 0	TOTAL 4359
<b>APPROACH %'s:</b>	36.80%	27.01%	36.19%	0.00%	19.78%	47.63%	32.59%	0.00%	18.73%	48.14%	33.13%	0.00%	48.52%	48.52%	2.95%	0.00%	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	254	178	237	0	65 0.855	146	111	0	112	300	201	0	330	323	20	0	2277
<b>PEAK HR FACTOR :</b>	0.934	0.856	0.846	0.000		0.890	0.841	0.000	0.875	0.893	0.914	0.000	0.809	0.918	0.556	0.000	0.971
				0.885													
				0.894													
				0.918													
				0.863													

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-006

**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	7	1	10	0	1	5	0	0	1	2	1	0	4	1	0	0	33	
7:15 AM	0	3	4	0	3	3	1	0	1	4	5	0	7	3	0	0	34	
7:30 AM	3	0	1	0	0	4	2	0	1	0	1	0	4	3	0	0	19	
7:45 AM	0	0	6	0	2	2	2	0	2	3	2	0	2	5	0	0	26	
8:00 AM	2	1	7	0	2	3	3	0	0	1	2	0	7	4	0	0	32	
8:15 AM	3	1	6	0	0	3	2	0	2	5	4	0	3	2	1	0	32	
8:30 AM	1	2	0	0	0	12	2	0	2	0	3	0	3	3	1	0	29	
8:45 AM	2	3	2	0	0	5	2	0	1	0	2	0	4	1	0	0	22	
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
<b>APPROACH %'s:</b>	18	11	36	0	8	37	14	0	10	15	20	0	34	22	2	0	227	
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																<b>TOTAL</b>	
<b>PEAK HR VOL :</b>	6	4	19	0	0.500	20	9	0	6	9	11	0	15	14	2	0	119	
<b>PEAK HR FACTOR :</b>	0.500	0.500	0.679	0.000		0.417	0.750	0.000	0.750	0.450	0.688	0.000	0.536	0.700	0.500	0.000	0.930	
					0.725				0.589				0.591				0.705	
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	1	1	1	0	1	0.5	0.5	0	1	1.5	0.5	0	2	0.5	0.5	0		
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
4:00 PM	2	0	3	0	0	4	2	0	0	1	1	0	6	2	0	0	21	
4:15 PM	3	1	2	0	0	0	1	0	0	4	0	0	5	1	1	0	18	
4:30 PM	0	0	6	0	1	4	0	0	0	2	1	0	8	1	0	0	23	
4:45 PM	3	1	1	0	0	1	0	0	0	0	7	0	4	1	0	0	18	
5:00 PM	2	1	3	0	1	0	0	0	0	1	0	0	2	3	0	0	13	
5:15 PM	1	0	0	0	1	0	0	0	0	1	1	0	5	1	0	0	10	
5:30 PM	1	2	2	0	0	0	0	0	1	0	0	0	3	0	0	0	9	
5:45 PM	0	0	1	0	0	1	0	0	0	0	0	0	3	1	0	0	6	
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
<b>APPROACH %'s:</b>	12	5	18	0	3	10	3	0	1	9	10	0	36	10	1	0	118	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>	
<b>PEAK HR VOL :</b>	6	2	10	0	0.500	5	0	0	0	4	9	0	19	6	0	0	64	
<b>PEAK HR FACTOR :</b>	0.500	0.500	0.417	0.000		0.750	0.313	0.000	0.000	0.500	0.321	0.000	0.594	0.500	0.000	0.694	0.696	
					0.750				0.400				0.464				0.694	

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester

## **City: Chester**

### **Control: Signalized**

Project ID: 22-380005-006

Date: 3/10/2022

Data - Bikes

NS/EW Streets:		Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
		1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	TOTAL
7:00 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>		NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0
<b>PEAK HR :</b>		<b>07:45 AM - 08:45 AM</b>																<b>TOTAL 0</b>
<b>PEAK HR VOL :</b>		0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>
<b>PEAK HR FACTOR :</b>																		

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M  
**City:** Chester

**Project ID:** 22-380005-006  
**Date:** 3/10/2022

### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Academy Ave/Rte 94		Academy Ave/Rte 94		NYS Rte 17M		NYS Rte 17M		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	3	0	0	0	0	0	0	3
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	2	0	0	0	0	0	0	0	2
8:45 AM	0	0	1	0	0	0	0	0	1
<b>TOTAL VOLUMES :</b>	EB 2	WB 3	EB 1	WB 0	NB 0	SB 0	NB 0	SB 0	<b>TOTAL</b> 6
<b>APPROACH %'s :</b>	40.00%	60.00%	100.00%	0.00%					
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	2	3	0	0	0	0	0	0	<b>TOTAL</b> 5
<b>PEAK HR FACTOR :</b>	0.250	0.250	0.417						0.417

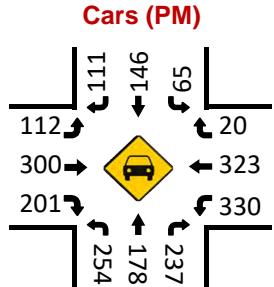
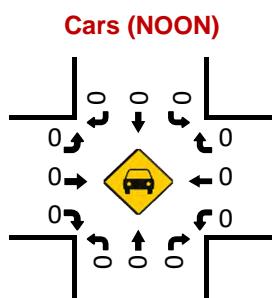
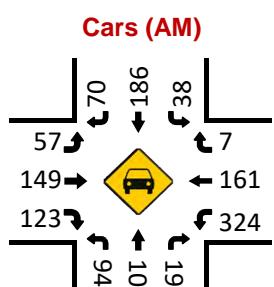
<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	1	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	1	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB 0	WB 0	EB 0	WB 0	NB 1	SB 1	NB 0	SB 0	<b>TOTAL</b> 2
<b>APPROACH %'s :</b>	50.00%		50.00%						
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	1	0	0	0	<b>TOTAL</b> 1
<b>PEAK HR FACTOR :</b>			0.250		0.250		0.250		0.250

## **Academy Ave/Rte 94 & NYS Rte 17M**

# Peak Hour Turning Movement Count

**ID:** 22-380005-006  
**City:** Chester

		Academy Ave/Rte 94										
		SOUTHBOUND										
PEAK HOURS	07:45 AM - 08:45 AM		AM	79	206	42	0	182	AM	7:00 AM - 09:00 AM		COUNT PERIODS
	NONE		NOON	0	0	0	0	0	NOON	NONE		
	04:30 PM - 05:30 PM		PM	111	151	68	0	312	PM	4:00 PM - 06:00 PM		
		AM	NOON	PM					PM	NOON	AM	
NYS Rte 17M  EASTBOUND	354	0	700									
	0	0	0									
	63	0	112									
	158	0	304									
	134	0	210									
	AM	NOON	PM									
		CONTROL									WESTBOUND	
		Signalized									NYS Rte 17M	
		TEV	1627	0	2341							
		PHF	AM 0.95	NOON	PM 0.98							
</												



**Pedestrians (Crosswalks)**

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester

#### **Control:** Signalized

Project ID: 22-380005-007

Date: 3/10/2022

## Data - Total

NS/EW Streets:		Rte 126 NB On/Off Ramp				Rte 126 NB On/Off Ramp				Rte 94				Rte 94				
AM	NL	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU		
7:00 AM	21	0	5	0	0	0	0	0	29	87	0	0	0	52	61	0	255	
7:15 AM	28	0	10	0	0	0	0	0	24	90	0	0	0	69	88	0	309	
7:30 AM	24	0	14	0	0	0	0	0	21	89	0	0	0	68	108	0	324	
7:45 AM	38	0	12	0	0	0	0	0	26	95	0	0	0	67	129	0	367	
8:00 AM	35	0	10	0	0	0	0	0	18	103	0	1	0	53	101	0	321	
8:15 AM	29	0	12	0	0	0	0	0	18	84	0	0	0	82	84	0	309	
8:30 AM	28	0	21	0	0	0	0	0	19	85	0	0	0	72	92	0	317	
8:45 AM	40	1	8	0	0	0	0	0	22	105	0	0	0	62	90	0	328	
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
APPROACH %'s :	243	1	92	0	0	0	0	0	177	738	0	1	0	525	753	0	2530	
PEAK HR :	07:30 AM - 08:30 AM				0.870	0.000	0.000	0.000	0.798	0.900	0.000	0.250	0.000	0.823	0.818	0.000	TOTAL	
PEAK HR VOL :	126	0	48	0										270	422	0	1321	
PEAK HR FACTOR :	0.829	0.000	0.857	0.000										0.932	0.883	0.900		

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
4:00 PM	54	0	34	0	0	0	0	0	44	125	0	0	0	89	107	0	453
4:15 PM	58	0	41	0	0	0	0	0	39	123	0	0	0	67	80	0	408
4:30 PM	58	0	46	0	0	0	0	0	65	117	0	0	0	98	96	0	480
4:45 PM	66	0	31	0	0	0	0	0	62	138	0	0	0	63	98	0	458
5:00 PM	61	0	41	0	0	0	0	0	51	130	0	0	0	75	113	1	472
5:15 PM	59	0	45	0	0	0	0	0	44	143	0	0	0	70	108	0	469
5:30 PM	54	0	51	0	0	0	0	0	56	111	0	0	0	65	94	0	431
5:45 PM	51	0	47	0	0	0	0	0	48	116	0	0	0	51	81	0	394
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	461	0	336	0	0	0	0	0	409	1003	0	0	0	578	777	1	3565
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	244	0	163	0	0	0	0	0	222	528	0	0	0	306	415	1	1879
<b>PEAK HR FACTOR :</b>	0.924	0.000	0.886	0.000	0.000	0.000	0.000	0.000	0.854	0.923	0.000	0.000	0.000	0.781	0.918	0.250	0.979
					0.978					0.938					0.930		

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-007

**Date:** 3/10/2022

## Data - Cars

NS/EW Streets:	Rte 126 NB On/Off Ramp				Rte 126 NB On/Off Ramp				Rte 94				Rte 94				
<b>AM</b>	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				<b>TOTAL</b>
	0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
7:00 AM	13	0	5	0	0	0	0	0	25	70	0	0	0	47	58	0	218
7:15 AM	19	0	10	0	0	0	0	0	21	83	0	0	0	63	79	0	275
7:30 AM	20	0	13	0	0	0	0	0	19	85	0	0	0	64	102	0	303
7:45 AM	30	0	12	0	0	0	0	0	20	91	0	0	0	64	125	0	342
8:00 AM	28	0	9	0	0	0	0	0	15	94	0	1	0	49	94	0	290
8:15 AM	25	0	11	0	0	0	0	0	17	74	0	0	0	75	80	0	282
8:30 AM	25	0	20	0	0	0	0	0	16	82	0	0	0	58	88	0	289
8:45 AM	35	1	8	0	0	0	0	0	18	99	0	0	0	55	86	0	302
<b>TOTAL VOLUMES :</b>	NL 195	NT 1	NR 88	NU 0	SL 0	ST 0	SR 0	SU 0	EL 151	ET 678	ER 0	EU 1	WL 0	WT 475	WR 712	WU 0	<b>TOTAL</b> 2301
<b>APPROACH %'s:</b>	68.66%	0.35%	30.99%	0.00%					18.19%	81.69%	0.00%	0.12%	0.00%	40.02%	59.98%	0.00%	
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	103	0	45	0					71	344	0	1	0	252	401	0	1217
<b>PEAK HR FACTOR :</b>	0.858	0.000	0.865	0.000	0.000	0.000	0.000	0.000	0.888	0.915	0.000	0.250	0.000	0.840	0.802	0.000	0.890
					0.881					0.937				0.864			
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
<b>PM</b>	0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	<b>TOTAL</b>
	48	0	33	0	0	0	0	0	40	122	0	0	0	81	105	0	429
4:00 PM	51	0	39	0	0	0	0	0	36	119	0	0	0	67	75	0	387
4:15 PM	45	0	45	0	0	0	0	0	60	112	0	0	0	90	91	0	443
4:30 PM	59	0	31	0	0	0	0	0	57	132	0	0	0	60	89	0	428
4:45 PM	53	0	40	0	0	0	0	0	50	126	0	0	0	75	111	1	456
5:00 PM	55	0	45	0	0	0	0	0	41	142	0	0	0	68	104	0	455
5:15 PM	49	0	51	0	0	0	0	0	55	106	0	0	0	64	92	0	417
5:30 PM	43	0	47	0	0	0	0	0	47	115	0	0	0	49	79	0	380
<b>TOTAL VOLUMES :</b>	NL 403	NT 0	NR 331	NU 0	SL 0	ST 0	SR 0	SU 0	EL 386	ET 974	ER 0	EU 0	WL 0	WT 554	WR 746	WU 1	<b>TOTAL</b> 3395
<b>APPROACH %'s:</b>	54.90%	0.00%	45.10%	0.00%					28.38%	71.62%	0.00%	0.00%	0.00%	42.58%	57.34%	0.08%	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	212	0	161	0					208	512	0	0	0	293	395	1	1782
<b>PEAK HR FACTOR :</b>	0.898	0.000	0.894	0.000	0.000	0.000	0.000	0.000	0.867	0.901	0.000	0.000	0.000	0.814	0.890	0.250	0.977
					0.933					0.952				0.921			

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-007

**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	Rte 126 NB On/Off Ramp				Rte 126 NB On/Off Ramp				Rte 94				Rte 94				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	TOTAL
7:00 AM	8	0	0	0	0	0	0	0	4	17	0	0	0	5	3	0	37
7:15 AM	9	0	0	0	0	0	0	0	3	7	0	0	0	6	9	0	34
7:30 AM	4	0	1	0	0	0	0	0	2	4	0	0	0	4	6	0	21
7:45 AM	8	0	0	0	0	0	0	0	6	4	0	0	0	3	4	0	25
8:00 AM	7	0	1	0	0	0	0	0	3	9	0	0	0	4	7	0	31
8:15 AM	4	0	1	0	0	0	0	0	1	10	0	0	0	7	4	0	27
8:30 AM	3	0	1	0	0	0	0	0	3	3	0	0	0	14	4	0	28
8:45 AM	5	0	0	0	0	0	0	0	4	6	0	0	0	7	4	0	26
<b>TOTAL VOLUMES :</b>	NL 48	NT 0	NR 4	NU 0	SL 0	ST 0	SR 0	SU 0	EL 26	ET 60	ER 0	EU 0	WL 0	WT 50	WR 41	WU 0	TOTAL 229
<b>APPROACH %'s:</b>	92.31%	0.00%	7.69%	0.00%					30.23%	69.77%	0.00%	0.00%	0.00%	54.95%	45.05%	0.00%	
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	23	0	3	0					12	27	0	0	0	18	21	0	104
<b>PEAK HR FACTOR :</b>	0.719	0.000	0.750	0.000	0.000	0.000	0.000	0.000	0.500	0.675	0.000	0.000	0.000	0.643	0.750	0.000	0.839
					0.813					0.813				0.886			
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
PM	0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	TOTAL
4:00 PM	6	0	1	0	0	0	0	0	4	3	0	0	0	8	2	0	24
4:15 PM	7	0	2	0	0	0	0	0	3	4	0	0	0	0	5	0	21
4:30 PM	13	0	1	0	0	0	0	0	5	5	0	0	0	8	5	0	37
4:45 PM	7	0	0	0	0	0	0	0	5	6	0	0	0	3	9	0	30
5:00 PM	8	0	1	0	0	0	0	0	1	4	0	0	0	0	2	0	16
5:15 PM	4	0	0	0	0	0	0	0	3	1	0	0	0	2	4	0	14
5:30 PM	5	0	0	0	0	0	0	0	1	5	0	0	0	1	2	0	14
5:45 PM	8	0	0	0	0	0	0	0	1	1	0	0	0	2	2	0	14
<b>TOTAL VOLUMES :</b>	NL 58	NT 0	NR 5	NU 0	SL 0	ST 0	SR 0	SU 0	EL 23	ET 29	ER 0	EU 0	WL 0	WT 24	WR 31	WU 0	TOTAL 170
<b>APPROACH %'s:</b>	92.06%	0.00%	7.94%	0.00%					44.23%	55.77%	0.00%	0.00%	0.00%	43.64%	56.36%	0.00%	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	32	0	2	0					14	16	0	0	0	13	20	0	97
<b>PEAK HR FACTOR :</b>	0.615	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.700	0.667	0.000	0.000	0.000	0.406	0.556	0.000	0.655
					0.607					0.682				0.635			

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester

#### **Control:** Signalized

**Project ID:** 22-380005-007

Date: 3/10/2022

Data - Bikes

NS/EW Streets:		Rte 126 NB On/Off Ramp				Rte 126 NB On/Off Ramp				Rte 94				Rte 94				
AM	NL	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU		
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>																		

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
4:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
4:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
4:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
5:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
5:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
5:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
5:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL 0</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>
<b>PEAK HR FACTOR :</b>																	<b>TOTAL 0</b>

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Rte 126 NB On/Off Ramp & Rte 94  
**City:** Chester

**Project ID:** 22-380005-007  
**Date:** 3/10/2022

### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Rte 126 NB On/Off Ramp		Rte 126 NB On/Off Ramp		Rte 94		Rte 94		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

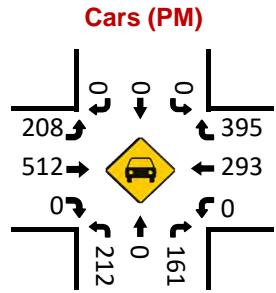
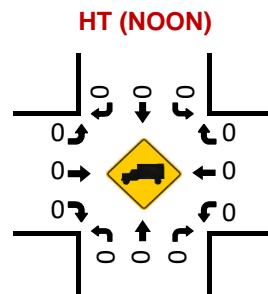
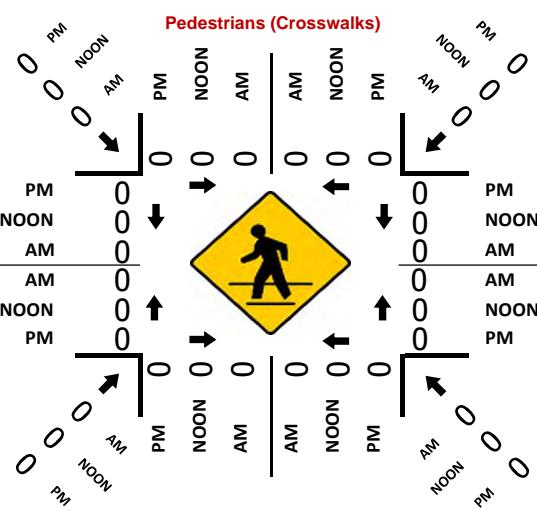
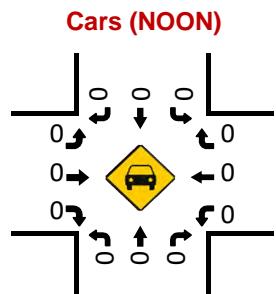
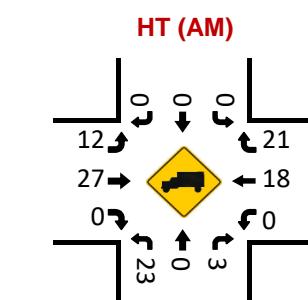
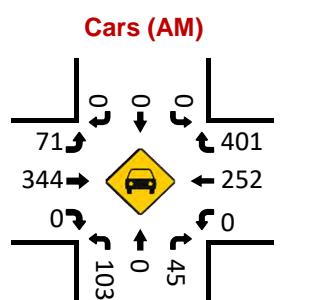
<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	1	0	0	0	0	1
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	1	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	2	0	0	0	0	2
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

## Rte 126 NB On/Off Ramp & Rte 94

# Peak Hour Turning Movement Count

**ID:** 22-380005-007  
**City:** Chester

ID: 22-380005-007		Rte 126 NB On/Off Ramp								Day: Thursday						
City: Chester		SOUTHBOUND								Date: 3/10/2022						
PEAK HOURS	07:30 AM - 08:30 AM			AM	0	0	0	0	505	AM	7:00 AM - 09:00 AM					
	NONE			NOON	0	0	0	0	0	NOON	NONE					
	04:30 PM - 05:30 PM			PM	0	0	0	0	637	PM	4:00 PM - 06:00 PM					
											COUNT PERIODS					
Rte 94 EASTBOUND	AM NOON PM															
	397	0	550	0 0 0 0				1  1 0 415								
	1	0	0	0				1 306 0 270								
	83	0	222	1				0 0 0 0								
	371	0	528	1				0 1 0 0								
Rte 94 WESTBOUND	AM NOON PM			0 0.5 0.5 1				1 692 0 419								
	0	0	0	0				1 PM NOON AM								
											Rte 94					
		CONTROL														
		Signalized														
		TEV	1321	0	1879											
		PHF	0.90	AM	NOON	PM	0.98									



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-008

**Date:** 3/10/2022

## Data - Total

NS/EW Streets:	Rte 126 SB On/Off Ramp				Rte 126 SB On/Off Ramp				Rte 94				Rte 94				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	0	0	0	61	0	28	0	0	54	77	0	30	42	0	0	292
7:15 AM	0	0	0	0	61	1	30	0	0	54	80	0	42	56	0	0	324
7:30 AM	0	0	0	0	61	0	40	0	0	48	69	0	35	55	0	0	308
7:45 AM	0	0	0	0	71	0	64	0	0	52	68	0	25	82	0	0	362
8:00 AM	0	0	0	0	72	1	43	0	0	48	69	0	19	66	0	0	318
8:15 AM	0	0	0	0	51	0	46	0	0	53	65	0	44	69	0	0	328
8:30 AM	0	0	0	0	50	0	36	0	0	52	55	0	42	60	0	0	295
8:45 AM	0	0	0	0	66	0	35	0	0	64	78	0	25	75	0	0	343
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	0	0	0	493	2	322	0	0	425	561	0	262	505	0	0	2570
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	255	1	193	0	0	201	271	0	123	272	0	0	1316
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.885	0.250	0.754	0.000	0.000	0.948	0.982	0.000	0.699	0.829	0.000	0.000	0.909

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0	0	0	0	1	0.5	0.5	0	0	1	1	0	1	1	0	0	TOTAL
PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	0	0	0	0	71	1	36	0	0	101	31	0	32	109	0	0	381
4:15 PM	0	0	0	0	81	0	38	0	0	78	43	0	26	101	0	0	367
4:30 PM	0	0	0	0	72	0	32	0	0	112	45	0	25	129	0	0	415
4:45 PM	0	0	0	0	76	0	61	0	0	121	54	0	24	107	0	0	443
5:00 PM	0	0	0	0	73	0	57	0	0	111	51	0	28	106	0	0	426
5:15 PM	0	0	0	0	90	0	54	0	0	94	43	0	29	101	0	0	411
5:30 PM	0	0	0	0	76	0	45	0	0	95	44	0	32	85	0	0	377
5:45 PM	0	0	0	0	74	1	43	0	0	88	41	0	13	90	0	0	350
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	0	0	0	613	2	366	0	0	800	352	0	209	828	0	0	3170
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	311	0	204	0	0	438	193	0	106	443	0	0	1695
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.864	0.000	0.836	0.000	0.000	0.905	0.894	0.000	0.914	0.859	0.000	0.000	0.957

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-008

**Date:** 3/10/2022

## Data - Cars

NS/EW Streets:	Rte 126 SB On/Off Ramp				Rte 126 SB On/Off Ramp				Rte 94				Rte 94					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	0	0	0	0	54	0	27	0	0	40	73	0	26	32	0	0	252	
7:15 AM	0	0	0	0	57	1	24	0	0	48	71	0	39	45	0	0	285	
7:30 AM	0	0	0	0	58	0	32	0	0	45	62	0	33	49	0	0	279	
7:45 AM	0	0	0	0	69	0	59	0	0	44	58	0	23	73	0	0	326	
8:00 AM	0	0	0	0	66	0	35	0	0	42	62	0	16	58	0	0	279	
8:15 AM	0	0	0	0	43	0	43	0	0	49	59	0	41	61	0	0	296	
8:30 AM	0	0	0	0	49	0	30	0	0	48	47	0	40	45	0	0	259	
8:45 AM	0	0	0	0	61	0	29	0	0	58	66	0	21	67	0	0	302	
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
<b>APPROACH %'s :</b>	0	0	0	0	457	1	279	0	0	374	498	0	239	430	0	0	2278	
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>				0.855	0	169	0	0	180	241	0	113	241	0	0	TOTAL	
<b>PEAK HR VOL :</b>	0	0	0	0		236	0	0.716	0	0.000	0.918	0.972	0.000	0.689	0.825	0.000	0.000	1180
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000		0.791				0.975				0.868			0.905	
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
PM	0	0	0	0	1	0.5	0.5	0	0	1	1	0	1	1	0	0	TOTAL	
4:00 PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU		
4:00 PM	0	0	0	0	68	1	30	0	0	97	31	0	31	96	0	0	354	
4:15 PM	0	0	0	0	78	0	35	0	0	73	36	0	26	94	0	0	342	
4:30 PM	0	0	0	0	68	0	27	0	0	107	40	0	24	110	0	0	376	
4:45 PM	0	0	0	0	73	0	54	0	0	113	47	0	24	96	0	0	407	
5:00 PM	0	0	0	0	71	0	49	0	0	108	46	0	28	98	0	0	400	
5:15 PM	0	0	0	0	90	0	45	0	0	90	40	0	29	95	0	0	389	
5:30 PM	0	0	0	0	75	0	44	0	0	90	38	0	31	80	0	0	358	
5:45 PM	0	0	0	0	73	1	37	0	0	87	37	0	13	80	0	0	328	
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
<b>APPROACH %'s :</b>	0	0	0	0	596	2	321	0	0	765	315	0	206	749	0	0	2954	
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>				0.839	0	175	0	0	418	173	0	105	399	0	0	TOTAL	
<b>PEAK HR VOL :</b>	0	0	0	0		302	0	0.810	0	0.000	0.925	0.920	0.000	0.905	0.907	0.000	0.000	1572
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000		0.883				0.923				0.940			0.966	

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-008

**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	Rte 126 SB On/Off Ramp				Rte 126 SB On/Off Ramp				Rte 94				Rte 94										
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND										
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL						
7:00 AM	0	0	0	0	7	0	1	0	0	14	4	0	4	10	0	0	40						
7:15 AM	0	0	0	0	4	0	6	0	0	6	9	0	3	11	0	0	39						
7:30 AM	0	0	0	0	3	0	8	0	0	3	7	0	2	6	0	0	29						
7:45 AM	0	0	0	0	2	0	5	0	0	8	10	0	2	9	0	0	36						
8:00 AM	0	0	0	0	6	1	8	0	0	6	7	0	3	8	0	0	39						
8:15 AM	0	0	0	0	8	0	3	0	0	4	6	0	3	8	0	0	32						
8:30 AM	0	0	0	0	1	0	6	0	0	4	8	0	2	15	0	0	36						
8:45 AM	0	0	0	0	5	0	6	0	0	6	12	0	4	8	0	0	41						
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	<b>TOTAL</b>						
<b>APPROACH %'s :</b>	0	0	0	0	36	1	43	0	0	51	63	0	23	75	0	0	292						
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>				19	1	24	0	0	21	30	0	10	31	0	0	<b>TOTAL</b>						
<b>PEAK HR VOL :</b>	0	0	0	0									0.594	0.250	0.750	0.000	136						
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.733				0.708				0.932				0.872						
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND										
PM	0	0	0	0	1	0.5	0.5	0	0	1	1	0	1	1	0	0	TOTAL						
4:00 PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU							
4:15 PM	0	0	0	0	3	0	6	0	0	4	0	0	1	13	0	0	27						
4:30 PM	0	0	0	0	3	0	3	0	0	5	7	0	0	7	0	0	25						
4:45 PM	0	0	0	0	4	0	5	0	0	5	5	0	1	19	0	0	39						
5:00 PM	0	0	0	0	3	0	7	0	0	8	7	0	0	11	0	0	36						
5:15 PM	0	0	0	0	2	0	8	0	0	3	5	0	0	8	0	0	26						
5:30 PM	0	0	0	0	0	0	9	0	0	4	3	0	0	6	0	0	22						
5:45 PM	0	0	0	0	1	0	1	0	0	5	6	0	1	5	0	0	19						
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	<b>TOTAL</b>						
<b>APPROACH %'s :</b>	0	0	0	0	17	0	45	0	0	35	37	0	3	79	0	0	216						
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>				9	0	29	0	0	20	20	0	1	44	0	0	<b>TOTAL</b>						
<b>PEAK HR VOL :</b>	0.000	0.000	0.000	0.000									0.563	0.000	0.806	0.000	123						
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.950				0.667				0.563				0.788						

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-008

**Date:** 3/10/2022

## Data - Bikes

NS/EW Streets:	Rte 126 SB On/Off Ramp				Rte 126 SB On/Off Ramp				Rte 94				Rte 94					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 0</b>	
<b>APPROACH %'s :</b>																		
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	0	0	0	0	1	0.5	0.5	0	0	1	1	1	0	1	1	0	0	TOTAL
NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU			
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 0</b>	
<b>APPROACH %'s :</b>																		
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>																	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94  
**City:** Chester

**Project ID:** 22-380005-008  
**Date:** 3/10/2022

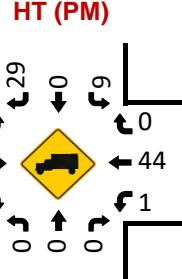
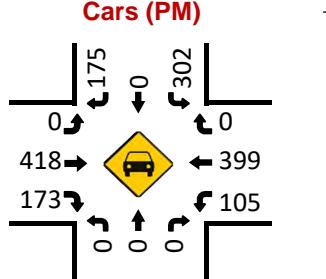
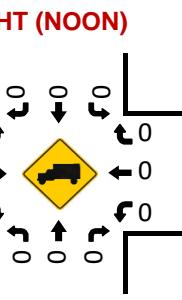
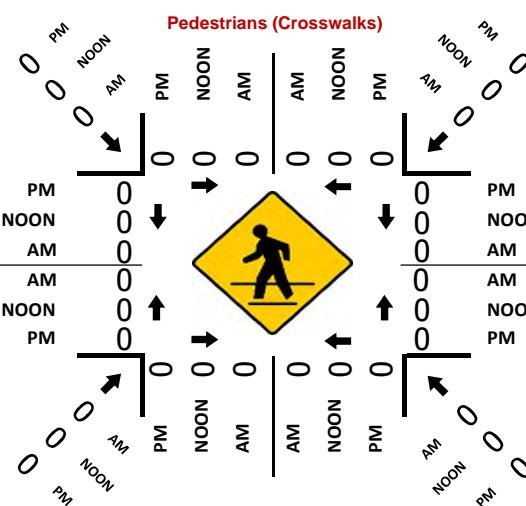
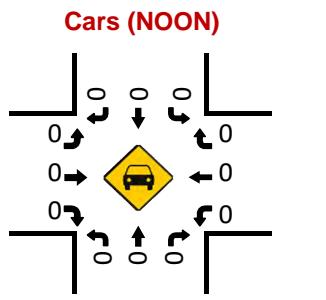
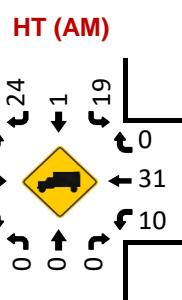
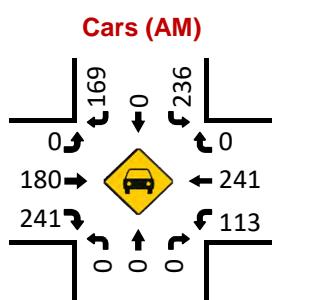
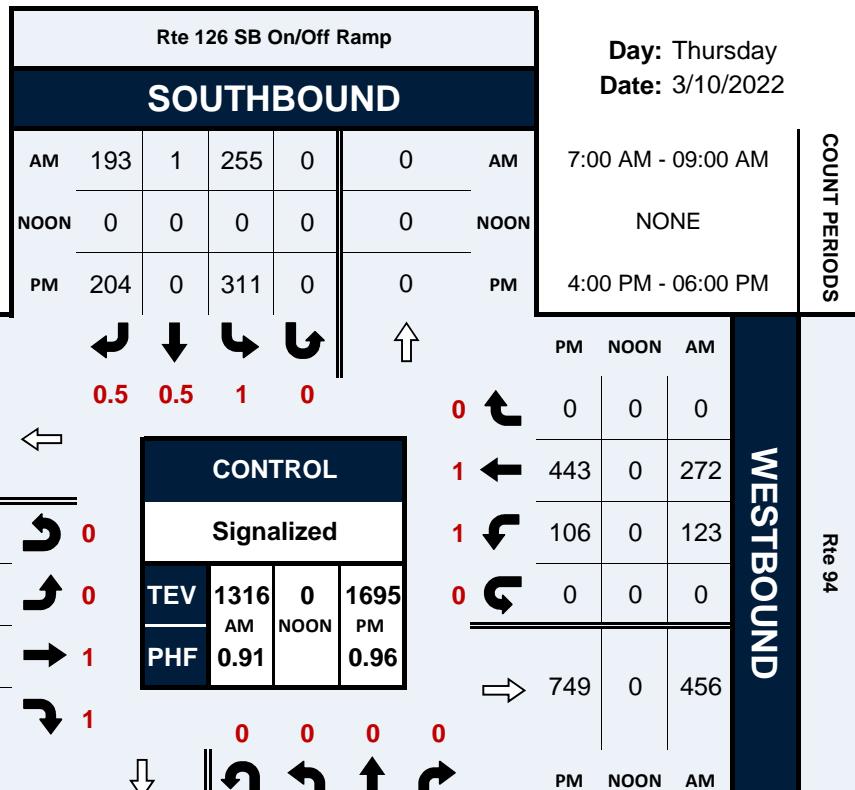
### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Rte 126 SB On/Off Ramp		Rte 126 SB On/Off Ramp		Rte 94		Rte 94		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG	<b>TOTAL</b>	
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	1	0	0	0	0	1
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	1	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
	0	0	0	2	0	0	0	0	2
<b>PEAK HR :</b>	<b>04:30 PM - 05:30 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

**Rte 126 SB On/Off Ramp & Rte 94****Peak Hour Turning Movement Count**

**ID:** 22-380005-008  
**City:** Chester



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M  
**City:** Monroe  
**Control:** 2-Way Stop(NB/SB)

**Project ID:** 22-380005-009  
**Date:** 3/10/2022

## Data - Total

NS/EW Streets:	Museum Village Rd				Museum Village Rd				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
7:00 AM	1	7	14	0	17	3	8	0	6	21	0	0	5	21	6	0	109
7:15 AM	1	13	4	0	26	3	6	0	9	43	0	0	2	25	6	0	138
7:30 AM	0	16	4	0	24	5	9	0	7	28	0	0	2	30	8	0	133
7:45 AM	0	17	2	0	28	1	16	0	11	34	0	0	1	54	5	0	169
8:00 AM	0	14	4	0	19	4	10	0	4	33	1	0	2	37	8	0	136
8:15 AM	0	13	0	0	21	3	15	0	7	29	0	0	5	47	4	0	144
8:30 AM	0	7	3	0	19	3	13	0	7	37	0	0	2	41	11	0	143
8:45 AM	0	6	4	0	25	4	10	0	8	47	0	0	2	33	11	0	150
<b>TOTAL VOLUMES :</b>	NL 2 1.54%	NT 93 71.54%	NR 35 26.92%	NU 0 0.00%	SL 179 61.30%	ST 26 8.90%	SR 87 29.79%	SU 0 0.00%	EL 59 17.77%	ET 272 81.93%	ER 1 0.30%	EU 0 0.00%	WL 21 5.71%	WT 288 78.26%	WR 59 16.03%	WU 0 0.00%	TOTAL 1122
<b>APPROACH %'s :</b>																	
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	51	9	0					29	133	1	0	10	179	28	0	592
<b>PEAK HR FACTOR :</b>	0.000	0.750	0.563	0.000	87	11	54	0	0.659	0.899	0.250	0.000	0.500	0.829	0.636	0.000	0.876
					0.789		0.844				0.906				0.904		
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
4:00 PM	0	6	3	0	21	16	9	0	10	44	0	0	7	79	21	0	216
4:15 PM	0	5	3	0	24	9	11	0	11	39	0	0	3	60	23	0	188
4:30 PM	0	7	1	0	28	9	12	0	10	28	1	0	4	62	12	0	174
4:45 PM	0	10	6	0	28	12	15	0	10	37	0	0	3	82	20	0	223
5:00 PM	0	7	6	0	26	8	14	0	12	49	1	0	6	78	26	0	233
5:15 PM	1	6	6	0	21	9	15	0	10	42	1	0	4	76	21	0	212
5:30 PM	0	10	2	0	21	10	10	0	8	26	0	0	6	56	25	0	174
5:45 PM	1	9	3	0	19	9	8	0	10	32	0	0	3	51	17	0	162
<b>TOTAL VOLUMES :</b>	NL 2 2.17%	NT 60 65.22%	NR 30 32.61%	NU 0 0.00%	SL 188 51.65%	ST 82 22.53%	SR 94 25.82%	SU 0 0.00%	EL 81 21.26%	ET 297 77.95%	ER 3 0.79%	EU 0 0.00%	WL 36 4.83%	WT 544 73.02%	WR 165 22.15%	WU 0 0.00%	TOTAL 1582
<b>APPROACH %'s :</b>																	
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	1	33	20	0					96	39	54	0	40	154	2	0	842
<b>PEAK HR FACTOR :</b>	0.250	0.825	0.833	0.000	0.857	0.813	0.900	0.000	0.833	0.786	0.500	0.000	0.792	0.890	0.885	0.000	0.903
					0.844		0.859				0.790				0.916		

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M  
**City:** Monroe  
**Control:** 2-Way Stop(NB/SB)

**Project ID:** 22-380005-009  
**Date:** 3/10/2022

## Data - Cars

NS/EW Streets:	Museum Village Rd				Museum Village Rd				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
7:00 AM	1	7	14	0	17	3	8	0	6	20	0	0	5	19	6	0	106
7:15 AM	1	13	4	0	26	3	6	0	7	39	0	0	2	24	5	0	130
7:30 AM	0	16	4	0	23	5	7	0	7	26	0	0	2	28	7	0	125
7:45 AM	0	17	2	0	26	1	16	0	8	31	0	0	1	51	5	0	158
8:00 AM	0	13	4	0	17	3	9	0	4	29	1	0	2	37	8	0	127
8:15 AM	0	11	0	0	20	3	15	0	7	28	0	0	5	45	4	0	138
8:30 AM	0	7	3	0	19	3	11	0	7	32	0	0	2	33	11	0	128
8:45 AM	0	6	4	0	25	2	9	0	7	44	0	0	2	31	10	0	140
<b>TOTAL VOLUMES :</b>	NL 2 1.57%	NT 90 70.87%	NR 35 27.56%	NU 0 0.00%	SL 173 62.45%	ST 23 8.30%	SR 81 29.24%	SU 0 0.00%	EL 53 17.49%	ET 249 82.18%	ER 1 0.33%	EU 0 0.00%	WL 21 6.09%	WT 268 77.68%	WR 56 16.23%	WU 0 0.00%	TOTAL 1052
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	48 0.706	9 0.563	0 0.000	82 0.788	10 0.833	51 0.797	0 0.000	26 0.813	120 0.938	1 0.250	0 0.000	10 0.500	166 0.814	28 0.636	0 0.000	551 0.872
	0.750				0.831				0.942				0.895				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
4:00 PM	0	6	3	0	21	16	9	0	9	42	0	0	7	79	21	0	213
4:15 PM	0	5	3	0	23	8	11	0	10	37	0	0	3	58	23	0	181
4:30 PM	0	7	1	0	28	9	11	0	10	26	1	0	3	57	10	0	163
4:45 PM	0	10	6	0	28	12	15	0	10	36	0	0	3	81	19	0	220
5:00 PM	0	7	6	0	25	8	14	0	11	48	1	0	6	74	26	0	226
5:15 PM	1	6	6	0	21	9	14	0	10	41	1	0	4	74	21	0	208
5:30 PM	0	10	2	0	21	8	9	0	8	25	0	0	5	56	25	0	169
5:45 PM	0	8	3	0	19	9	8	0	10	32	0	0	3	50	17	0	159
<b>TOTAL VOLUMES :</b>	NL 1 1.11%	NT 59 65.56%	NR 30 33.33%	NU 0 0.00%	SL 186 52.25%	ST 79 22.19%	SR 91 25.56%	SU 0 0.00%	EL 78 21.20%	ET 287 77.99%	ER 3 0.82%	EU 0 0.00%	WL 34 4.69%	WT 529 72.97%	WR 162 22.34%	WU 0 0.00%	TOTAL 1539
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	1 0.250	33 0.825	20 0.833	0 0.000	95 0.848	37 0.771	52 0.867	0 0.000	39 0.886	150 0.781	2 0.500	0 0.000	18 0.750	285 0.880	91 0.875	0 0.000	823 0.910
	0.844				0.836				0.796				0.929				

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M

**City:** Monroe

**Control:** 2-Way Stop(NB/SB)

**Project ID:** 22-380005-009

**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	Museum Village Rd				Museum Village Rd				NYS Rte 17M				NYS Rte 17M										
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND										
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL						
7:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	2	0	0	3						
7:15 AM	0	0	0	0	0	0	0	0	2	4	0	0	0	1	1	0	8						
7:30 AM	0	0	0	0	1	0	2	0	0	2	0	0	0	2	1	0	8						
7:45 AM	0	0	0	0	2	0	0	0	3	3	0	0	0	3	0	0	11						
8:00 AM	0	1	0	0	2	1	1	0	0	4	0	0	0	0	0	0	9						
8:15 AM	0	2	0	0	1	0	0	0	0	1	0	0	0	2	0	0	6						
8:30 AM	0	0	0	0	0	0	2	0	0	5	0	0	0	8	0	0	15						
8:45 AM	0	0	0	0	0	2	1	0	1	3	0	0	0	2	1	0	10						
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL						
<b>APPROACH %'s :</b>	0	3	0	0	6	3	6	0	6	23	0	0	0	20	3	0	70						
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>				0.375	0.250	0.375	0.000	0.250	0.650	0.000	0.000	0.000	13	0	0	41						
<b>PEAK HR VOL :</b>	0	3	0	0																			
<b>PEAK HR FACTOR :</b>	0.000	0.375	0.000	0.000										0.406	0.000	0.000	0.683						
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND										
	0	1	0	0	0	1	0	0	0	1	0	0	0	1	0	0	TOTAL						
4:00 PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL						
4:15 PM	0	0	0	0	0	0	0	0	1	2	0	0	0	0	0	0	3						
4:30 PM	0	0	0	0	0	0	0	1	0	1	0	0	0	2	0	0	7						
4:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	5	2	0	11						
5:00 PM	0	0	0	0	1	0	0	0	1	1	0	0	0	4	0	0	7						
5:15 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	2	0	0	4						
5:30 PM	0	0	0	0	0	2	1	0	0	1	0	0	1	0	0	0	5						
5:45 PM	1	1	0	0	0	0	0	0	0	0	0	0	0	1	0	0	3						
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL						
<b>APPROACH %'s :</b>	1	1	0	0	2	3	3	0	3	10	0	0	2	15	3	0	43						
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>				0.417	0.250	0.500	0.000	0.250	1.000	0.000	0.000	0.250	0.438	0.250	0.000	19						
<b>PEAK HR VOL :</b>	0	0	0	0										7	1	0							
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000										0.417	0.563	0.679							

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M

**City:** Monroe

**Control:** 2-Way Stop(NB/SB)

Project ID: 22-380005-009

Date: 3/10/2022

Data - Bikes

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M  
**City:** Monroe

**Project ID:** 22-380005-009  
**Date:** 3/10/2022

### Data - Pedestrians (Crosswalks)

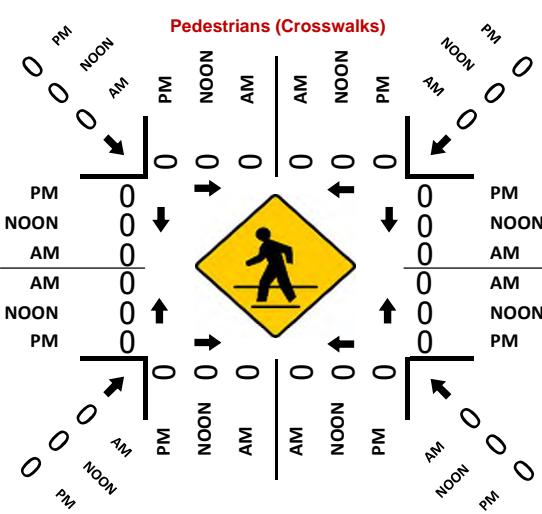
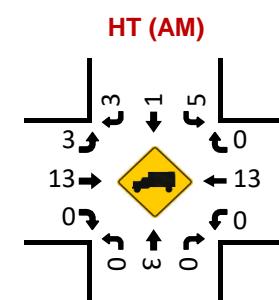
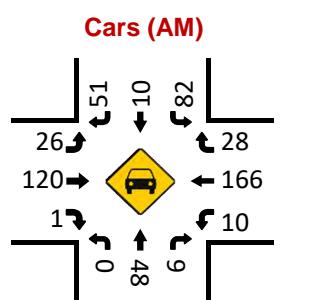
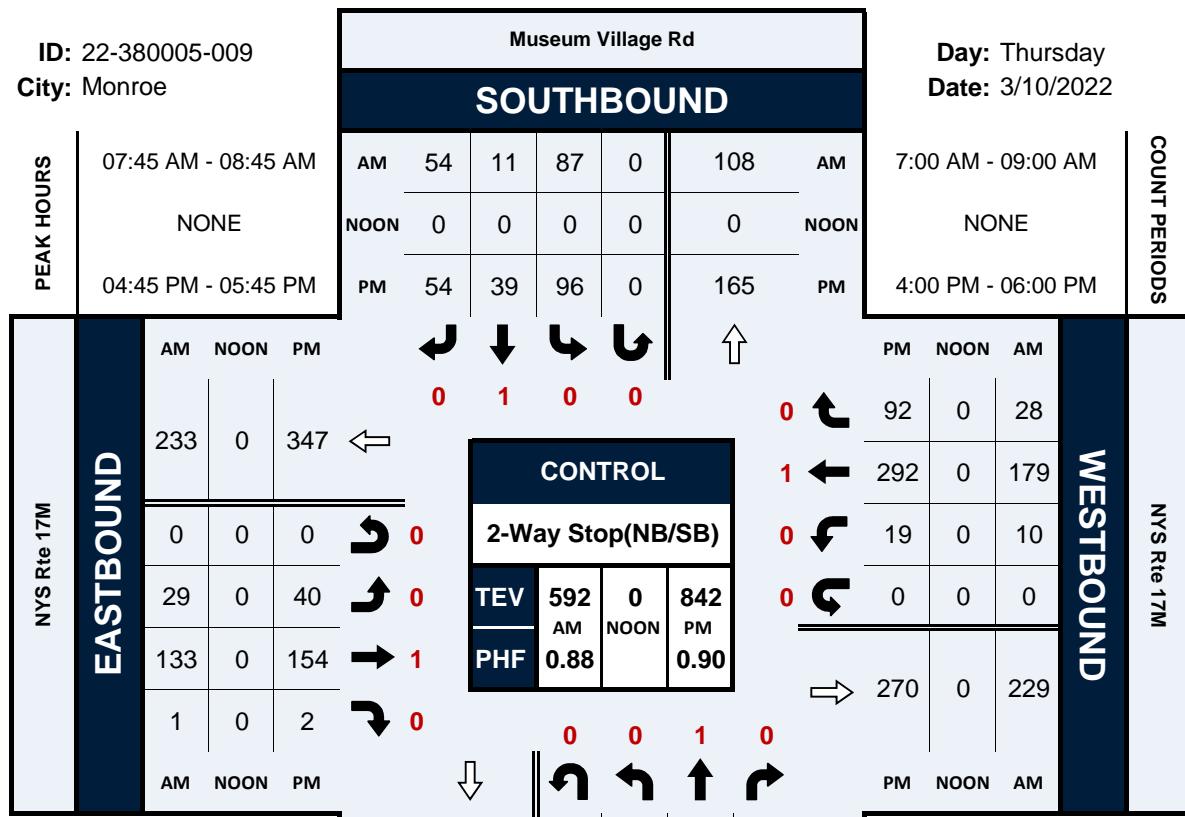
NS/EW Streets:	Museum Village Rd		Museum Village Rd		NYS Rte 17M		NYS Rte 17M		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>07:45 AM - 08:45 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

**Museum Village Rd & NYS Rte 17M****Peak Hour Turning Movement Count**

**ID:** 22-380005-009  
**City:** Monroe

**Day:** Thursday  
**Date:** 3/10/2022



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-010

**Date:** 3/10/2022

## Data - Total

NS/EW Streets:	Rte 208				Rte 208				Museum Village Rd S				Museum Village Rd S				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	2	58	0	0	0	164	13	0	20	0	4	0	0	0	0	0	261
7:15 AM	1	52	0	0	0	166	17	0	10	0	7	0	0	0	0	0	253
7:30 AM	1	87	0	0	0	225	22	0	16	0	7	0	0	0	0	0	358
7:45 AM	3	85	0	0	0	198	26	0	27	0	6	0	0	0	0	0	345
8:00 AM	1	78	0	0	0	172	9	0	17	0	3	0	0	0	0	0	280
8:15 AM	0	71	0	0	0	203	19	0	21	0	7	0	0	0	0	0	321
8:30 AM	1	84	0	0	0	184	20	0	13	0	5	0	0	0	0	0	307
8:45 AM	2	84	0	0	0	208	14	0	15	0	11	0	0	0	0	0	334
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	11	599	0	0	0	1520	140	0	139	0	50	0	0	0	0	0	2459
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																TOTAL
<b>PEAK HR VOL :</b>	5	321	0	0	0	798	76	0	81	0	23	0	0	0	0	0	1304
<b>PEAK HR FACTOR :</b>	0.417	0.922	0.000	0.000	0.000	0.887	0.731	0.000	0.750	0.000	0.821	0.000	0.000	0.000	0.000	0.000	0.911
	0.926				0.885				0.788								
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0	1	0	0	0	1	0	0	0	1	0	0	0	0	0	0	TOTAL
4:00 PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:15 PM	3	192	0	0	0	119	16	0	26	0	4	0	0	0	0	0	360
4:30 PM	5	181	0	0	0	140	14	0	35	0	5	0	0	0	0	0	380
4:45 PM	3	202	0	0	0	143	17	0	45	0	4	0	0	0	0	0	414
5:00 PM	1	228	0	0	0	117	17	0	37	0	8	0	0	0	0	0	408
5:15 PM	7	227	0	0	0	105	18	0	33	0	10	0	0	0	0	0	400
5:30 PM	5	264	0	0	0	113	18	0	36	0	8	0	0	0	0	0	444
5:45 PM	6	262	0	0	0	141	18	0	39	0	5	0	0	0	0	0	471
	0	215	0	0	0	119	12	0	35	0	2	0	0	0	0	0	383
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	30	1771	0	0	0	997	130	0	286	0	46	0	0	0	0	0	3260
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																TOTAL
<b>PEAK HR VOL :</b>	19	981	0	0	0	476	71	0	145	0	31	0	0	0	0	0	1723
<b>PEAK HR FACTOR :</b>	0.679	0.929	0.000	0.000	0.000	0.844	0.986	0.000	0.929	0.000	0.775	0.000	0.000	0.000	0.000	0.000	0.915
	0.929				0.860				0.978								

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-010

**Date:** 3/10/2022

## Data - Cars

NS/EW Streets:	Rte 208				Rte 208				Museum Village Rd S				Museum Village Rd S				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	2	54	0	0	0	158	13	0	20	0	4	0	0	0	0	0	251
7:15 AM	0	49	0	0	0	158	16	0	8	0	7	0	0	0	0	0	238
7:30 AM	1	82	0	0	0	216	21	0	15	0	7	0	0	0	0	0	342
7:45 AM	2	76	0	0	0	186	25	0	21	0	6	0	0	0	0	0	316
8:00 AM	1	71	0	0	0	165	9	0	15	0	3	0	0	0	0	0	264
8:15 AM	0	58	0	0	0	193	19	0	15	0	5	0	0	0	0	0	290
8:30 AM	1	68	0	0	0	175	17	0	10	0	3	0	0	0	0	0	274
8:45 AM	1	79	0	0	0	196	12	0	14	0	9	0	0	0	0	0	311
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	8	537	0	0	0	1447	132	0	118	0	44	0	0	0	0	0	2286
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	4	287	0	0	0	760	74	0	66	0	21	0	0	0	0	0	1212
<b>PEAK HR FACTOR :</b>	0.500	0.875	0.000	0.000	0.000	0.880	0.740	0.000	0.786	0.000	0.750	0.000	0.000	0.000	0.000	0.886	
	0.877				0.880				0.806								
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0	1	0	0	0	1	0	0	0	1	0	0	0	0	0	0	TOTAL
4:00 PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	348
4:15 PM	2	188	0	0	0	115	15	0	24	0	4	0	0	0	0	0	348
4:30 PM	5	176	0	0	0	134	12	0	31	0	5	0	0	0	0	0	363
4:45 PM	3	198	0	0	0	133	16	0	42	0	4	0	0	0	0	0	396
5:00 PM	0	223	0	0	0	111	17	0	36	0	8	0	0	0	0	0	395
5:15 PM	7	223	0	0	0	98	18	0	33	0	10	0	0	0	0	0	389
5:30 PM	5	259	0	0	0	111	16	0	36	0	8	0	0	0	0	0	435
5:45 PM	6	259	0	0	0	138	15	0	38	0	5	0	0	0	0	0	461
	0	212	0	0	0	114	12	0	33	0	2	0	0	0	0	0	373
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	28	1738	0	0	0	954	121	0	273	0	46	0	0	0	0	0	3160
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	18	964	0	0	0	458	66	0	143	0	31	0	0	0	0	0	1680
<b>PEAK HR FACTOR :</b>	0.643	0.931	0.000	0.000	0.000	0.830	0.917	0.000	0.941	0.000	0.775	0.000	0.000	0.000	0.000	0.911	
	0.926				0.856				0.989								

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

City: Monroe

Project ID: 22-380005-010

Date: 3/10/2022

## Data - HT

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	
4:00 PM	1 0	4 5	0 0	0 0	0 0	4 6	1 2	0 0	2 4	0 0	12 17						
4:15 PM	0 0	5 4	0 0	0 0	0 0	10 6	1 0	0 0	3 1	0 0	18 13						
4:30 PM	0 1	4 5	0 0	0 0	0 0	10 6	1 0	0 0	3 1	0 0	18 13						
4:45 PM	0 1	5 5	0 0	0 0	0 0	6 6	0 0	0 0	1 1	0 0	13						
5:00 PM	0 0	4 5	0 0	0 0	0 0	7 2	0 2	0 0	11 9								
5:15 PM	0 0	5 3	0 0	0 0	0 0	2 3	2 3	0 0	0 1	0 0	10 10						
5:30 PM	0 0	3 3	0 0	0 0	0 0	3 5	3 0	0 0	2 2	0 0	10 10						
5:45 PM	0 0	3 3	0 0	0 0	0 0	5 0	0 0	0 0	2 2	0 0	10 10						
TOTAL VOLUMES :	NL 2	NT 33	NR 0	NU 0	SL 0	ST 43	SR 9	SU 0	EL 13	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 100
APPROACH %'s :	5.71%	94.29%	0.00%	0.00%	0.00%	82.69%	17.31%	0.00%	100.00%	0.00%	0.00%	0.00%					
PEAK HR :	<b>04:45 PM - 05:45 PM</b>																TOTAL
PEAK HR VOL :	1 0.250	17 0.850	0 0.000	0 0.000	0 0.000	18 0.643	5 0.417	0 0.000	2 0.500	0 0.000	43 0.827						
PEAK HR FACTOR :																	

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

City: Monroe

Project ID: 22-380005-010

Date: 3/10/2022

Data - Bikes

# National Data & Surveying Services

## Intersection Turning Movement Count

**Location:** Rte 208 & Museum Village Rd S  
**City:** Monroe

**Project ID:** 22-380005-010  
**Date:** 3/10/2022

### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Rte 208		Rte 208		Museum Village Rd S		Museum Village Rd S		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>07:30 AM - 08:30 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

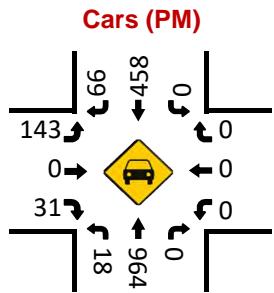
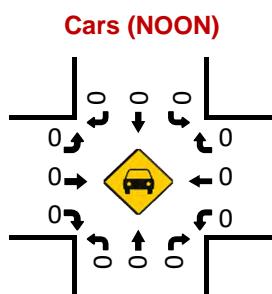
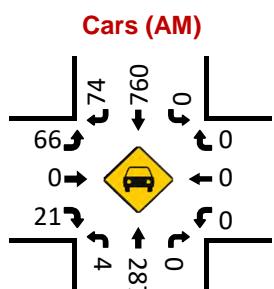
<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>0</b>

## Rte 208 & Museum Village Rd S

## Peak Hour Turning Movement Count

**ID:** 22-380005-010  
**City:** Monroe

		Rte 208												
		SOUTHBOUND												
PEAK HOURS	07:30 AM - 08:30 AM			AM	76	798	0	0	402	AM	7:00 AM - 09:00 AM			COUNT PERIODS
	NONE			NOON	0	0	0	0	0	NOON	NONE			
	04:45 PM - 05:45 PM			PM	71	476	0	0	1126	PM	4:00 PM - 06:00 PM			
EASTBOUND		AM	NOON	PM						PM NOON AM			WESTBOUND	
Museum Village Rd S		81	0	90		0	1	0	0	0	0	0	Museum Village Rd S	
		0	0	0		0	0	0	0	0	0	0		
		81	0	145		0	0	0	0	0	0	0		
		0	0	0		1	0	0	0	0	0	0		
		23	0	31		0	0	0	0	0	0	0		
CONTROL		1-Way Stop(EB)												
TEV		1304		0	1723									
PHF		AM 0.91		NOON	PM 0.91									
							0	0	1	0	0	0		
							AM	NOON	PM	AM	NOON	AM		



The diagram illustrates pedestrian movement patterns across a crosswalk, centered around a yellow diamond sign with a walking figure. The grid is defined by vertical and horizontal lines. Arrows indicate the direction of pedestrian flow:

- Top row: Right arrow (AM), Left arrow (PM)
- Middle row: Right arrow (NOON), Left arrow (NOON)
- Bottom row: Upward arrow (AM), Upward arrow (PM), Left arrow (NOON), Right arrow (AM)
- Outer border: Diagonal arrows pointing up-right (NOON), Diagonal arrows pointing down-left (PM), Diagonal arrows pointing up-left (AM), Diagonal arrows pointing down-right (NOON)

Time markers are placed at the intersections of the grid lines:

- Top row: PM, NOON, AM
- Middle row: NOON, AM, NOON, PM
- Bottom row: AM, NOON, PM, AM
- Outer border: NOON, PM, AM, NOON, PM, NOON, AM, NOON

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-011

**Date:** 3/10/2022

## Data - Total

NS/EW Streets:	Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	0	0	0	0	0	0	0	0	21	0	0	0	25	0	0	46
7:15 AM	0	0	0	0	0	0	0	0	0	38	0	0	0	32	0	0	70
7:30 AM	0	0	0	0	0	0	0	0	0	30	0	0	0	37	0	0	67
7:45 AM	0	0	0	0	0	0	0	0	0	33	0	0	0	44	0	0	77
8:00 AM	0	0	0	0	0	0	0	0	0	30	0	0	0	37	0	0	67
8:15 AM	0	0	0	0	0	0	0	0	0	35	0	0	0	52	1	0	88
8:30 AM	0	0	0	0	1	0	0	0	0	35	0	0	0	53	0	0	89
8:45 AM	0	0	0	0	0	0	0	0	0	52	0	0	0	42	0	0	94
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	0	0	0	1	0	0	0	0	274	0	0	0	322	1	0	598
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	1	0	0	0	0	152	0	0	0	184	1	0	338
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.731	0.000	0.000	0.000	0.868	0.250	0.000	0.899
<b>PM</b>	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0	0	0	0	0	1	0	0	0	1	0	0	0	1	0	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 PM	0	0	0	0	0	0	0	0	0	43	0	0	0	77	1	0	121
4:15 PM	0	0	0	0	0	0	2	0	2	44	0	0	0	69	0	0	117
4:30 PM	0	0	0	0	2	0	0	0	0	28	0	0	0	62	1	0	93
4:45 PM	0	0	0	0	1	0	0	0	0	35	0	0	0	99	0	0	135
5:00 PM	0	0	0	0	0	0	0	0	0	41	0	0	0	88	0	0	129
5:15 PM	0	0	0	0	1	0	0	0	0	44	0	0	0	83	0	0	128
5:30 PM	0	0	0	0	0	0	0	0	0	31	0	0	0	65	0	0	96
5:45 PM	0	0	0	0	0	0	0	0	0	33	0	0	0	60	0	0	93
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	0	0	0	4	0	2	0	0	299	0	0	0	603	2	0	912
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	2	0	0	0	0	151	0	0	0	335	0	0	488
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0.858	0.000	0.000	0.000	0.846	0.000	0.000	0.904

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

### **Control: No Control**

**Project ID:** 22-380005-011

Date: 3/10/2022

## Data - Cars

NS/EW Streets:		Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	0	0	0	0	0	0	0	0	0	0	20	0	0	0	22	0	0	42
7:15 AM	0	0	0	0	0	0	0	0	0	0	34	0	0	0	30	0	0	64
7:30 AM	0	0	0	0	0	0	0	0	0	0	27	0	0	0	32	0	0	59
7:45 AM	0	0	0	0	0	0	0	0	0	0	31	0	0	0	41	0	0	72
8:00 AM	0	0	0	0	0	0	0	0	0	0	27	0	0	0	37	0	0	64
8:15 AM	0	0	0	0	0	0	0	0	0	0	34	0	0	0	49	1	0	84
8:30 AM	0	0	0	0	1	0	0	0	0	0	32	0	0	0	43	0	0	76
8:45 AM	0	0	0	0	0	0	0	0	0	0	50	0	0	0	38	0	0	88
TOTAL VOLUMES : APPROACH %'s :		NL 0	NT 0	NR 0	NU 0	SL 1	ST 0	SR 0	SU 0	EL 0	ET 255	ER 0	EU 0	WL 0	WT 292	WR 1	WU 0	TOTAL 549
PEAK HR :	08:00 AM - 09:00 AM																TOTAL	
PEAK HR VOL :	0	0	0	0	1	0	0	0	0	143	0	0	0	167	1	0	312	
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.715	0.000	0.000	0.000	0.852	0.250	0.000	0.886	
					0.250					0.715					0.840			

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
4:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 2	0 0	0 2	41 41	0 0	0 0	0 0	76 66	1 0	0 0	118 111
4:15 PM	0 0	0 0	0 0	0 0	0 0	0 1	0 0	0 0	0 0	28 34	0 0	0 0	0 0	58 95	1 0	0 0	89 130
4:30 PM	0 0	0 0	0 0	0 0	2 1	0 0	0 0	0 0	0 0	34 31	0 0	0 0	0 0	95 59	0 0	0 0	130
4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	39 31	0 0	0 0	0 0	84 59	0 0	0 0	123 90
5:00 PM	0 0	0 0	0 0	0 0	0 1	0 0	0 0	0 0	0 0	39 43	0 0	0 0	0 0	84 81	0 0	0 0	125
5:15 PM	0 0	0 0	0 0	0 0	1 0	0 0	0 0	0 0	0 0	30 30	0 0	0 0	0 0	63 63	0 0	0 0	93
5:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	30 31	0 0	0 0	0 0	63 59	0 0	0 0	90
TOTAL VOLUMES : APPROACH %'s :	NL 0	NT 0	NR 0	NU 0	SL 4	ST 0	SR 2	SU 0	EL 2	ET 287	ER 0	EU 0	WL 0	WT 582	WR 2	WU 0	TOTAL 879
PEAK HR :	<b>04:45 PM - 05:45 PM</b>				2 0.500	0 0.000	0 0.000	0 0.000	0 0.000	146 0.849	0 0.000	0 0.000	0 0.000	323 0.850	0 0.000	0 0.000	TOTAL 471
PEAK HR VOL :	0 0.000	0 0.000	0 0.000	0 0.000	2 0.500	0 0.000	0 0.000	0 0.000	0 0.000	146 0.849	0 0.000	0 0.000	0 0.000	323 0.850	0 0.000	0 0.000	471 0.906
PEAK HR FACTOR :																	

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-011

**Date:** 3/10/2022

## Data - HT

NS/EW Streets:	Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
7:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	3	0	0	4	
7:15 AM	0	0	0	0	0	0	0	0	0	4	0	0	0	2	0	0	6	
7:30 AM	0	0	0	0	0	0	0	0	0	3	0	0	0	5	0	0	8	
7:45 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	3	0	0	5	
8:00 AM	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	3	
8:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	3	0	0	4	
8:30 AM	0	0	0	0	0	0	0	0	0	3	0	0	0	10	0	0	13	
8:45 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	4	0	0	6	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 19	ER 0	EU 0	WL 0	WT 30	WR 0	WU 0	<b>TOTAL 49</b>	
<b>APPROACH %'s :</b>										0.00%	100.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>																<b>TOTAL</b>	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	9	0	0	0	17	0	0	<b>26</b>	
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.750	0.000	0.000	0.000	0.425	0.000	0.000	0.425	<b>0.500</b>
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
	0	0	0	0	0	1	0	0	0	1	0	0	0	1	0	0	TOTAL	
4:00 PM	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 2	ER 0	EU 0	WL 0	WT 1	WR 0	WU 0	<b>TOTAL 3</b>	
4:15 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	3	0	0	6	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	
4:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	4	0	0	5	
5:00 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	4	0	0	6	
5:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	2	0	0	3	
5:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	2	0	0	3	
5:45 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	1	0	0	3	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 12	ER 0	EU 0	WL 0	WT 21	WR 0	WU 0	<b>TOTAL 33</b>	
<b>APPROACH %'s :</b>										0.00%	100.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL</b>	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	5	0	0	0	12	0	0	<b>17</b>	
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.625	0.000	0.000	0.000	0.750	0.000	0.000	0.750	<b>0.708</b>

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

#### **Control:** No Control

**Project ID:** 22-380005-011

Date: 3/10/2022

Data - Bikes

NS/EW Streets:		Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M				
AM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
		NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>		NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0
<b>PEAK HR :</b>		<b>08:00 AM - 09:00 AM</b>																<b>TOTAL 0</b>
<b>PEAK HR VOL :</b>		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	<b>TOTAL 0</b>
<b>PEAK HR FACTOR :</b>		0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	<b>TOTAL 0</b>

PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL	
	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU		
4:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
4:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:00 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:15 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:30 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
5:45 PM	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0 0	0	
<b>TOTAL VOLUMES : APPROACH %'s :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	TOTAL 0	
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	0.000 0.000	<b>TOTAL 0</b>

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M      **Project ID:** 22-380005-011  
**City:** Chester      **Date:** 3/10/2022

## Data - Pedestrians (Crosswalks)

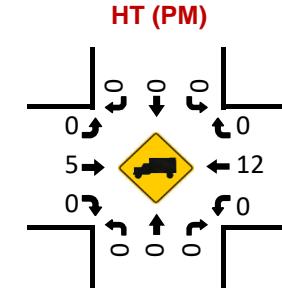
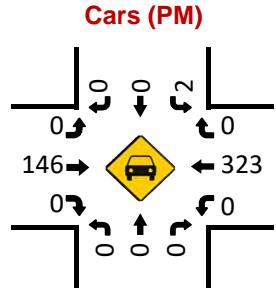
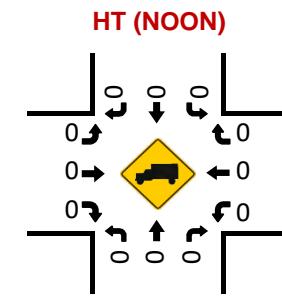
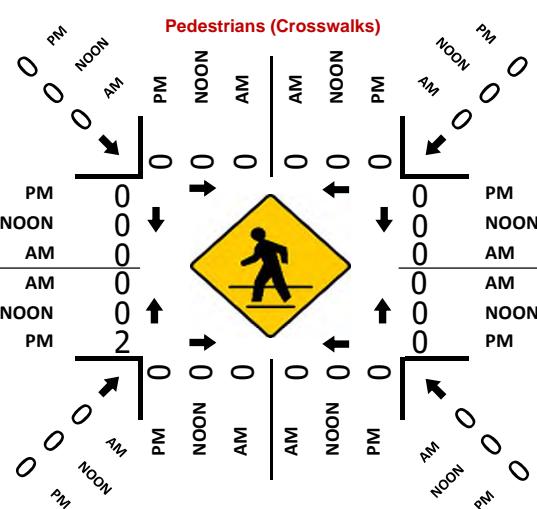
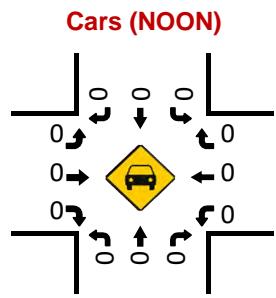
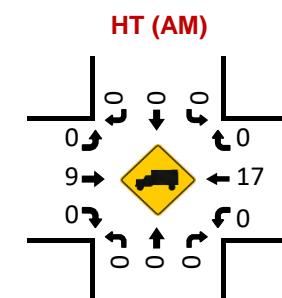
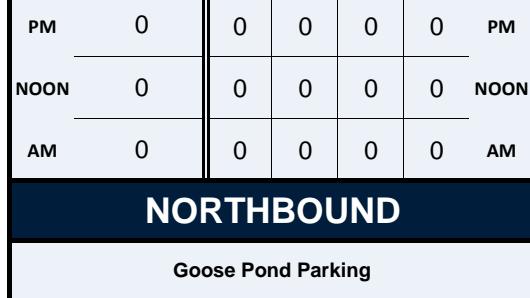
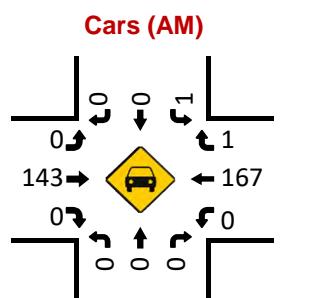
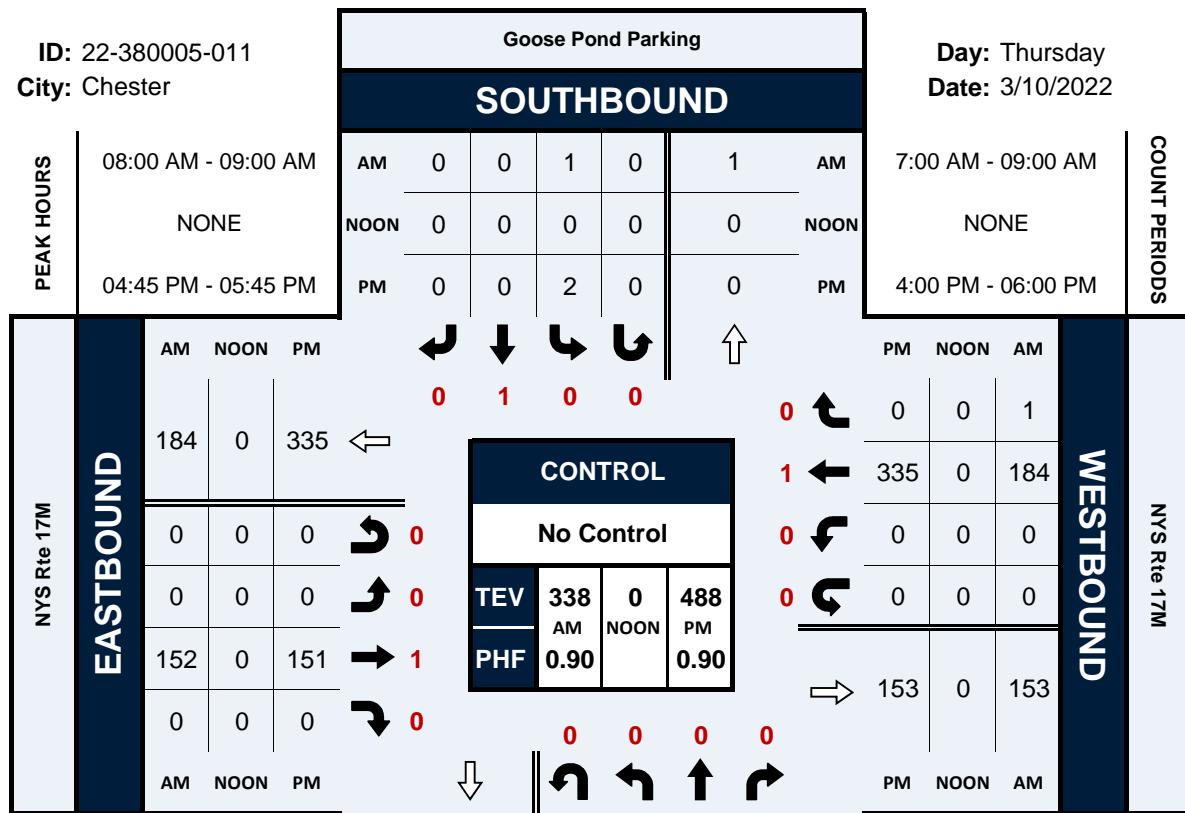
NS/EW Streets:	Goose Pond Parking		Goose Pond Parking		NYS Rte 17M		NYS Rte 17M		
<b>AM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									<b>TOTAL</b>

<b>PM</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	1	1
4:15 PM	0	0	0	0	0	0	1	0	1
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	1	0	1
5:00 PM	0	0	0	0	0	0	1	0	1
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
	0	0	0	0	0	0	3	1	4
							75.00%	25.00%	
<b>PEAK HR :</b>	<b>04:45 PM - 05:45 PM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	2	0	2
<b>PEAK HR FACTOR :</b>							0.500		0.500

**Goose Pond Parking & NYS Rte 17M****Peak Hour Turning Movement Count**

**ID:** 22-380005-011  
**City:** Chester

**Day:** Thursday  
**Date:** 3/10/2022



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M

**City:** Chester

**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001

**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	Craigville Rd				Craigville Rd				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
11:00 AM	0	0	0	0	7	0	2	0	2	26	0	0	0	46	4	0	87
11:15 AM	0	0	0	0	10	0	6	0	0	22	0	0	0	48	11	0	97
11:30 AM	0	0	0	0	3	0	5	0	1	29	0	0	0	46	12	0	96
11:45 AM	0	0	0	0	6	0	2	0	2	23	0	0	0	54	6	0	93
12:00 PM	0	0	0	0	3	0	4	0	0	34	0	0	0	44	6	0	91
12:15 PM	0	0	0	0	5	0	2	0	1	31	0	0	0	49	11	0	99
12:30 PM	0	0	0	0	7	0	1	0	1	24	0	0	0	35	6	0	74
12:45 PM	0	0	0	0	3	0	4	0	0	29	0	0	0	38	7	0	81
1:00 PM	0	0	0	0	5	0	3	0	1	28	0	0	0	41	4	0	82
1:15 PM	0	0	0	0	5	0	2	0	0	27	0	0	0	37	5	0	76
1:30 PM	0	0	0	0	6	0	3	0	1	26	0	0	0	45	9	0	90
1:45 PM	0	0	0	0	5	0	3	0	0	19	0	0	0	53	7	0	87
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 65 63.73%	ST 0 0.00%	SR 37 36.27%	SU 0 0.00%	EL 9 2.75%	ET 318 97.25%	ER 0 0.00%	EU 0 0.00%	WL 0 0.00%	WT 536 85.90%	WR 88 14.10%	WU 0 0.00%	TOTAL 1053
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	17 0.708	0 0.000	13 0.650	0 0.000	4 0.500	117 0.860	0 0.000	0 0.000	0 0.000	193 0.894	35 0.729	0 0.000	379 0.957
<b>PEAK HR FACTOR :</b>	0.938				0.890				0.890				0.950				

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M

**City:** Chester

**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	Craigville Rd				Craigville Rd				NYS Rte 17M				NYS Rte 17M					
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL	
11:00 AM	0	0	0	0	6	0	2	0	2	24	0	0	0	45	3	0	82	
11:15 AM	0	0	0	0	10	0	6	0	0	22	0	0	0	47	11	0	96	
11:30 AM	0	0	0	0	3	0	5	0	1	29	0	0	0	45	12	0	95	
11:45 AM	0	0	0	0	6	0	2	0	2	21	0	0	0	53	6	0	90	
12:00 PM	0	0	0	0	3	0	4	0	0	34	0	0	0	40	6	0	87	
12:15 PM	0	0	0	0	5	0	2	0	1	30	0	0	0	49	11	0	98	
12:30 PM	0	0	0	0	7	0	1	0	1	24	0	0	0	35	6	0	74	
12:45 PM	0	0	0	0	3	0	4	0	0	29	0	0	0	38	7	0	81	
1:00 PM	0	0	0	0	5	0	3	0	1	25	0	0	0	40	2	0	76	
1:15 PM	0	0	0	0	5	0	2	0	0	26	0	0	0	37	4	0	74	
1:30 PM	0	0	0	0	6	0	3	0	1	26	0	0	0	45	7	0	88	
1:45 PM	0	0	0	0	5	0	3	0	0	19	0	0	0	53	7	0	87	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 64 63.37%	ST 0 0.00%	SR 37 36.63%	SU 0 0.00%	EL 9 2.83%	ET 309 97.17%	ER 0 0.00%	EU 0 0.00%	WL 0 0.00%	WT 527 86.54%	WR 82 13.46%	WU 0 0.00%	TOTAL 1028	
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	17 0.708 0.938	0 0.000	13 0.650	0 0.000	4 0.500	114 0.838 0.868	0 0.000	0 0.000	0 0.000	187 0.882 0.925	35 0.729 0.925	0 0.000	370 0.944	
<b>PEAK HR FACTOR :</b>																		

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M

**City:** Chester

**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001

**Date:** 3/26/2022

## Data - HT

NS/EW Streets:	Craigville Rd				Craigville Rd				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	1	0	0	0	0	2	0	0	0	1	1	0	5
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	1	0	0	3
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
12:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	1	2	0	6
1:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	2
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 1	ST 0	SR 0	SU 0	EL 0	ET 9	ER 0	EU 0	WL 0	WT 9	WR 6	WU 0	<b>TOTAL 25</b>
<b>APPROACH %'s :</b>	100.00% 0.00% 0.00% 0.00%				0.00% 100.00% 0.00% 0.00%				0.00% 60.00% 40.00% 0.00%				0.00% 0.375 0.375 0.375				<b>TOTAL</b>
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	3	0	0	0	6	0	0	<b>9</b>
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.375	0.000	0.000	0.000	0.375	0.000	0.000	<b>0.563</b>

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & NYS Rte 17M

**City:** Chester

**Control:** 1-Way Stop(SB)

**Project ID:** 22-380005-001

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Craigville Rd				Craigville Rd				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 1	WR 0	WU 0	<b>TOTAL 1</b>
<b>APPROACH %'s :</b>													0.00% 100.00% 0.00% 0.00%				
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1 0.250	0 0.000	0 0.000	<b>1 0.250</b>
<b>PEAK HR FACTOR :</b>																	

# National Data & Surveying Services Intersection Turning

Location: Craigville Rd & NYS Rte 17M  
 City: Chester

Project ID: 22-380005-001  
 Date: 3/26/2022

## Movement Count

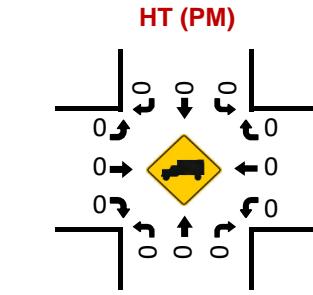
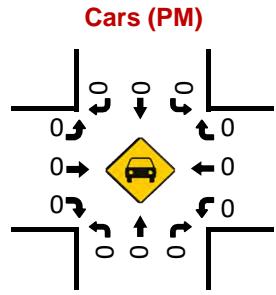
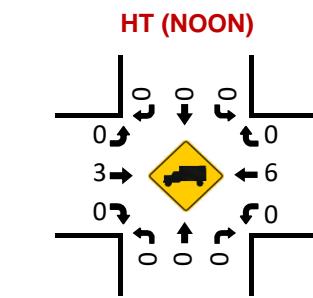
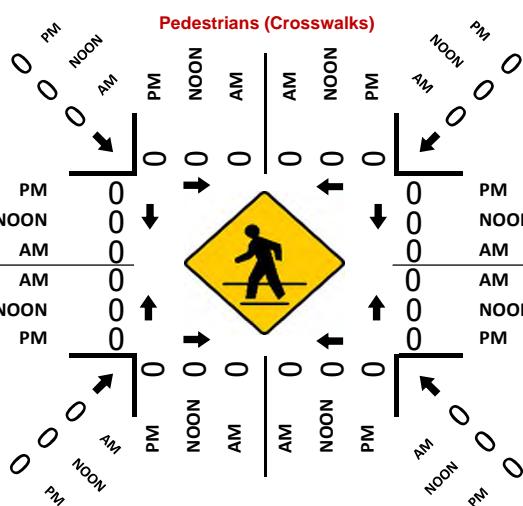
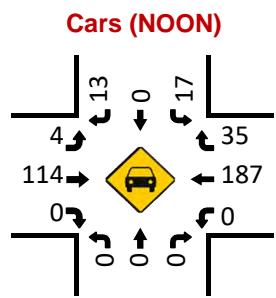
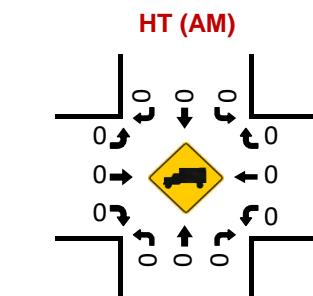
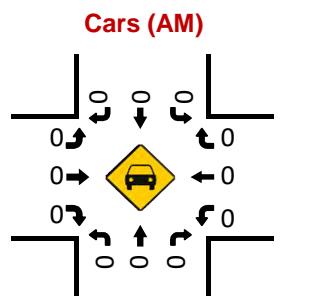
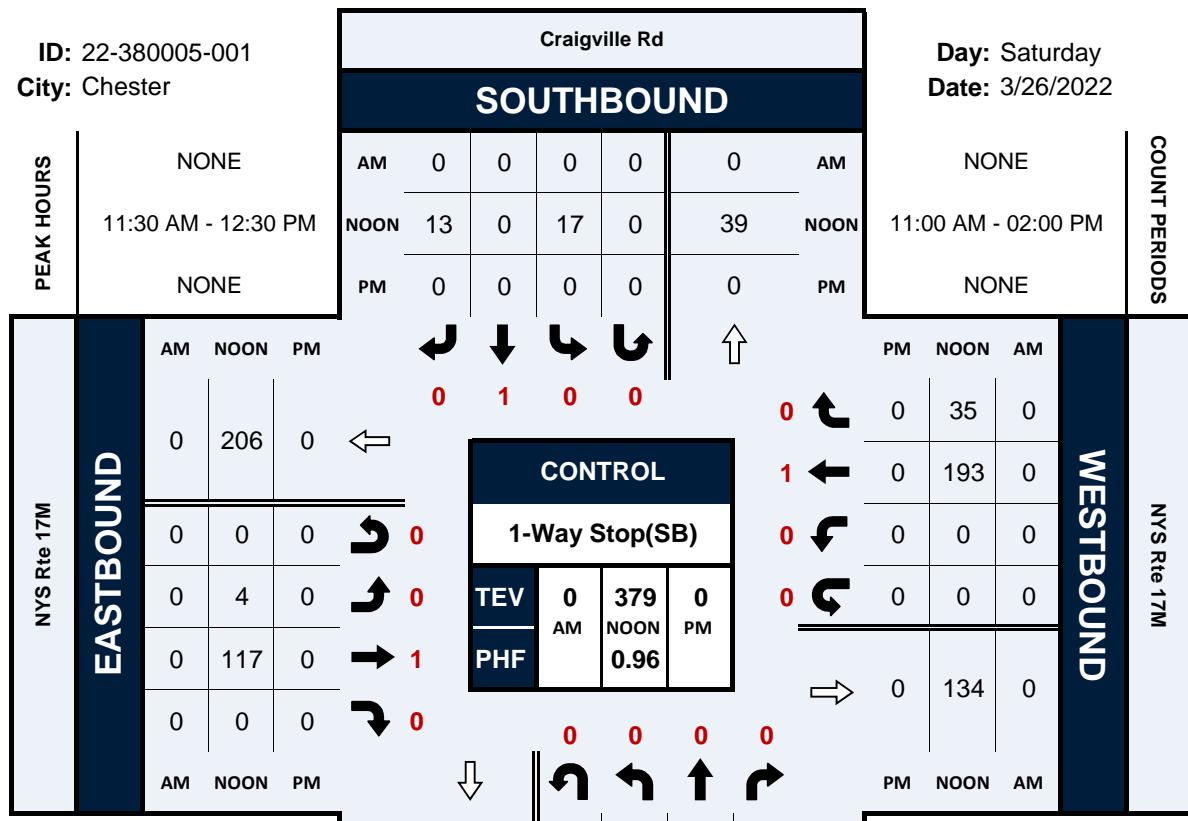
### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Craigville Rd		Craigville Rd		NYS Rte 17M		NYS Rte 17M		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
NOON	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
APPROACH %'s :	0	0	0	0	0	0	0	0	0
PEAK HR :	12:00 AM - 01:00 AM								TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :									

**Craigville Rd & NYS Rte 17M****Peak Hour Turning Movement Count**

**ID:** 22-380005-001  
**City:** Chester

**Day:** Saturday  
**Date:** 3/26/2022



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd

**City:** Chester

**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002

**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	Craigville Rd				Craigville Rd				Old Mansion Rd				Old Mansion Rd				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	3	4	0	15	9	0	0	0	0	0	0	2	0	16	0	49
11:15 AM	0	10	0	0	18	5	0	0	0	0	0	0	2	0	11	0	46
11:30 AM	0	13	1	0	18	7	0	0	0	0	0	0	1	0	7	0	47
11:45 AM	0	5	0	0	23	6	0	0	0	0	0	0	2	0	16	0	52
12:00 PM	0	5	3	0	15	3	0	0	0	0	0	0	1	0	19	0	46
12:15 PM	0	8	0	0	14	5	0	0	0	0	0	0	1	0	13	0	41
12:30 PM	0	9	2	0	8	7	0	0	0	0	0	0	1	0	9	0	36
12:45 PM	0	10	0	0	27	3	0	0	0	0	0	0	2	0	5	0	47
1:00 PM	0	5	4	0	16	7	0	0	0	0	0	0	1	0	14	0	47
1:15 PM	0	7	3	0	13	6	0	0	0	0	0	0	1	0	14	0	44
1:30 PM	0	9	3	0	9	6	0	0	0	0	0	0	5	0	11	0	43
1:45 PM	0	12	0	0	12	6	0	0	0	0	0	0	0	0	17	0	47
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	96	20	0	188	70	0	0	0	0	0	0	19	0	152	0	545
<b>PEAK HR :</b>	<b>11:00 AM - 12:00 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	31	5	0	74	27	0	0	0	0	0	0	7	0	50	0	194
<b>PEAK HR FACTOR :</b>	0.000	0.596	0.313	0.000	0.804	0.750	0.000	0.000	0.000	0.000	0.000	0.000	0.875	0.000	0.781	0.000	0.933
					0.643					0.871							

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd

**City:** Chester

**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	Craigville Rd				Craigville Rd				Old Mansion Rd				Old Mansion Rd				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	2	4	0	15	8	0	0	0	0	0	0	2	0	16	0	47
11:15 AM	0	10	0	0	18	5	0	0	0	0	0	0	2	0	11	0	46
11:30 AM	0	13	1	0	17	7	0	0	0	0	0	0	1	0	7	0	46
11:45 AM	0	5	0	0	23	6	0	0	0	0	0	0	2	0	14	0	50
12:00 PM	0	5	3	0	14	3	0	0	0	0	0	0	1	0	19	0	45
12:15 PM	0	8	0	0	14	5	0	0	0	0	0	0	1	0	13	0	41
12:30 PM	0	9	2	0	8	7	0	0	0	0	0	0	1	0	9	0	36
12:45 PM	0	10	0	0	27	3	0	0	0	0	0	0	2	0	5	0	47
1:00 PM	0	3	4	0	16	7	0	0	0	0	0	0	1	0	14	0	45
1:15 PM	0	6	3	0	13	6	0	0	0	0	0	0	1	0	13	0	42
1:30 PM	0	9	3	0	9	5	0	0	0	0	0	0	5	0	11	0	42
1:45 PM	0	10	0	0	12	6	0	0	0	0	0	0	0	0	17	0	45
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	90	20	0	186	68	0	0	0	0	0	0	19	0	149	0	532
PEAK HR :	<b>11:00 AM - 12:00 PM</b>				73.23% 0.793 0.625	26.77% 0.813 0.853	0.00% 0.000 0.000	0.00% 0.000 0.000	0.000 0.000 0.000	0.000 0.000 0.000	0.000 0.000 0.000	0.000 0.000 0.000	11.31% 0.875 0.764	0.00% 0.000 0.750	88.69% 0.000 0.000	0.00% 0.000 0.000	TOTAL
PEAK HR VOL :	0	30	5	0									7	0	48	0	189
PEAK HR FACTOR :	0.000	0.577	0.313	0.000									0.875	0.000	0.750	0.000	0.945

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd

**City:** Chester

**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002

**Date:** 3/26/2022

## Data - HT

NS/EW Streets:	Craigville Rd				Craigville Rd				Old Mansion Rd				Old Mansion Rd				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2
12:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
1:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	2
1:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
1:45 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	6	0	0	2	2	0	0	0	0	0	0	0	0	3	0	13
<b>PEAK HR :</b>	<b>11:00 AM - 12:00 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	1	0	0	1	1	0	0	0	0	0	0	0	0	2	0	5
<b>PEAK HR FACTOR :</b>	0.000	0.250	0.000	0.000	0.250	0.250	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.625

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & Old Mansion Rd

**City:** Chester

**Control:** 1-Way Stop(WB)

**Project ID:** 22-380005-002

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Craigville Rd				Craigville Rd				Old Mansion Rd				Old Mansion Rd				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	0 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 0</b>
<b>APPROACH %'s :</b>																	
<b>PEAK HR :</b>	<b>11:00 AM - 12:00 PM</b>																<b>TOTAL 0</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>
<b>PEAK HR FACTOR :</b>																	

# National Data & Surveying Services Intersection Turning

Location: Craigville Rd & Old Mansion Rd  
 City: Chester

Project ID: 22-380005-002  
 Date: 3/26/2022

## Movement Count

### Data - Pedestrians (Crosswalks)

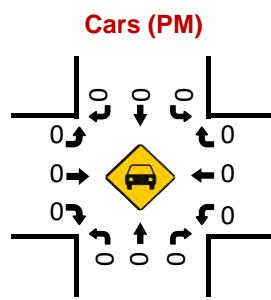
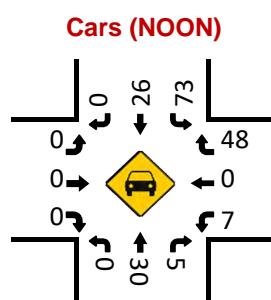
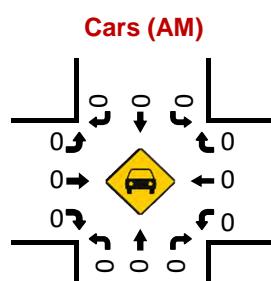
NS/EW Streets:	Craigville Rd		Craigville Rd		Old Mansion Rd		Old Mansion Rd		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
NOON	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
APPROACH %'s :	0	0	0	0	0	0	0	0	0
PEAK HR :	12:00 AM - 01:00 AM								TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :									

## Craigville Rd & Old Mansion Rd

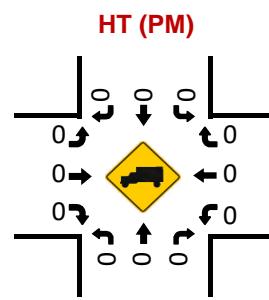
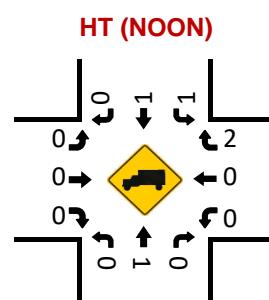
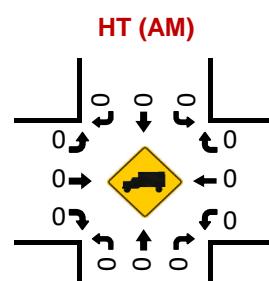
# Peak Hour Turning Movement Count

**ID:** 22-380005-002  
**City:** Chester

Craigville Rd			Southbound			Day: Saturday Date: 3/26/2022			
PEAK HOURS	NONE						NONE		
	11:00 AM - 12:00 PM			AM 0 0 0 0   0 AM			11:00 AM - 02:00 PM		
	NONE			NOON 0 27 74 0   81 NOON			NONE		
	PM 0 0 0 0   0 PM						PM		
Old Mansion Rd	AM NOON PM						PM NOON AM		
	0	0	0		0	1	0		0
	0	0	0		0		1		0
	0	0	0		0		0		79
	0	0	0		0		0		0
EASTBOUND	AM NOON PM						PM NOON AM		
	0	0	0		0	1	0		0
	0	0	0		0		1		0
	0	0	0		0		0		0
	0	0	0		0		0		0
WESTBOUND	AM NOON PM						PM NOON AM		
	0	50	0		0	0	0		0
	0	0	0		0	0	0		0
	0	7	0		0	0	0		0
	0	0	0		0	0	0		0



**Pedestrians (Crosswalks)**



National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-003

Date: 3/26/2022

## Data - Total

NS/EW Streets:	Craigville Rd				Craigville Rd				WB Exit 128 Off-Ramp				WB Exit 128 Off-Ramp				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	
11:00 AM	0	5	0	0	0	8	0	0	1	0	1	0	0	0	0	0	15
11:15 AM	0	11	0	0	0	13	0	0	1	0	1	0	0	0	0	0	26
11:30 AM	0	13	0	0	0	7	0	0	3	0	1	0	0	0	0	0	24
11:45 AM	0	8	0	0	0	8	0	0	0	0	0	0	0	0	0	0	16
12:00 PM	0	6	0	0	0	4	0	0	1	0	3	0	0	0	0	0	14
12:15 PM	0	12	0	0	0	6	0	0	1	0	1	0	0	0	0	0	20
12:30 PM	0	7	0	0	0	8	0	0	1	0	0	0	0	0	0	0	16
12:45 PM	0	7	0	0	0	6	0	0	2	0	0	0	0	0	0	0	15
1:00 PM	0	5	0	0	0	7	0	0	1	0	1	0	0	0	0	0	14
1:15 PM	0	5	0	0	0	7	0	0	4	0	0	0	0	0	0	0	16
1:30 PM	0	10	0	0	0	8	0	0	2	0	1	0	0	0	0	0	21
1:45 PM	0	7	0	0	0	8	0	0	1	0	1	0	0	0	0	0	17
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	0	96	0	0	0	90	0	0	18	0	10	0	0	0	0	0	214
0.00% 100.00% 0.00% 0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	64.29%	0.00%	35.71%	0.00%					
PEAK HR :	11:00 AM - 12:00 PM																TOTAL
PEAK HR VOL :	0	37	0	0	0	36	0	0	5	0	3	0	0	0	0	0	81
PEAK HR FACTOR :	0.000	0.712	0.000	0.000	0.000	0.692	0.000	0.000	0.417	0.000	0.750	0.000	0.000	0.000	0.000	0.000	0.779
					0.712												

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-003

Date: 3/26/2022

## Data - Cars

NS/EW Streets:	Craigville Rd				Craigville Rd				WB Exit 128 Off-Ramp				WB Exit 128 Off-Ramp				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	TOTAL
11:00 AM	0	4	0	0	0	7	0	0	1	0	1	0	0	0	0	0	13
11:15 AM	0	11	0	0	0	13	0	0	1	0	1	0	0	0	0	0	26
11:30 AM	0	13	0	0	0	7	0	0	3	0	1	0	0	0	0	0	24
11:45 AM	0	8	0	0	0	8	0	0	0	0	0	0	0	0	0	0	16
12:00 PM	0	6	0	0	0	4	0	0	1	0	3	0	0	0	0	0	14
12:15 PM	0	12	0	0	0	6	0	0	1	0	1	0	0	0	0	0	20
12:30 PM	0	7	0	0	0	8	0	0	1	0	0	0	0	0	0	0	16
12:45 PM	0	7	0	0	0	6	0	0	2	0	0	0	0	0	0	0	15
1:00 PM	0	3	0	0	0	7	0	0	1	0	1	0	0	0	0	0	12
1:15 PM	0	4	0	0	0	7	0	0	4	0	0	0	0	0	0	0	15
1:30 PM	0	8	0	0	0	8	0	0	2	0	1	0	0	0	0	0	19
1:45 PM	0	7	0	0	0	8	0	0	1	0	1	0	0	0	0	0	17
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	90	0	0	0	89	0	0	18	0	10	0	0	0	0	0	207
<b>PEAK HR :</b>	<b>11:00 AM - 12:00 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	36	0	0	0	35	0	0	5	0	3	0	0	0	0	0	<b>79</b>
<b>PEAK HR FACTOR :</b>	0.000	0.692	0.000	0.000	0.000	0.673	0.000	0.000	0.417	0.000	0.750	0.000	0.000	0.000	0.000	0.000	<b>0.760</b>

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-003

Date: 3/26/2022

## Data - HT

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Craigville Rd & WB Exit 128 Off-Ramp

**City:** Chester

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-003

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Craigville Rd				Craigville Rd				WB Exit 128 Off-Ramp				WB Exit 128 Off-Ramp				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 0</b>
<b>APPROACH %'s :</b>																	
<b>PEAK HR :</b>	<b>11:00 AM - 12:00 PM</b>																<b>TOTAL 0</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>
<b>PEAK HR FACTOR :</b>																	

# National Data & Surveying Services Intersection Turning

**Location:** Craigville Rd & WB Exit 128 Off-Ramp  
**City:** Chester

**Project ID:** 22-380005-003  
**Date:** 3/26/2022

## Movement Count

### Data - Pedestrians (Crosswalks)

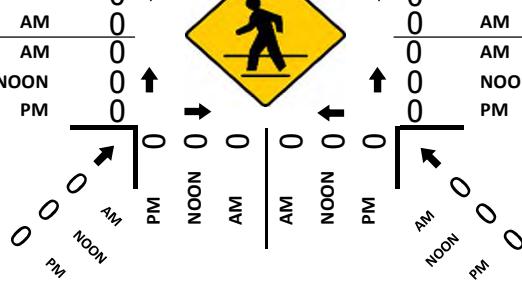
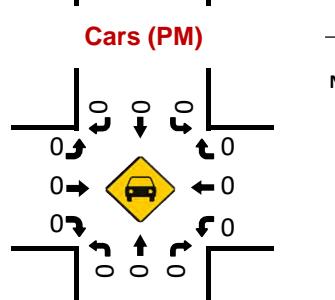
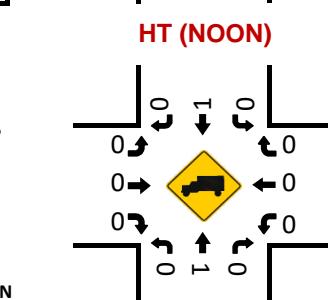
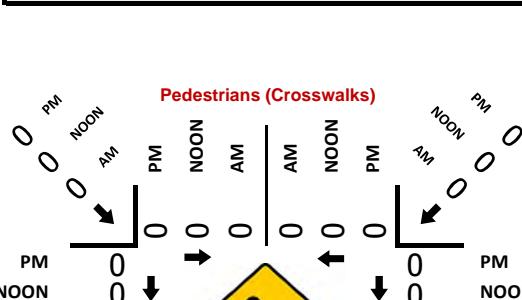
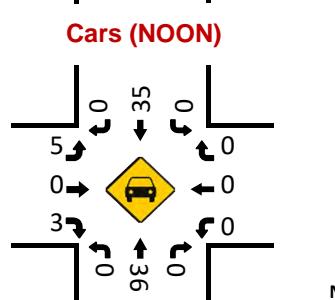
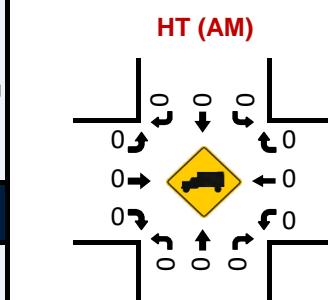
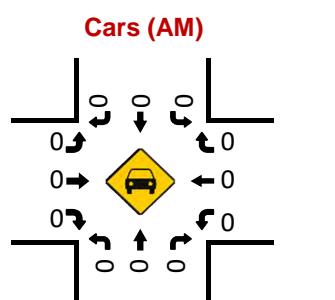
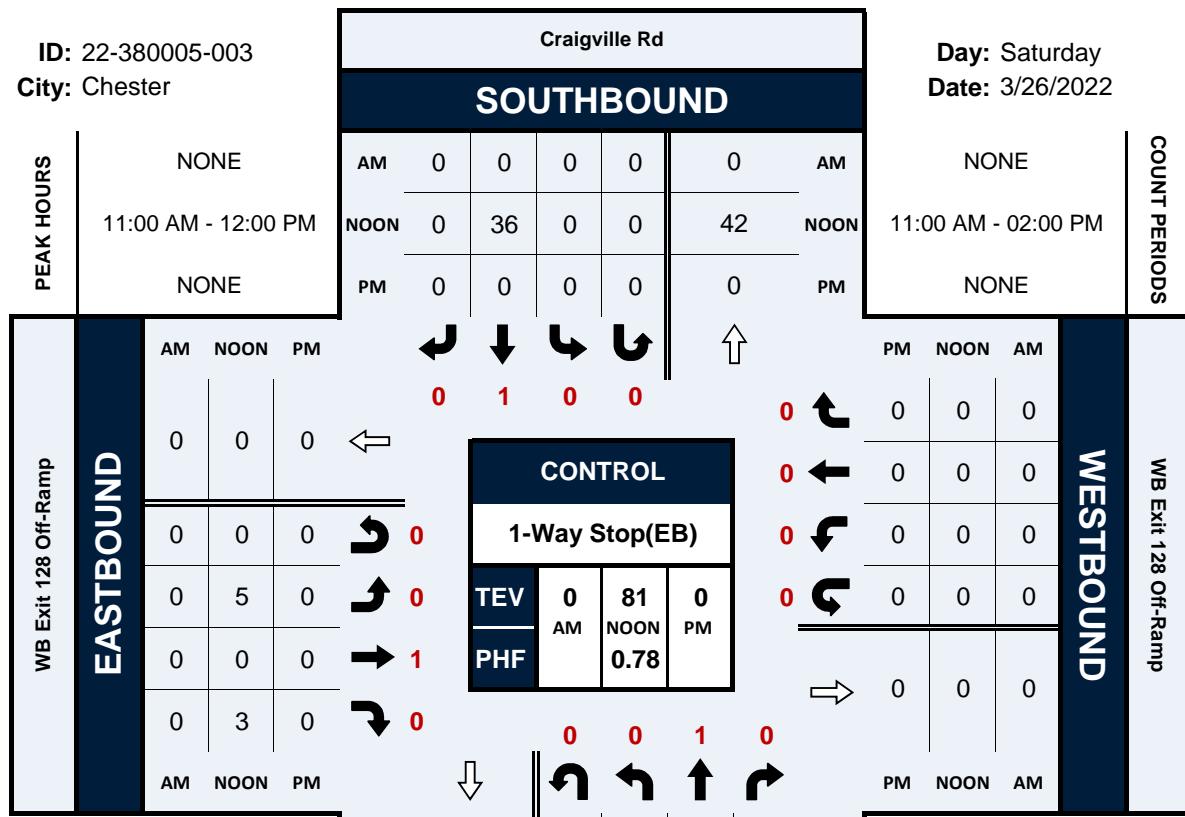
NS/EW Streets:	Craigville Rd		Craigville Rd		WB Exit 128 Off-Ramp		WB Exit 128 Off-Ramp		
<b>NOON</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	EB	WB	EB	WB	NB	SB	NB	SB	<b>TOTAL</b>
<b>APPROACH %'s :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR :</b>	<b>12:00 AM - 01:00 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0
<b>PEAK HR FACTOR :</b>									

## Craigville Rd & WB Exit 128 Off-Ramp

### Peak Hour Turning Movement Count

ID: 22-380005-003  
City: Chester

Day: Saturday  
Date: 3/26/2022



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-004

**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	41	28	0	0	0	40	1	0	110
11:15 AM	0	0	0	0	0	0	0	0	31	24	0	0	0	58	2	0	115
11:30 AM	0	0	0	0	0	0	0	0	37	29	0	0	0	48	1	0	115
11:45 AM	0	0	0	0	0	0	0	0	40	29	0	0	0	58	1	0	128
12:00 PM	0	0	0	0	0	0	0	0	43	32	0	0	0	46	0	0	121
12:15 PM	0	0	0	0	0	0	0	0	49	30	0	0	0	49	0	0	128
12:30 PM	0	0	0	0	0	0	0	0	29	30	0	0	0	38	0	0	97
12:45 PM	0	0	0	0	0	0	0	0	33	25	0	0	0	43	1	0	102
1:00 PM	0	0	0	0	0	0	0	0	34	31	0	0	0	47	0	0	112
1:15 PM	0	0	0	0	0	0	0	0	34	26	0	0	0	37	0	0	97
1:30 PM	0	0	0	0	0	0	0	0	38	24	0	0	0	46	0	0	108
1:45 PM	0	0	0	0	0	0	0	0	35	22	0	0	0	59	1	0	117
TOTAL VOLUMES :	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 444 57.36%	ET 330 42.64%	ER 0 0.00%	EU 0 0.00%	WL 0 0.00%	WT 569 98.78%	WR 7 1.22%	WU 0 0.00%	TOTAL 1350
PEAK HR :	<b>11:30 AM - 12:30 PM</b>																TOTAL
PEAK HR VOL :	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	169 0.862 0.915	120 0.938 0.915	0 0.000 0.915	0 0.000 0.915	0 0.000	201 0.866 0.860	2 0.500 0.500	0 0.000 0.961	492 0.961
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.862	0.938	0.000	0.000	0.000	0.866	0.500	0.000	

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-004

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	40	26	0	0	0	40	1	0	107
11:15 AM	0	0	0	0	0	0	0	0	31	23	0	0	0	57	1	0	112
11:30 AM	0	0	0	0	0	0	0	0	35	29	0	0	0	48	1	0	113
11:45 AM	0	0	0	0	0	0	0	0	39	28	0	0	0	55	1	0	123
12:00 PM	0	0	0	0	0	0	0	0	41	32	0	0	0	44	0	0	117
12:15 PM	0	0	0	0	0	0	0	0	48	29	0	0	0	47	0	0	124
12:30 PM	0	0	0	0	0	0	0	0	29	30	0	0	0	38	0	0	97
12:45 PM	0	0	0	0	0	0	0	0	32	25	0	0	0	43	1	0	101
1:00 PM	0	0	0	0	0	0	0	0	34	28	0	0	0	46	0	0	108
1:15 PM	0	0	0	0	0	0	0	0	33	25	0	0	0	37	0	0	95
1:30 PM	0	0	0	0	0	0	0	0	38	24	0	0	0	46	0	0	108
1:45 PM	0	0	0	0	0	0	0	0	34	22	0	0	0	59	1	0	116
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 434 57.48%	ET 321 42.52%	ER 0 0.00%	EU 0 0.00%	WL 0	WT 560 98.94%	WR 6 1.06%	WU 0 0.00%	TOTAL 1321
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	163 0.849	118 0.922	0 0.000	0 0.000	0 0.000	194 0.882	2 0.500	0 0.000	477 0.962
<b>PEAK HR FACTOR :</b>																	

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-004

**Date:** 3/26/2022

## Data - HT

NS/EW Streets:	NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	1	2	0	0	0	0	0	0	3
11:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1	0	3
11:30 AM	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	2
11:45 AM	0	0	0	0	0	0	0	0	1	1	0	0	0	3	0	0	5
12:00 PM	0	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	4
12:15 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	2	0	0	4
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1
1:00 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	1	0	0	4
1:15 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	2
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 10 52.63%	ET 9 47.37%	ER 0 0.00%	EU 0 0.00%	WL 0 0.00%	WT 9 90.00%	WR 1 10.00%	WU 0 0.00%	<b>TOTAL</b> 29
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	6 0.750	2 0.500	0 0.000	0 0.000	0 0.000	7 0.583	0 0.000	0 0.000	<b>15</b> 0.750
<b>PEAK HR FACTOR :</b>																	

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-004

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M Exit 127 EB On-Ramp				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 1	WR 0	WU 0	<b>TOTAL 1</b>
<b>APPROACH %'s :</b>													0.00% 100.00% 0.00% 0.00%				
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1 0.250	0 0.000	0 0.000	<b>1 0.250</b>
<b>PEAK HR FACTOR :</b>																	

# National Data & Surveying Services Intersection Turning Movement Count

Location: NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M  
 City: Chester

Project ID: 22-380005-004  
 Date: 3/26/2022

## Data - Pedestrians (Crosswalks)

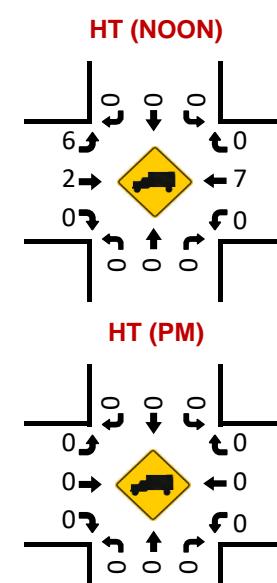
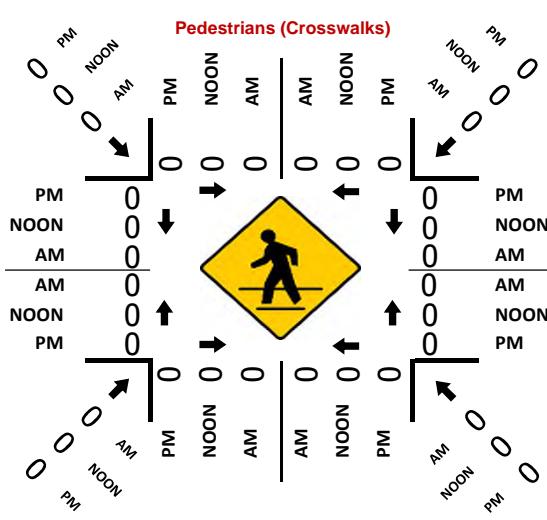
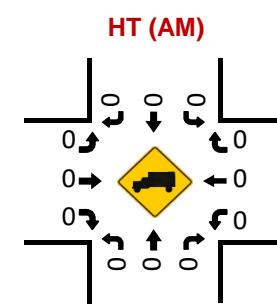
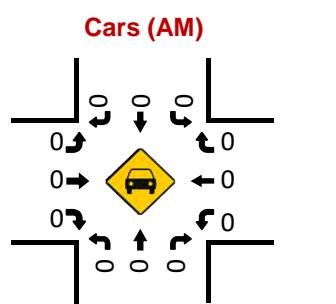
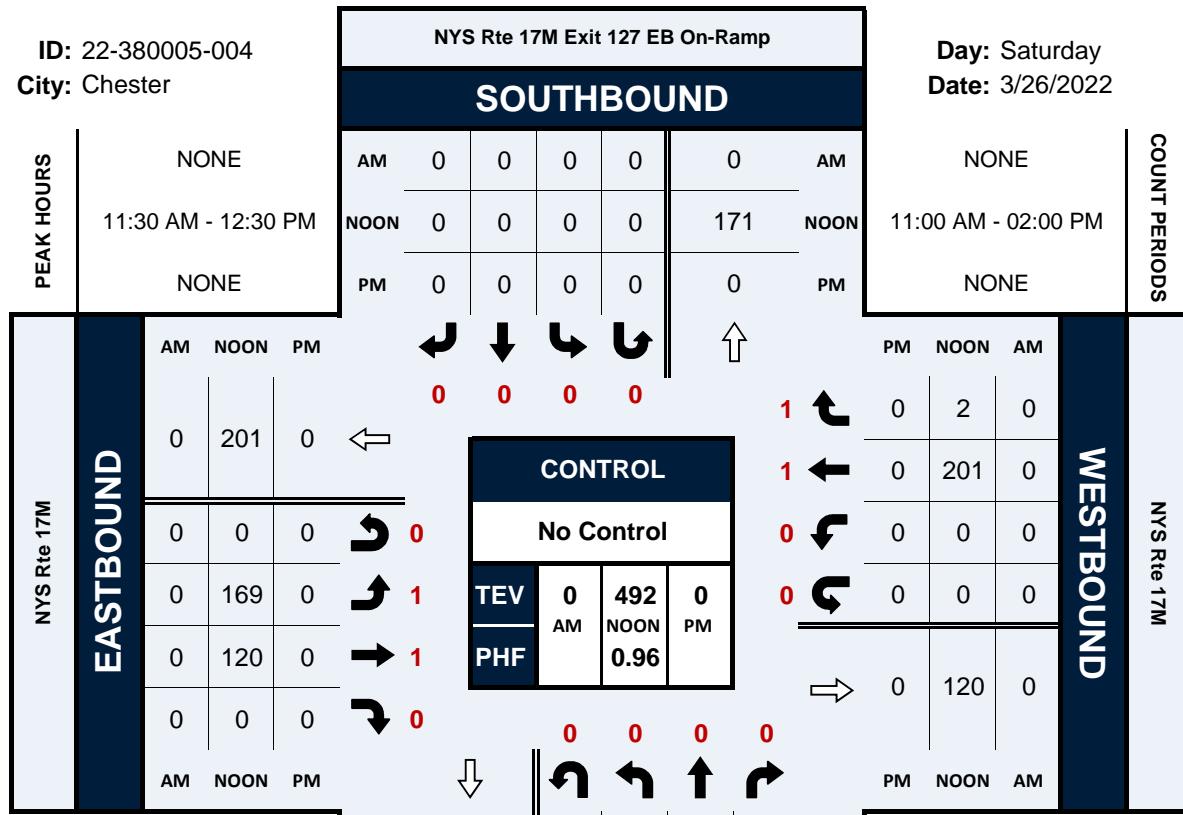
NS/EW Streets:	NYS Rte 17M Exit 127 EB On-Ramp		NYS Rte 17M Exit 127 EB On-Ramp		NYS Rte 17M		NYS Rte 17M		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
NOON	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
APPROACH %'s :	0	0	0	0	0	0	0	0	0
PEAK HR :	12:00 AM - 01:00 AM								TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :									

# NYS Rte 17M Exit 127 EB On-Ramp & NYS Rte 17M

## Peak Hour Turning Movement Count

ID: 22-380005-004  
City: Chester

Day: Saturday  
Date: 3/26/2022



National Data & Surveying Services Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester  
**Control:** Signalized

**Project ID:** 22-380005-005  
**Date:** 3/26/2022

## Data - Total

NS/EW Streets:		Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M				
NOON		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
		0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
11:00 AM	96	0	37	0	0	1	34	15	0	5	34	83	0	2	38	0	0	345
11:15 AM	82	0	31	0	0	1	27	15	0	11	34	76	0	4	52	0	0	333
11:30 AM	104	3	25	0	1	15	18	0	8	38	84	0	11	37	0	0	0	344
11:45 AM	100	2	28	0	1	27	21	0	8	48	95	0	10	58	0	0	0	398
12:00 PM	107	1	30	0	5	26	22	0	8	49	94	0	12	42	0	0	0	396
12:15 PM	89	2	36	0	5	26	14	0	13	33	87	0	6	45	0	0	0	356
12:30 PM	107	0	26	0	1	23	29	0	9	36	103	0	8	30	3	0	0	375
12:45 PM	91	3	32	0	0	41	17	0	6	34	105	0	9	38	0	0	0	376
1:00 PM	79	2	29	0	2	45	15	0	5	37	90	0	12	39	0	0	0	355
1:15 PM	106	1	36	0	4	26	8	0	4	24	80	0	7	37	0	0	0	333
1:30 PM	74	0	39	0	3	44	13	0	6	28	73	0	11	40	0	0	0	331
1:45 PM	80	0	27	0	4	31	15	0	2	30	90	0	9	52	0	0	0	340
TOTAL VOLUMES : APPROACH %'s :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL	
	1115	14	376	0	28	365	202	0	85	425	1060	0	101	508	3	0	4282	
PEAK HR :	11:45 AM - 12:45 PM				4.71%	61.34%	33.95%	0.00%	5.41%	27.07%	67.52%	0.00%	16.50%	83.01%	0.49%	0.00%	TOTAL	
PEAK HR VOL :	403	5	120	0	12	102	86	0	38	166	379	0	36	175	3	0	1525	
PEAK HR FACTOR :	0.942	0.625	0.833	0.000	0.600	0.944	0.741	0.000	0.731	0.847	0.920	0.000	0.750	0.754	0.250	0.000	0.958	
	0.957				0.943				0.965				0.787				TOTAL	

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester  
**Control:** Signalized

**Project ID:** 22-380005-005  
**Date:** 3/26/2022

Data - Cars																	
NS/EW Streets:	Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	96	0	36	0	1	31	15	0	5	33	82	0	2	38	0	0	339
11:15 AM	81	0	30	0	1	25	15	0	11	32	74	0	4	51	0	0	324
11:30 AM	102	3	24	0	1	15	18	0	8	38	84	0	11	37	0	0	341
11:45 AM	99	2	28	0	1	25	21	0	7	47	93	0	10	55	0	0	388
12:00 PM	107	1	28	0	5	26	22	0	8	49	94	0	11	41	0	0	392
12:15 PM	87	2	35	0	5	26	14	0	13	32	87	0	6	43	0	0	350
12:30 PM	104	0	26	0	1	23	29	0	9	36	101	0	8	30	3	0	370
12:45 PM	91	3	31	0	0	41	17	0	6	34	105	0	9	38	0	0	375
1:00 PM	77	2	29	0	2	42	15	0	5	34	88	0	12	38	0	0	344
1:15 PM	104	1	36	0	4	26	8	0	4	22	79	0	7	37	0	0	328
1:30 PM	73	0	39	0	3	43	13	0	6	28	73	0	11	40	0	0	329
1:45 PM	80	0	26	0	4	31	15	0	2	29	89	0	9	52	0	0	337
TOTAL VOLUMES :	NL 1101	NT 14	NR 368	NU 0	SL 28	ST 354	SR 202	SU 0	EL 84	ET 414	ER 1049	EU 0	WL 100	WT 500	WR 3	WU 0	TOTAL 4217
APPROACH %'s :	74.24%	0.94%	24.81%	0.00%	4.79%	60.62%	34.59%	0.00%	5.43%	26.76%	67.81%	0.00%	16.58%	82.92%	0.50%	0.00%	
PEAK HR :	11:45 AM - 12:45 PM																TOTAL
PEAK HR VOL :	397	5	117	0	12	100	86	0	37	164	375	0	35	169	3	0	1500
PEAK HR FACTOR :	0.928	0.625	0.836	0.000	0.600	0.962	0.741	0.000	0.712	0.837	0.928	0.000	0.795	0.768	0.250	0.000	0.957
					0.954		0.934			0.954				0.796			

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester  
**Control:** Signalized

**Project ID:** 22-380005-005  
**Date:** 3/26/2022

NS/EW Streets:	Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M				
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	TOTAL
11:00 AM	0	0	1	0	0	3	0	0	0	1	1	0	0	0	0	0	6
11:15 AM	1	0	1	0	0	2	0	0	0	2	2	0	0	1	0	0	9
11:30 AM	2	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	3
11:45 AM	1	0	0	0	0	2	0	0	1	1	2	0	0	3	0	0	10
12:00 PM	0	0	2	0	0	0	0	0	0	0	0	0	1	1	0	0	4
12:15 PM	2	0	1	0	0	0	0	0	0	1	0	0	0	2	0	0	6
12:30 PM	3	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	5
12:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
1:00 PM	2	0	0	0	0	3	0	0	0	3	2	0	0	1	0	0	11
1:15 PM	2	0	0	0	0	0	0	0	0	2	1	0	0	0	0	0	5
1:30 PM	1	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2
1:45 PM	0	0	1	0	0	0	0	0	0	1	1	0	0	0	0	0	3
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	14	0	8	0	0	11	0	0	1	11	11	0	1	8	0	0	65
<b>PEAK HR :</b>	<b>11:45 AM - 12:45 PM</b>																TOTAL
<b>PEAK HR VOL :</b>	6	0	3	0	0	2	0	0	1	2	4	0	1	6	0	0	25
<b>PEAK HR FACTOR :</b>	0.500	0.000	0.375	0.000	0.000	0.250	0.000	0.000	0.250	0.500	0.500	0.000	0.250	0.500	0.000	0.000	0.625

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Kings Hwy/Lehigh Ave & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-005

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Kings Hwy/Lehigh Ave				Kings Hwy/Lehigh Ave				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	1 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	1 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 1	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 1</b>
<b>APPROACH %'s :</b>																	
<b>PEAK HR :</b>	<b>11:45 AM - 12:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1 0.250	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>1</b>
<b>PEAK HR FACTOR :</b>																	<b>0.250</b>

National Data & Surveying Services Intersection Turning

**Location:** Kings Hwy/Lehigh Ave, MYS RT-12M  
**City:** Chester

**Project ID:** 22-380005-005  
**Date:** 3/26/2022

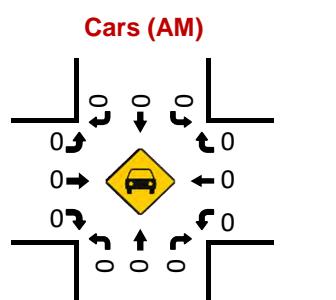
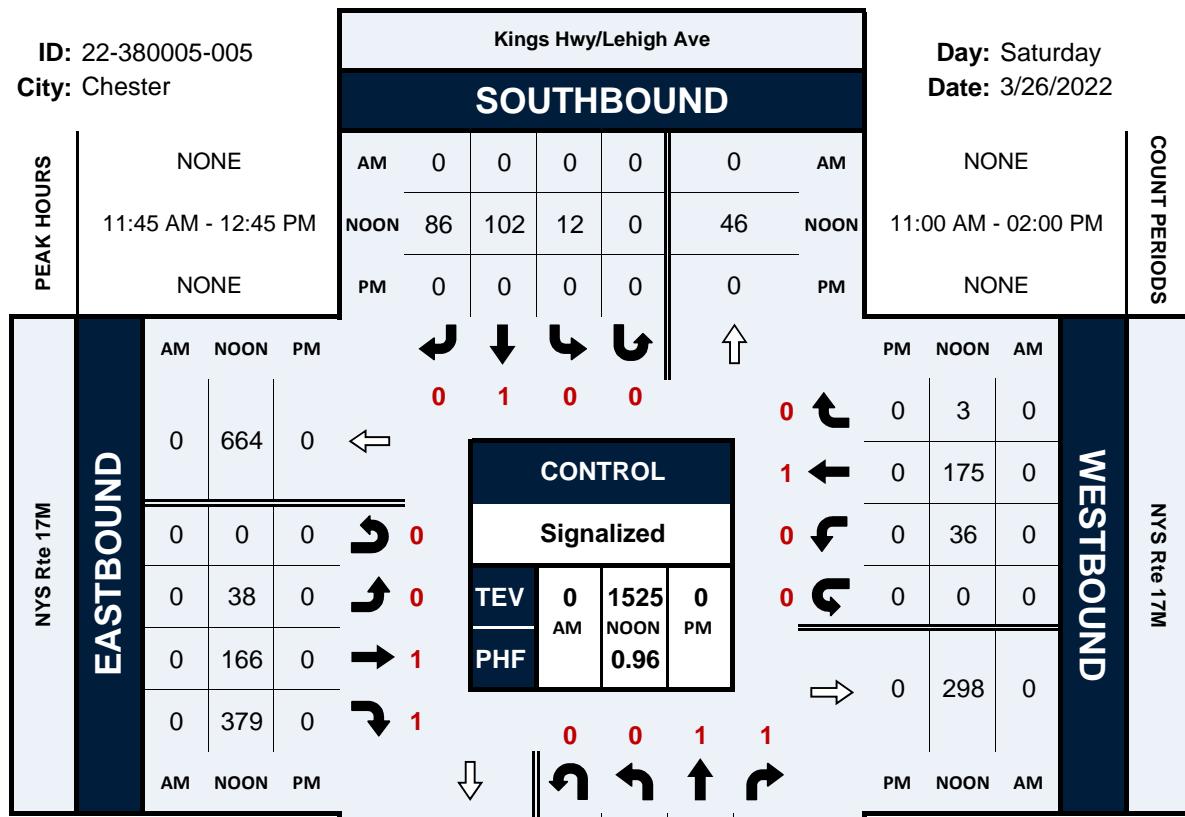
## Data - Pedestrians (Crosswalks)

NS/EW Streets:	Kings Hwy/Lehigh Ave		Kings Hwy/Lehigh Ave		NYS Rte 17M		NYS Rte 17M		
<b>NOON</b>	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		<b>TOTAL</b>
	EB	WB	EB	WB	NB	SB	NB	SB	
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES : APPROACH %'s :</b>	<b>EB</b> 0	<b>WB</b> 0	<b>EB</b> 0	<b>WB</b> 0	<b>NB</b> 0	<b>SB</b> 0	<b>NB</b> 0	<b>SB</b> 0	<b>TOTAL</b> 0
<b>PEAK HR :</b>	<b>12:00 AM - 01:00 AM</b>								<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0		0		0		0		<b>0</b>
<b>PEAK HR FACTOR :</b>									<b>0</b>

**Kings Hwy/Lehigh Ave & NYS Rte 17M****Peak Hour Turning Movement Count**

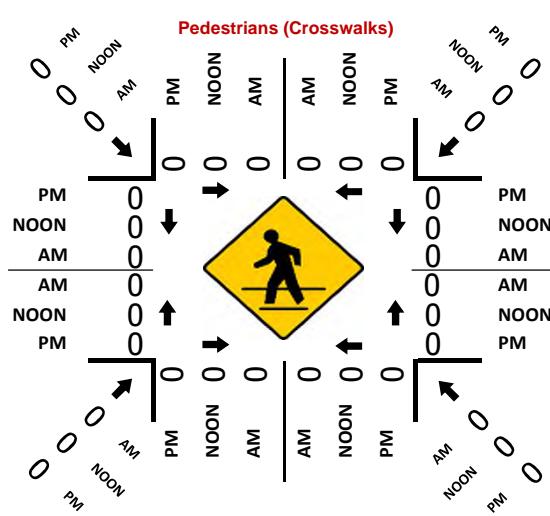
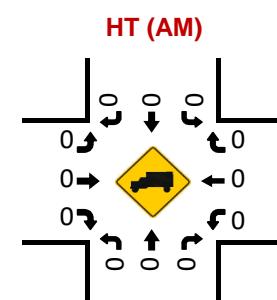
**ID:** 22-380005-005  
**City:** Chester

**Day:** Saturday  
**Date:** 3/26/2022



**NORTHBOUND**

Kings Hwy/Lehigh Ave											
NORTHBOUND											
Kings Hwy/Lehigh Ave	PM			NOON			AM			PM	
	0	0	0	0	0	0	0	0	0		0
	0	0	0	0	0	0	0	0	0		0
Kings Hwy/Lehigh Ave	PM	NOON	AM	PM	NOON	AM	PM	NOON	AM		
	0	0	0	0	0	0	0	0	0		
	0	0	0	0	0	0	0	0	0		
	0	0	0	0	0	0	0	0	0		
	0	0	0	0	0	0	0	0	0		
	0	0	0	0	0	0	0	0	0		



National Data & Surveying Services Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester  
**Control:** Signalized

**Project ID:** 22-380005-006  
**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	
11:00 AM	38	30	47	0	17	39	22	0	24	61	50	0	67	63	6	0	464
11:15 AM	43	33	63	0	17	51	34	0	19	52	40	0	70	86	6	0	514
11:30 AM	46	22	45	0	15	38	29	0	28	65	50	0	70	76	5	0	489
11:45 AM	62	30	67	0	10	45	40	0	26	76	46	0	67	78	3	0	550
12:00 PM	66	48	72	0	15	40	38	0	44	79	40	0	88	81	10	0	621
12:15 PM	60	28	54	0	16	39	42	0	24	84	65	0	62	86	4	0	564
12:30 PM	62	33	63	0	15	47	20	0	28	73	57	0	88	82	7	0	575
12:45 PM	63	24	68	0	16	34	25	0	27	77	41	0	69	64	4	0	512
1:00 PM	61	25	56	0	9	24	23	0	25	64	51	0	55	65	4	0	462
1:15 PM	55	24	45	0	13	42	26	0	29	54	46	0	77	74	2	0	487
1:30 PM	45	29	50	0	11	38	24	0	13	61	51	0	51	58	7	0	438
1:45 PM	50	33	51	0	14	36	25	0	20	61	53	0	74	74	12	0	503
TOTAL VOLUMES : APPROACH %'s :	NL 651 38.50%	NT 359 21.23%	NR 681 40.27%	NU 0 0.00%	SL 168 16.99%	ST 473 47.83%	SR 348 35.19%	SU 0 0.00%	EL 307 18.02%	ET 807 47.36%	ER 590 34.62%	EU 0 0.00%	WL 838 46.69%	WT 887 49.42%	WR 70 3.90%	WU 0 0.00%	TOTAL 6179
PEAK HR :	11:45 AM - 12:45 PM																TOTAL
PEAK HR VOL :	250	139	256	0	56	171	140	0	122	312	208	0	305	327	24	0	2310
PEAK HR FACTOR :	0.947	0.724	0.889	0.000	0.875	0.910	0.833	0.000	0.693	0.929	0.800	0.000	0.866	0.951	0.600	0.000	0.930
					0.867				0.946				0.928				0.916

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester  
**Control:** Signalized

**Project ID:** 22-380005-006  
**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M				
NOON	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				TOTAL
	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	
11:00 AM	36	30	46	0	17	38	22	0	23	61	50	0	67	63	6	0	459
11:15 AM	43	33	61	0	17	51	34	0	19	51	39	0	70	85	5	0	508
11:30 AM	46	22	44	0	15	35	29	0	28	65	50	0	69	76	5	0	484
11:45 AM	61	30	66	0	10	44	40	0	26	76	44	0	63	78	3	0	541
12:00 PM	66	47	71	0	15	40	38	0	44	79	39	0	88	80	10	0	617
12:15 PM	60	26	50	0	16	38	42	0	24	84	62	0	61	84	3	0	550
12:30 PM	62	33	61	0	15	46	20	0	28	72	57	0	88	80	6	0	568
12:45 PM	63	24	67	0	15	34	25	0	27	77	40	0	68	62	4	0	506
1:00 PM	60	24	56	0	8	23	23	0	24	61	51	0	54	65	4	0	453
1:15 PM	55	24	45	0	13	42	26	0	29	53	46	0	74	73	2	0	482
1:30 PM	45	29	50	0	11	38	23	0	13	61	51	0	50	58	7	0	436
1:45 PM	48	33	51	0	14	36	25	0	20	59	51	0	74	74	12	0	497
TOTAL VOLUMES : APPROACH %'s :	NL 645 38.67%	NT 355 21.28%	NR 668 40.05%	NU 0 0.00%	SL 166 16.97%	ST 465 47.55%	SR 347 35.48%	SU 0 0.00%	EL 305 18.11%	ET 799 47.45%	ER 580 34.44%	EU 0 0.00%	WL 826 46.64%	WT 878 49.58%	WR 67 3.78%	WU 0 0.00%	TOTAL 6101 2276 0.922
PEAK HR :	11:45 AM - 12:45 PM																TOTAL
PEAK HR VOL :	249	136	248	0	56	168	140	0	122	311	202	0	300	322	22	0	2276
PEAK HR FACTOR :	0.943	0.723	0.873	0.000	0.875	0.913	0.833	0.000	0.693	0.926	0.815	0.000	0.852	0.958	0.550	0.000	0.922
					0.860				0.948				0.934				0.904

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-006

**Date:** 3/26/2022

## Data - HT

NS/EW Streets:	Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU	
11:00 AM	2	0	1	0	0	1	0	0	1	0	0	0	0	0	0	0	5
11:15 AM	0	0	2	0	0	0	0	0	0	1	1	0	0	0	1	1	6
11:30 AM	0	0	1	0	0	3	0	0	0	0	0	0	1	0	0	0	5
11:45 AM	1	0	1	0	0	1	0	0	0	0	2	0	4	0	0	0	9
12:00 PM	0	1	1	0	0	0	0	0	0	0	1	0	0	1	0	0	4
12:15 PM	0	2	4	0	0	1	0	0	0	0	3	0	1	2	1	0	14
12:30 PM	0	0	2	0	0	1	0	0	0	1	0	0	0	2	1	0	7
12:45 PM	0	0	1	0	1	0	0	0	0	0	1	0	1	2	0	0	6
1:00 PM	1	1	0	0	1	1	0	0	1	3	0	0	1	0	0	0	9
1:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	3	1	0	0	5
1:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	0	2
1:45 PM	2	0	0	0	0	0	0	0	0	2	2	0	0	0	0	0	6
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	6	4	13	0	2	8	1	0	2	8	10	0	12	9	3	0	78
	26.09%	17.39%	56.52%	0.00%	18.18%	72.73%	9.09%	0.00%	10.00%	40.00%	50.00%	0.00%	50.00%	37.50%	12.50%	0.00%	
<b>PEAK HR :</b>	<b>11:45 AM - 12:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	1	3	8	0	0	3	0	0	0	1	6	0	5	5	2	0	34
<b>PEAK HR FACTOR :</b>	0.250	0.375	0.500	0.000	0.000	0.750	0.000	0.000	0.000	0.250	0.500	0.000	0.313	0.625	0.500	0.000	0.607
	0.500				0.750				0.583				0.750				

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Academy Ave/Rte 94 & NYS Rte 17M

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-006

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Academy Ave/Rte 94				Academy Ave/Rte 94				NYS Rte 17M				NYS Rte 17M				TOTAL	
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
NOON	1 NL	1 NT	1 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	1 EL	1.5 ET	0.5 ER	0 EU	2 WL	0.5 WT	0.5 WR	0 WU		
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 1	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 1</b>	
<b>APPROACH %'s :</b>									100.00% 0.00% 0.00% 0.00%									
<b>PEAK HR :</b>	<b>11:45 AM - 12:45 PM</b>																<b>TOTAL 0</b>	
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000		

# National Data & Surveying Services Intersection Turning

Location: Academy Ave/Rte 94 & NYS Rte 17M  
 City: Chester

Project ID: 22-380005-006  
 Date: 3/26/2022

## Movement Count

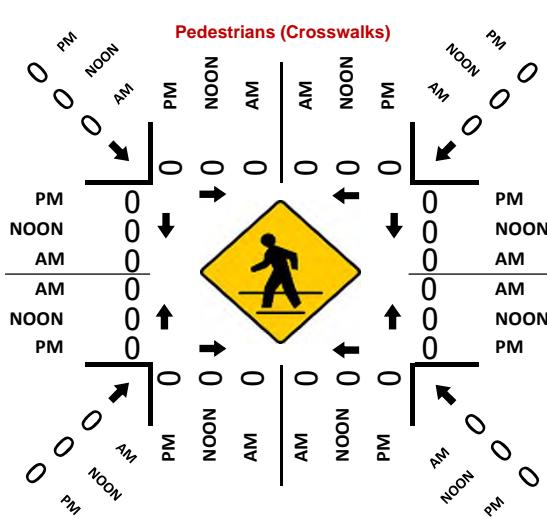
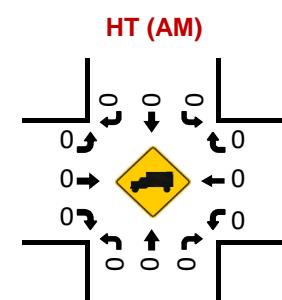
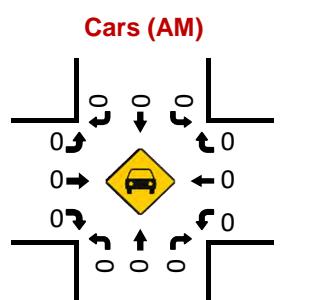
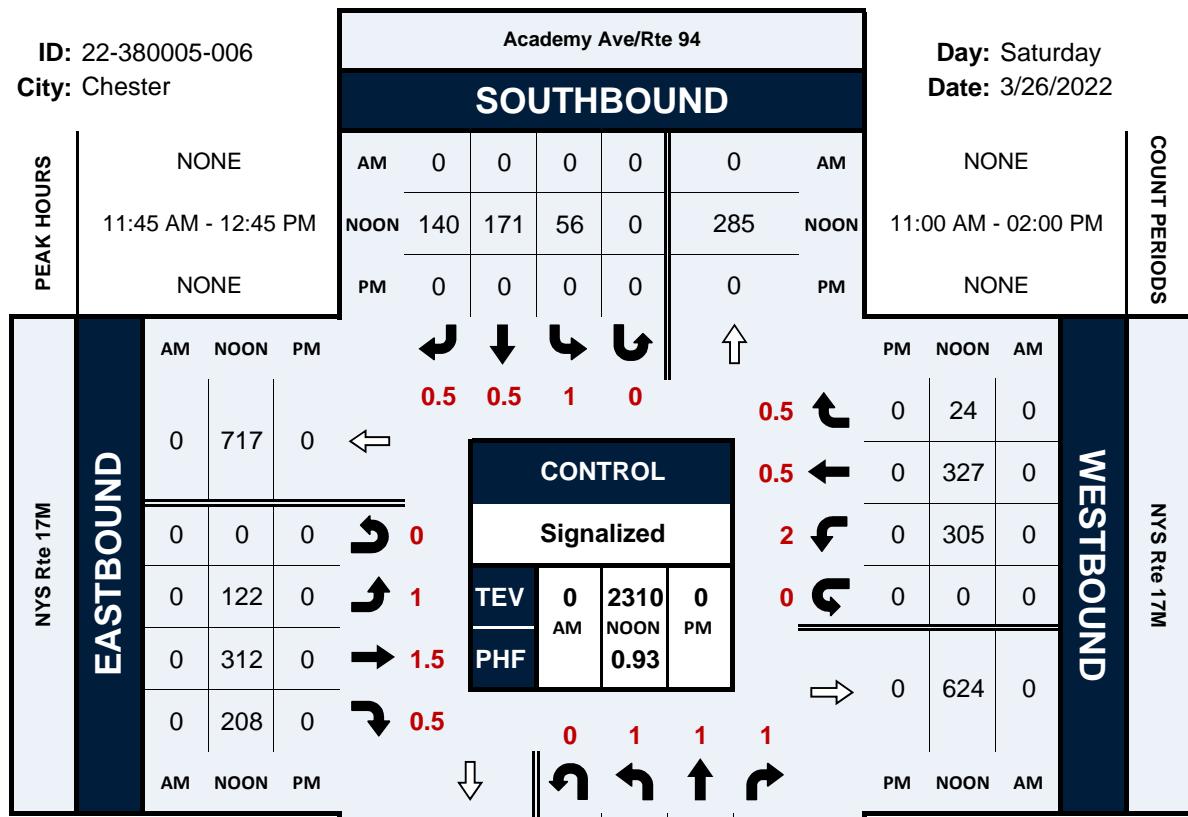
### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Academy Ave/Rte 94		Academy Ave/Rte 94		NYS Rte 17M		NYS Rte 17M		
NOON	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	1	0	1
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	1	0	0	0	0	0	1
1:45 PM	0	0	0	0	1	0	0	0	1
TOTAL VOLUMES :	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
APPROACH %'s :	0	0	1	0	1	0	1	0	3
PEAK HR :	12:00 AM - 01:00 AM								TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :									

**Academy Ave/Rte 94 & NYS Rte 17M****Peak Hour Turning Movement Count**

**ID:** 22-380005-006  
**City:** Chester

**Day:** Saturday  
**Date:** 3/26/2022



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-007

**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	Rte 126 NB On/Off Ramp				Rte 126 NB On/Off Ramp				Rte 94				Rte 94				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	
11:00 AM	25	0	21	0	0	0	0	0	17	96	0	0	0	71	87	0	317
11:15 AM	24	0	25	0	0	0	0	0	21	112	0	0	0	78	81	0	341
11:30 AM	38	0	28	0	0	0	0	0	17	88	0	0	0	85	74	0	330
11:45 AM	22	1	38	0	0	0	0	0	25	119	0	0	0	74	80	0	359
12:00 PM	30	0	41	0	0	0	0	0	28	143	0	0	0	91	81	0	414
12:15 PM	26	0	21	0	0	0	0	0	27	123	0	0	0	80	87	0	364
12:30 PM	30	0	33	0	0	0	0	0	13	127	0	0	0	89	99	0	391
12:45 PM	30	0	32	0	0	0	0	0	16	119	0	0	0	72	76	0	345
1:00 PM	28	0	29	0	0	0	0	0	16	114	0	0	0	62	68	0	317
1:15 PM	37	0	32	0	0	0	0	0	11	91	0	0	0	68	95	0	334
1:30 PM	26	1	27	0	0	0	0	0	16	97	0	0	0	68	74	0	309
1:45 PM	30	0	23	0	0	0	0	0	22	114	0	0	0	81	81	0	351
<b>TOTAL VOLUMES :</b>	NL 346	NT 2	NR 350	NU 0	SL 0	ST 0	SR 0	SU 0	EL 229	ET 1343	ER 0	EU 0	WL 0	WT 919	WR 983	WU 0	TOTAL 4172
<b>APPROACH %'s :</b>	49.57%	0.29%	50.14%	0.00%					14.57%	85.43%	0.00%	0.00%	0.00%	48.32%	51.68%	0.00%	
<b>PEAK HR :</b>	<b>11:45 AM - 12:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	108	1	133	0	0	0	0	0	93	512	0	0	0	334	347	0	1528
<b>PEAK HR FACTOR :</b>	0.900	0.250	0.811	0.000	0.000	0.000	0.000	0.000	0.830	0.895	0.000	0.000	0.000	0.918	0.876	0.000	0.923
					0.852								0.885				

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester  
**Control:** Signalized

**Project ID:** 22-380005-007

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:		Rte 126 NB On/Off Ramp				Rte 126 NB On/Off Ramp				Rte 94				Rte 94				
NOON		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
		0.5 NL	0.5 NT	1 NR	0 NU	0 SL	0 ST	0 SR	0 SU	1 EL	1 ET	0 ER	0 EU	0 WL	1 WT	1 WR	0 WU	TOTAL
11:00 AM	22	0	20	0	0	0	0	0	0	16	94	0	0	0	71	86	0	309
11:15 AM	23	0	25	0	0	0	0	0	0	21	110	0	0	0	77	81	0	337
11:30 AM	34	0	28	0	0	0	0	0	0	17	87	0	0	0	84	72	0	322
11:45 AM	21	1	38	0	0	0	0	0	0	24	117	0	0	0	71	75	0	347
12:00 PM	29	0	41	0	0	0	0	0	0	27	141	0	0	0	90	81	0	409
12:15 PM	25	0	21	0	0	0	0	0	0	25	117	0	0	0	76	86	0	350
12:30 PM	29	0	33	0	0	0	0	0	0	13	125	0	0	0	88	99	0	387
12:45 PM	28	0	32	0	0	0	0	0	0	15	118	0	0	0	71	75	0	339
1:00 PM	27	0	29	0	0	0	0	0	0	16	112	0	0	0	60	68	0	312
1:15 PM	34	0	32	0	0	0	0	0	0	11	91	0	0	0	68	92	0	328
1:30 PM	22	1	27	0	0	0	0	0	0	14	97	0	0	0	67	74	0	302
1:45 PM	27	0	23	0	0	0	0	0	0	22	112	0	0	0	79	81	0	344
TOTAL VOLUMES : APPROACH %'s :	NL	NT	NR	NU	SL	ST	SR	SU		EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
	321	2	349	0	0	0	0	0		221	1321	0	0	0	902	970	0	4086
PEAK HR :	11:45 AM - 12:45 PM									14.33%	85.67%	0.00%	0.00%	0.00%	48.18%	51.82%	0.00%	TOTAL
PEAK HR VOL :	104	1	133	0	0	0	0	0		89	500	0	0	0	325	341	0	1493
PEAK HR FACTOR :	0.897	0.250	0.811	0.000	0.000	0.000	0.000	0.000		0.824	0.887	0.000	0.000	0.000	0.903	0.861	0.000	0.913
	0.850				0.876				0.890									

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester

### **Control:** Signalized

Project ID: 22-380005-007

Date: 3/26/2022

Data - HT

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 NB On/Off Ramp & Rte 94

**City:** Chester

### **Control:** Signalized

**Project ID:** 22-380005-007

Date: 3/26/2022

## Data - Bikes

# National Data & Surveying ServicesIntersection Turning

**Location:** Rte 126 NB On/Off Ramp & Rte 94  
**City:** Chester

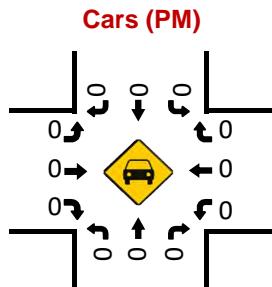
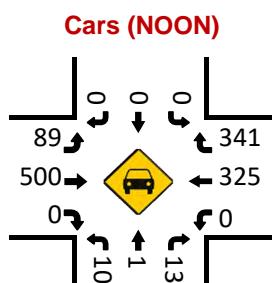
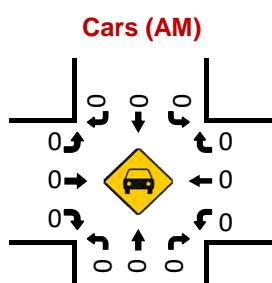
**Project ID:** 22-380005-007  
**Date:** 3/26/2022

## Data - Pedestrians (Crosswalks)

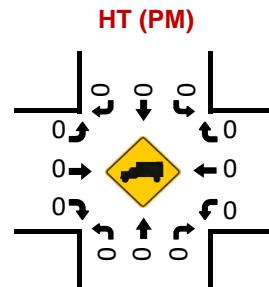
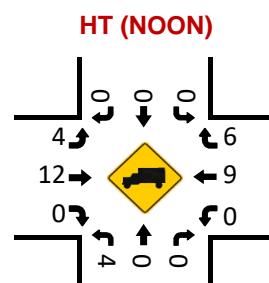
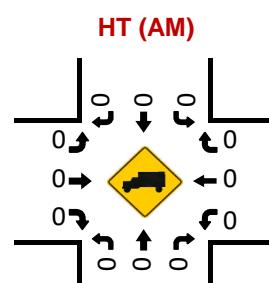
## Rte 126 NB On/Off Ramp & Rte 94

## Peak Hour Turning Movement Count

**ID:** 22-380005-007  
**City:** Chester



**Pedestrians (Crosswalks)**



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-008

**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	Rte 126 SB On/Off Ramp				Rte 126 SB On/Off Ramp				Rte 94				Rte 94				TOTAL			
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND							
NOON	0 NL	0 NT	0 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	0 EL	1 ET	1 ER	0 EU	1 WL	1 WT	0 WR	0 WU				
11:00 AM	0	0	0	0	49	0	23	0	0	64	66	0	23	71	0	0	296			
11:15 AM	0	0	0	0	62	0	20	0	0	71	44	0	34	70	0	0	301			
11:30 AM	0	0	0	0	47	0	23	0	0	58	51	0	27	97	0	0	303			
11:45 AM	0	0	0	0	71	0	35	0	0	73	43	0	22	72	0	0	316			
12:00 PM	0	0	0	0	85	0	38	0	0	91	42	0	35	88	0	0	379			
12:15 PM	0	0	0	0	63	1	27	0	0	82	42	0	43	62	0	0	320			
12:30 PM	0	0	0	0	73	0	36	0	0	70	69	0	42	78	0	0	368			
12:45 PM	0	0	0	0	72	0	32	0	0	62	35	0	37	64	0	1	303			
1:00 PM	0	0	0	0	77	0	26	0	0	50	49	0	28	62	0	0	292			
1:15 PM	0	0	0	0	62	0	29	0	0	41	43	0	33	70	0	0	278			
1:30 PM	0	0	0	0	59	0	25	0	0	53	38	0	32	62	0	0	269			
1:45 PM	0	0	0	0	72	0	23	0	0	64	38	0	34	78	0	0	309			
TOTAL VOLUMES :	NL 0	NT 0	NR 0	NU 0	SL 792	ST 1	SR 337	SU 0	EL 0	ET 779	ER 560	EU 0	WL 390	WT 874	WR 0	WU 1	TOTAL 3734			
APPROACH %'s :	70.09% 0.09% 29.82% 0.00%				0.00% 58.18% 41.82% 0.00%				30.83% 69.09% 0.00% 0.08%				TOTAL							
PEAK HR :	<b>11:45 AM - 12:45 PM</b>																TOTAL			
PEAK HR VOL :	0 0.000	0 0.000	0 0.000	0 0.000	292 0.859	1 0.250	136 0.895	0 0.000	0 0.000	316 0.868	196 0.710	0 0.000	142 0.826	300 0.852	0 0.000	0 0.000	TOTAL 1383			
PEAK HR FACTOR :	0.872				0.921				0.921				0.898				0.912			

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-008

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	Rte 126 SB On/Off Ramp				Rte 126 SB On/Off Ramp				Rte 94				Rte 94				TOTAL	
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
NOON	0 NL	0 NT	0 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	0 EL	1 ET	1 ER	0 EU	1 WL	1 WT	0 WR	0 WU		
11:00 AM	0	0	0	0	47	0	21	0	0	63	66	0	23	67	0	0	287	
11:15 AM	0	0	0	0	60	0	19	0	0	71	38	0	34	68	0	0	290	
11:30 AM	0	0	0	0	46	0	22	0	0	58	45	0	26	93	0	0	290	
11:45 AM	0	0	0	0	70	0	34	0	0	71	40	0	21	69	0	0	305	
12:00 PM	0	0	0	0	83	0	37	0	0	90	38	0	35	86	0	0	369	
12:15 PM	0	0	0	0	61	1	24	0	0	76	39	0	41	60	0	0	302	
12:30 PM	0	0	0	0	71	0	35	0	0	70	68	0	41	76	0	0	361	
12:45 PM	0	0	0	0	71	0	31	0	0	61	33	0	37	61	0	1	295	
1:00 PM	0	0	0	0	77	0	24	0	0	48	48	0	28	59	0	0	284	
1:15 PM	0	0	0	0	62	0	27	0	0	41	42	0	33	67	0	0	272	
1:30 PM	0	0	0	0	59	0	23	0	0	51	31	0	32	57	0	0	253	
1:45 PM	0	0	0	0	71	0	20	0	0	63	34	0	33	75	0	0	296	
TOTAL VOLUMES :	NL 0	NT 0	NR 0	NU 0	SL 778	ST 1	SR 317	SU 0	EL 0	ET 763	ER 522	EU 0	WL 384	WT 838	WR 0	WU 1	TOTAL 3604	
APPROACH %'s :	70.99% 0.09% 28.92% 0.00%				0.00% 59.38% 40.62% 0.00%				31.40% 68.52% 0.00% 0.08%				TOTAL					
PEAK HR :	<b>11:45 AM - 12:45 PM</b>																TOTAL	
PEAK HR VOL :	0	0	0	0	285	1	130	0	0	307	185	0	138	291	0	0	1337	
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.858	0.250	0.878	0.000	0.000	0.853	0.680	0.000	0.841	0.846	0.000	0.000	0.906	
					0.867					0.891								

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

**Control:** Signalized

**Project ID:** 22-380005-008

**Date:** 3/26/2022

## Data - HT

NS/EW Streets:	Rte 126 SB On/Off Ramp				Rte 126 SB On/Off Ramp				Rte 94				Rte 94				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	1 SL	0.5 ST	0.5 SR	0 SU	0 EL	1 ET	1 ER	0 EU	1 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	2	0	2	0	0	1	0	0	0	4	0	0	9
11:15 AM	0	0	0	0	2	0	1	0	0	0	6	0	0	2	0	0	11
11:30 AM	0	0	0	0	1	0	1	0	0	0	6	0	1	4	0	0	13
11:45 AM	0	0	0	0	1	0	1	0	0	2	3	0	1	3	0	0	11
12:00 PM	0	0	0	0	2	0	1	0	0	1	4	0	0	2	0	0	10
12:15 PM	0	0	0	0	2	0	3	0	0	6	3	0	2	2	0	0	18
12:30 PM	0	0	0	0	2	0	1	0	0	0	1	0	1	2	0	0	7
12:45 PM	0	0	0	0	1	0	1	0	0	1	2	0	0	3	0	0	8
1:00 PM	0	0	0	0	0	0	2	0	0	2	1	0	0	3	0	0	8
1:15 PM	0	0	0	0	0	0	2	0	0	0	1	0	0	3	0	0	6
1:30 PM	0	0	0	0	0	0	2	0	0	2	7	0	0	5	0	0	16
1:45 PM	0	0	0	0	1	0	3	0	0	1	4	0	1	3	0	0	13
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 14 41.18%	ST 0 0.00%	SR 20 58.82%	SU 0 0.00%	EL 0 0.00%	ET 16 29.63%	ER 38 70.37%	EU 0 0.00%	WL 6 14.29%	WT 36 85.71%	WR 0 0.00%	WU 0 0.00%	TOTAL 130
<b>PEAK HR :</b>	<b>11:45 AM - 12:45 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	7 0.875	0 0.000	6 0.500	0 0.000	0 0.000	9 0.375	11 0.688	0 0.000	4 0.500	9 0.750	0 0.000	0 0.000	46 0.639
<b>PEAK HR FACTOR :</b>	0.650				0.556				0.556				0.813				

National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 126 SB On/Off Ramp & Rte 94

**City:** Chester

### **Control:** Signalized

**Project ID:** 22-380005-008

Date: 3/26/2022

# Data - Bikes

# National Data & Surveying Services Intersection Turning

Location: Rte 126 SB On/Off Ramp & Rte 94  
 City: Chester

Project ID: 22-380005-008  
 Date: 3/26/2022

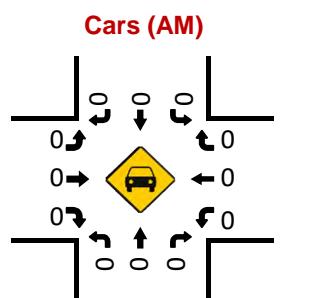
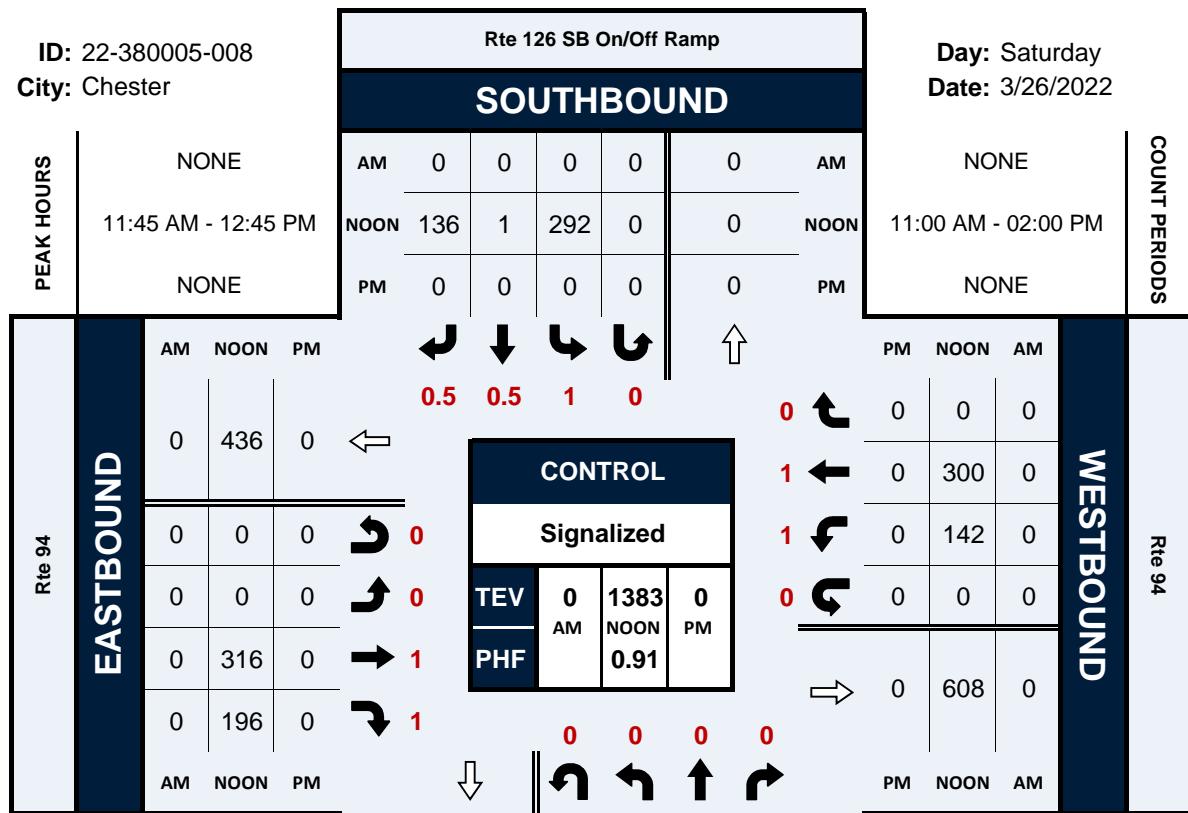
## Movement Count

### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Rte 126 SB On/Off Ramp		Rte 126 SB On/Off Ramp		Rte 94		Rte 94			
NOON	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL	
	EB	WB	EB	WB	NB	SB	NB	SB		
11:00 AM	0	0	0	0	0	0	0	0	0	
11:15 AM	1	0	0	0	0	0	0	0	1	
11:30 AM	0	0	0	0	0	0	0	0	0	
11:45 AM	0	0	0	0	0	0	0	0	0	
12:00 PM	0	0	0	0	0	0	0	0	0	
12:15 PM	0	0	0	0	0	0	1	0	1	
12:30 PM	0	0	0	0	0	0	0	0	0	
12:45 PM	0	0	0	0	0	0	0	0	0	
1:00 PM	0	0	0	0	0	0	0	0	0	
1:15 PM	0	0	0	0	0	0	0	0	0	
1:30 PM	0	0	1	1	0	0	0	0	2	
1:45 PM	0	0	0	0	0	0	0	0	0	
TOTAL VOLUMES :	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL	
APPROACH %'s :	1	0	1	1	0	0	1	0	4	
PEAK HR :	12:00 AM - 01:00 AM		0	0	0	0	1	0	TOTAL	
PEAK HR VOL :	0	0								
PEAK HR FACTOR :							0.250		0.250	

**Rte 126 SB On/Off Ramp & Rte 94****Peak Hour Turning Movement Count**

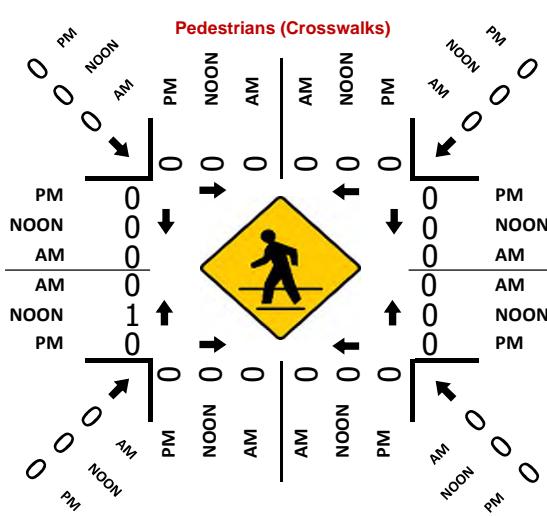
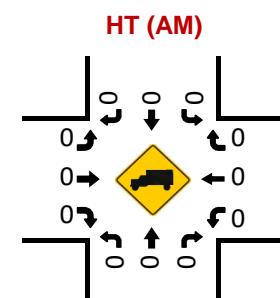
**ID:** 22-380005-008  
**City:** Chester



**NORTHBOUND**

**Rte 126 SB On/Off Ramp**

PEAK HOURS	AM	NOON	PM	AM	NOON	PM
NONE	0	0	0	0	0	0
11:45 AM - 12:45 PM	339	0	0	0	0	0
NONE	0	0	0	0	0	0
	AM	NOON	PM	AM	NOON	PM



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M

**City:** Monroe

**Control:** 2-Way Stop(NB/SB)

**Project ID:** 22-380005-009

**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	Museum Village Rd				Museum Village Rd				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	9	8	0	19	8	7	0	4	23	1	0	4	44	7	0	134
11:15 AM	0	5	1	0	15	2	10	0	5	34	0	0	2	41	9	0	124
11:30 AM	0	3	1	0	17	3	6	0	9	32	0	0	1	47	7	0	126
11:45 AM	0	7	0	0	27	6	9	0	6	32	0	0	0	52	5	0	144
12:00 PM	0	9	1	0	18	1	5	0	4	34	0	0	3	35	12	0	122
12:15 PM	0	2	1	0	21	8	6	0	4	32	1	0	1	54	10	0	140
12:30 PM	0	4	2	0	23	4	5	0	3	27	0	0	2	37	8	0	115
12:45 PM	0	1	3	0	13	10	4	0	5	34	0	0	3	46	6	0	125
1:00 PM	0	2	3	0	9	6	13	0	2	30	0	0	2	37	10	0	114
1:15 PM	0	5	3	0	9	6	7	0	5	27	2	0	3	43	3	0	113
1:30 PM	0	2	2	0	19	7	6	0	3	27	0	0	0	55	13	0	134
1:45 PM	0	4	1	0	12	9	9	0	4	30	0	0	2	44	8	0	123
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	53	26	0	202	70	87	0	54	362	4	0	23	535	98	0	1514
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	21	3	0	83	18	26	0	23	130	1	0	5	188	34	0	532
<b>PEAK HR FACTOR :</b>	0.000	0.583	0.750	0.000	0.769	0.563	0.722	0.000	0.639	0.956	0.250	0.000	0.417	0.870	0.708	0.000	0.924

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M

**City:** Monroe

**Control:** 2-Way Stop(NB/SB)

**Project ID:** 22-380005-009

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	Museum Village Rd				Museum Village Rd				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	9	8	0	19	8	7	0	4	21	1	0	4	42	7	0	130
11:15 AM	0	5	1	0	15	2	10	0	5	33	0	0	2	41	9	0	123
11:30 AM	0	3	1	0	17	3	6	0	9	32	0	0	1	47	7	0	126
11:45 AM	0	7	0	0	26	6	9	0	6	30	0	0	0	52	5	0	141
12:00 PM	0	9	1	0	18	1	4	0	4	34	0	0	3	33	12	0	119
12:15 PM	0	2	1	0	21	8	6	0	4	32	1	0	1	54	10	0	140
12:30 PM	0	4	1	0	23	4	5	0	3	27	0	0	1	36	8	0	112
12:45 PM	0	1	3	0	13	10	4	0	5	34	0	0	3	45	6	0	124
1:00 PM	0	2	2	0	9	6	12	0	2	29	0	0	2	36	10	0	110
1:15 PM	0	5	3	0	9	6	6	0	5	26	1	0	3	42	3	0	109
1:30 PM	0	2	2	0	19	6	6	0	3	27	0	0	0	55	13	0	133
1:45 PM	0	4	1	0	12	9	9	0	4	30	0	0	2	44	8	0	123
<b>TOTAL VOLUMES :</b>	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
<b>APPROACH %'s :</b>	0	53	24	0	201	69	84	0	54	355	3	0	22	527	98	0	1490
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	21	3	0	82	18	25	0	23	128	1	0	5	186	34	0	526
<b>PEAK HR FACTOR :</b>	0.000	0.583	0.750	0.000	0.788	0.563	0.694	0.000	0.639	0.941	0.250	0.000	0.417	0.861	0.708	0.000	0.933

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M

**City:** Monroe

**Control:** 2-Way Stop(NB/SB)

**Project ID:** 22-380005-009

**Date:** 3/26/2022

## Data - HT

NS/EW Streets:	Museum Village Rd				Museum Village Rd				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	0	4
11:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	1	0	0	0	0	2	0	0	0	0	0	0	3
12:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	2	0	3
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	1	1	0	0	3
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	1
1:00 PM	0	0	1	0	0	0	1	0	0	1	0	0	0	1	1	0	4
1:15 PM	0	0	0	0	0	0	1	0	0	1	1	0	0	1	0	0	4
1:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 2	NU 0	SL 1	ST 1	SR 3	SU 0	EL 0	ET 7	ER 1	EU 0	WL 1	WT 8	WR 0	WU 0	<b>TOTAL 24</b>
<b>APPROACH %'s :</b>	0.00%	0.00%	100.00%	0.00%	20.00%	20.00%	60.00%	0.00%	0.00%	87.50%	12.50%	0.00%	11.11%	88.89%	0.00%	0.00%	
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	1	0	1	0	0	2	0	0	0	2	0	0	<b>6</b>
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.000	<b>0.500</b>

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Museum Village Rd & NYS Rte 17M

**City:** Monroe

**Control:** 2-Way Stop(NB/SB)

**Project ID:** 22-380005-009

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Museum Village Rd				Museum Village Rd				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 1	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 1</b>
<b>APPROACH %'s :</b>	0.00% 0.00% 100.00% 0.00%																
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1 0.250	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 1</b>
<b>PEAK HR FACTOR :</b>	0.250																

# National Data & Surveying Services Intersection Turning

**Location:** Museum Village Rd & NYS Rte 17M  
**City:** Monroe

**Project ID:** 22-380005-009  
**Date:** 3/26/2022

## Movement Count

### Data - Pedestrians (Crosswalks)

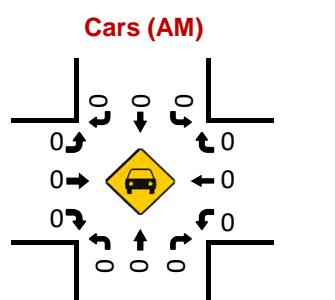
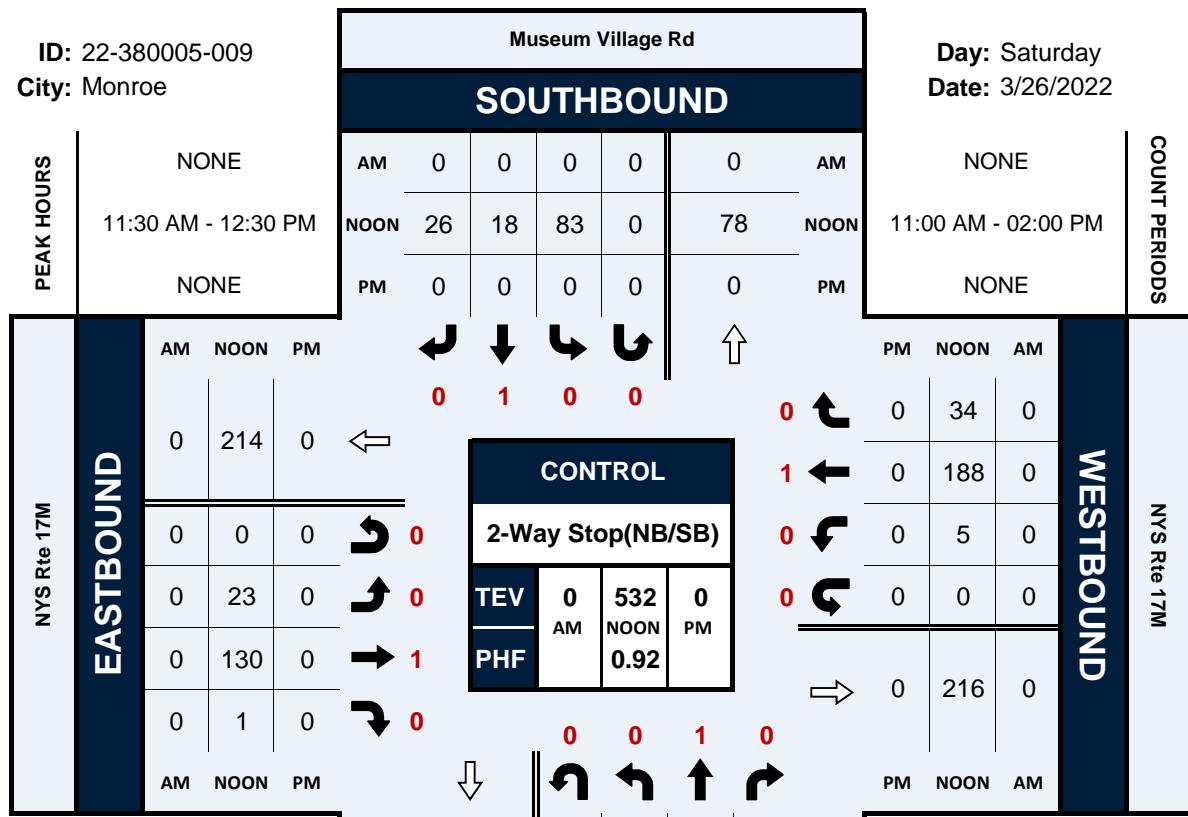
NS/EW Streets:	Museum Village Rd		Museum Village Rd		NYS Rte 17M		NYS Rte 17M		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
NOON	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
APPROACH %'s :	0	0	0	0	0	0	0	0	0
PEAK HR :	12:00 AM - 01:00 AM								TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :									

## Museum Village Rd & NYS Rte 17M

### Peak Hour Turning Movement Count

ID: 22-380005-009  
City: Monroe

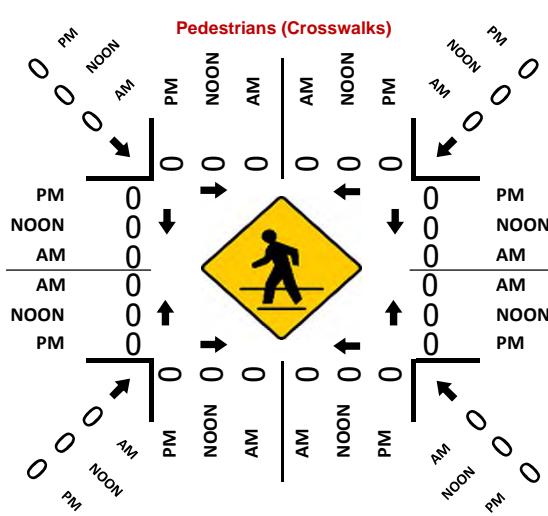
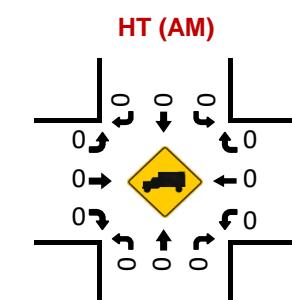
Day: Saturday  
Date: 3/26/2022



**NORTHBOUND**

Museum Village Rd

PM			NOON			AM		
0	0	0	0	0	21	0	0	0
0	24	0	0	0	3	0	0	0
0	0	0	0	0	0	0	0	0



National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-010

Date: 3/26/2022

## Data - Total

NS/EW Streets:		Rte 208				Rte 208				Museum Village Rd S				Museum Village Rd S			
NOON		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
		0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU
11:00 AM		2	77	0	0	0	142	9	0	12	0	0	0	0	0	0	242
11:15 AM		0	78	0	0	0	111	10	0	15	0	2	0	0	0	0	216
11:30 AM		0	102	0	0	0	122	10	0	20	0	0	0	0	0	0	254
11:45 AM		0	83	0	0	0	139	9	0	16	0	1	0	0	0	0	248
12:00 PM		2	98	0	0	0	123	9	0	15	0	2	0	0	0	0	249
12:15 PM		3	87	0	0	0	135	12	0	16	0	2	0	0	0	0	255
12:30 PM		0	95	0	0	0	126	10	0	32	0	0	0	0	0	0	263
12:45 PM		1	122	0	0	0	102	3	0	11	0	1	0	0	0	0	240
1:00 PM		2	107	0	0	0	99	8	0	19	0	0	0	0	0	0	235
1:15 PM		1	118	0	0	0	121	6	0	9	0	1	0	0	0	0	256
1:30 PM		1	105	0	0	0	101	12	0	10	0	1	0	0	0	0	230
1:45 PM		3	119	0	0	0	105	6	0	21	0	2	0	0	0	0	256
TOTAL VOLUMES : APPROACH %'s :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
	15	1191	0	0	0	1426	104	0	196	0	12	0	0	0	0	0	2944
PEAK HR :	11:45 AM - 12:45 PM				0.00%	93.20%	6.80%	0.00%	94.23%	0.00%	5.77%	0.00%	0.000	0.000	0.000	0.000	TOTAL
PEAK HR VOL :	5	363	0	0									0	0	0	0	1015
PEAK HR FACTOR :	0.417	0.926	0.000	0.000	0.000	0.941	0.833	0.000	0.617	0.000	0.625	0.000	0.000	0.000	0.000	0.000	0.965
					0.920					0.951					0.656		

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-010

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	Rte 208				Rte 208				Museum Village Rd S				Museum Village Rd S				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU	
11:00 AM	2	74	0	0	0	138	9	0	12	0	0	0	0	0	0	0	235
11:15 AM	0	76	0	0	0	109	10	0	14	0	2	0	0	0	0	0	211
11:30 AM	0	100	0	0	0	121	10	0	20	0	0	0	0	0	0	0	251
11:45 AM	0	83	0	0	0	138	8	0	16	0	1	0	0	0	0	0	246
12:00 PM	2	97	0	0	0	121	9	0	14	0	2	0	0	0	0	0	245
12:15 PM	3	86	0	0	0	134	12	0	16	0	2	0	0	0	0	0	253
12:30 PM	0	93	0	0	0	125	10	0	31	0	0	0	0	0	0	0	259
12:45 PM	1	120	0	0	0	101	3	0	11	0	1	0	0	0	0	0	237
1:00 PM	2	106	0	0	0	98	7	0	18	0	0	0	0	0	0	0	231
1:15 PM	1	117	0	0	0	120	6	0	9	0	1	0	0	0	0	0	254
1:30 PM	1	103	0	0	0	101	11	0	10	0	1	0	0	0	0	0	227
1:45 PM	3	119	0	0	0	104	6	0	21	0	2	0	0	0	0	0	255
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
APPROACH %'s :	15	1174	0	0	0	1410	101	0	192	0	12	0	0	0	0	0	2904
PEAK HR :	11:45 AM - 12:45 PM																TOTAL
PEAK HR VOL :	5	359	0	0	0	518	39	0	77	0	5	0	0	0	0	0	1003
PEAK HR FACTOR :	0.417	0.925	0.000	0.000	0.000	0.938	0.813	0.000	0.621	0.000	0.625	0.000	0.000	0.000	0.000	0.000	0.968
						0.954					0.661						

National Data & Surveying Services **Intersection Turning Movement Count**

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-010

Date: 3/26/2022

## Data - HT

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Rte 208 & Museum Village Rd S

**City:** Monroe

**Control:** 1-Way Stop(EB)

**Project ID:** 22-380005-010

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Rte 208				Rte 208				Museum Village Rd S				Museum Village Rd S				TOTAL	
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
NOON	0 NL	1 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	0 WT	0 WR	0 WU		
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	WU 0	<b>TOTAL 0</b>	
<b>APPROACH %'s :</b>																		
<b>PEAK HR :</b>	<b>11:45 AM - 12:45 PM</b>																	
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	<b>TOTAL 0</b>	
<b>PEAK HR FACTOR :</b>																		

# National Data & Surveying Services Intersection Turning

Location: Rte 208 & Museum Village Rd S  
 City: Monroe

Project ID: 22-380005-010  
 Date: 3/26/2022

## Movement Count

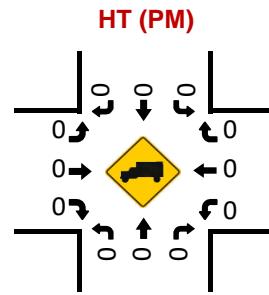
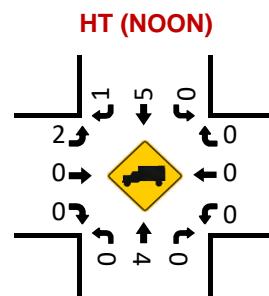
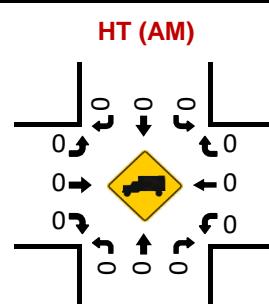
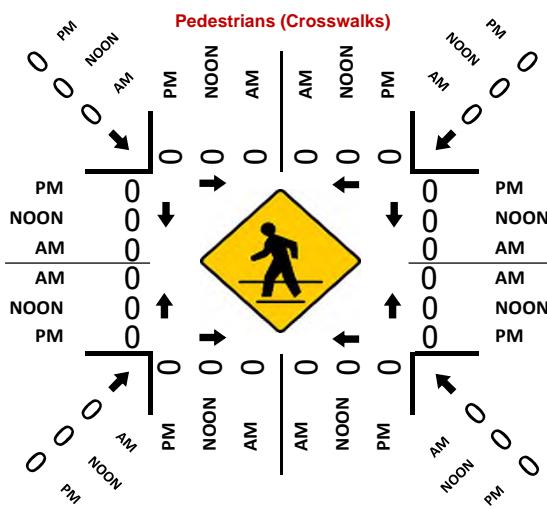
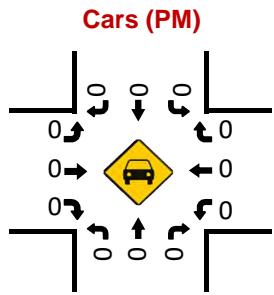
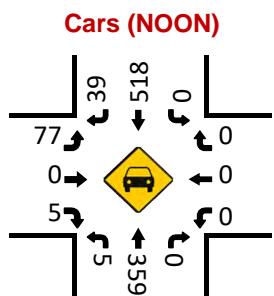
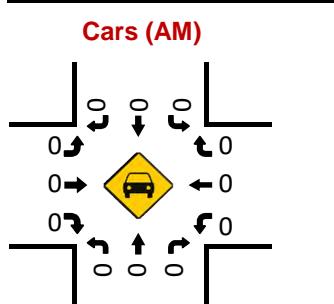
### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Rte 208		Rte 208		Museum Village Rd S		Museum Village Rd S		
NOON	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
11:00 AM	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
APPROACH %'s :	0	0	0	0	0	0	0	0	0
PEAK HR :	12:00 AM - 01:00 AM								TOTAL
PEAK HR VOL :	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :									

## Rte 208 & Museum Village Rd S

# Peak Hour Turning Movement Count

**ID:** 22-380005-010  
**City:** Monroe



# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-011

**Date:** 3/26/2022

## Data - Total

NS/EW Streets:	Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	1	32	0	0	0	50	0	0	83
11:15 AM	0	0	0	0	0	0	0	0	2	30	0	0	0	59	0	0	91
11:30 AM	0	0	0	0	1	0	0	0	0	32	0	0	0	58	0	0	91
11:45 AM	0	0	0	0	1	0	1	0	2	27	0	0	0	59	1	0	91
12:00 PM	0	0	0	0	0	0	1	0	0	37	0	0	0	50	0	0	88
12:15 PM	0	0	0	0	0	0	1	0	0	36	0	0	0	60	3	0	100
12:30 PM	0	0	0	0	0	0	1	0	0	31	0	0	0	38	0	0	70
12:45 PM	0	0	0	0	0	0	0	0	0	32	0	0	0	46	0	0	78
1:00 PM	0	0	0	0	4	0	0	0	0	33	0	0	0	44	0	0	81
1:15 PM	0	0	0	0	0	0	0	0	0	32	0	0	0	42	0	0	74
1:30 PM	0	0	0	0	1	0	0	0	0	32	0	0	0	54	0	0	87
1:45 PM	0	0	0	0	0	0	0	0	0	24	0	0	0	60	0	0	84
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 7	ST 0	SR 4	SU 0	EL 5	ET 378	ER 0	EU 0	WL 0	WT 620	WR 4	WU 0	TOTAL 1018
<b>APPROACH %'s :</b>	63.64% 0.00% 36.36% 0.00%				1.31% 98.69% 0.00% 0.00%				0.00% 99.36% 0.64% 0.00%				0.00% 99.36% 0.64% 0.00%				TOTAL
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																TOTAL
<b>PEAK HR VOL :</b>	0	0	0	0	2	0	3	0	2	132	0	0	0	227	4	0	370
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.500	0.000	0.750	0.000	0.250	0.892	0.000	0.000	0.000	0.946	0.333	0.000	0.925
					0.625					0.905				0.917			

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-011

**Date:** 3/26/2022

## Data - Cars

NS/EW Streets:	Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	1	29	0	0	0	48	0	0	78
11:15 AM	0	0	0	0	0	0	0	0	2	30	0	0	0	58	0	0	90
11:30 AM	0	0	0	0	1	0	0	0	0	32	0	0	0	57	0	0	90
11:45 AM	0	0	0	0	1	0	1	0	2	25	0	0	0	58	1	0	88
12:00 PM	0	0	0	0	0	0	1	0	0	37	0	0	0	46	0	0	84
12:15 PM	0	0	0	0	0	0	1	0	0	35	0	0	0	60	3	0	99
12:30 PM	0	0	0	0	0	0	1	0	0	31	0	0	0	38	0	0	70
12:45 PM	0	0	0	0	0	0	0	0	0	32	0	0	0	45	0	0	77
1:00 PM	0	0	0	0	4	0	0	0	0	30	0	0	0	42	0	0	76
1:15 PM	0	0	0	0	0	0	0	0	0	31	0	0	0	41	0	0	72
1:30 PM	0	0	0	0	1	0	0	0	0	32	0	0	0	52	0	0	85
1:45 PM	0	0	0	0	0	0	0	0	0	24	0	0	0	60	0	0	84
TOTAL VOLUMES :	NL 0	NT 0	NR 0	NU 0	SL 7	ST 0	SR 4	SU 0	EL 5	ET 368	ER 0	EU 0	WL 0	WT 605	WR 4	WU 0	TOTAL 993
APPROACH %'s :	63.64% 0.00% 36.36% 0.00%				1.34% 98.66% 0.00% 0.00%				0.00% 99.34% 0.66% 0.00%				0.00% 99.34% 0.66% 0.00%				TOTAL
PEAK HR :	<b>11:30 AM - 12:30 PM</b>																TOTAL
PEAK HR VOL :	0 0.000	0 0.000	0 0.000	0 0.000	2 0.500	0 0.000	3 0.750	0 0.000	2 0.250	129 0.872	0 0.000	0 0.000	0 0.000	221 0.921	4 0.333	0 0.000	361 0.912
PEAK HR FACTOR :	0.625				0.885				0.893								

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-011

**Date:** 3/26/2022

## Data - HT

NS/EW Streets:	Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	3	0	0	0	2	0	0	5
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	1	0	0	3
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
12:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
1:00 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	2	0	0	5
1:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 10	ER 0	EU 0	WL 0	WT 15	WR 0	WU 0	<b>TOTAL 25</b>
<b>APPROACH %'s :</b>									0.00% 100.00% 0.00% 0.00%				0.00% 100.00% 0.00% 0.00%				<b>TOTAL</b>
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0	0	0	0	0	0	0	0	0	3	0	0	0	6	0	0	<b>9</b>
<b>PEAK HR FACTOR :</b>	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.375	0.000	0.000	0.000	0.375	0.000	0.000	<b>0.563</b>

# National Data & Surveying Services Intersection Turning Movement Count

**Location:** Goose Pond Parking & NYS Rte 17M

**City:** Chester

**Control:** No Control

**Project ID:** 22-380005-011

**Date:** 3/26/2022

## Data - Bikes

NS/EW Streets:	Goose Pond Parking				Goose Pond Parking				NYS Rte 17M				NYS Rte 17M				TOTAL
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
NOON	0 NL	0 NT	0 NR	0 NU	0 SL	1 ST	0 SR	0 SU	0 EL	1 ET	0 ER	0 EU	0 WL	1 WT	0 WR	0 WU	
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>TOTAL VOLUMES :</b>	NL 0	NT 0	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 1	WR 0	WU 0	<b>TOTAL 1</b>
<b>APPROACH %'s :</b>													0.00% 100.00% 0.00% 0.00%				
<b>PEAK HR :</b>	<b>11:30 AM - 12:30 PM</b>																<b>TOTAL</b>
<b>PEAK HR VOL :</b>	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	0 0.000	1 0.250	0 0.000	0 0.000	<b>1 0.250</b>
<b>PEAK HR FACTOR :</b>																	

# National Data & Surveying Services Intersection Turning

Location: Goose Pond Parking & NYS Rte 17M  
 City: Chester

Project ID: 22-380005-011  
 Date: 3/26/2022

## Movement Count

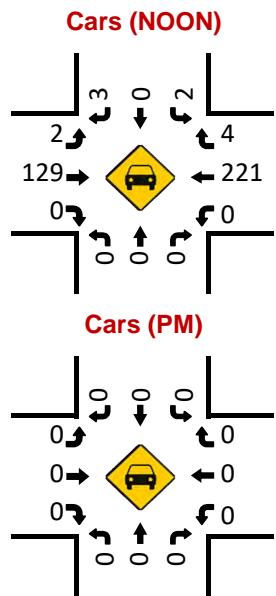
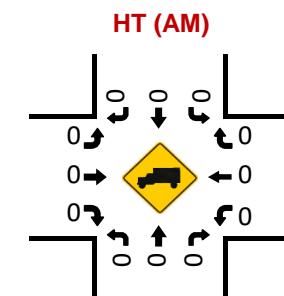
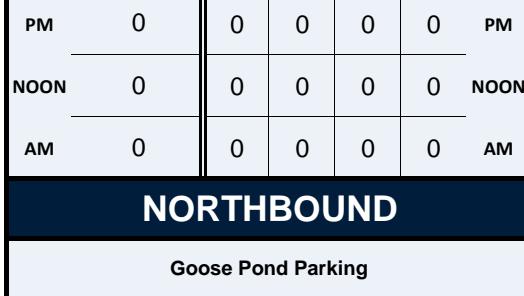
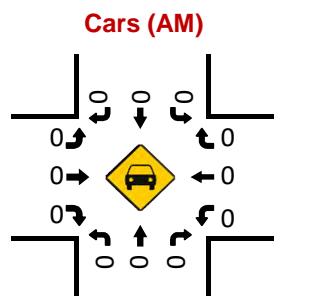
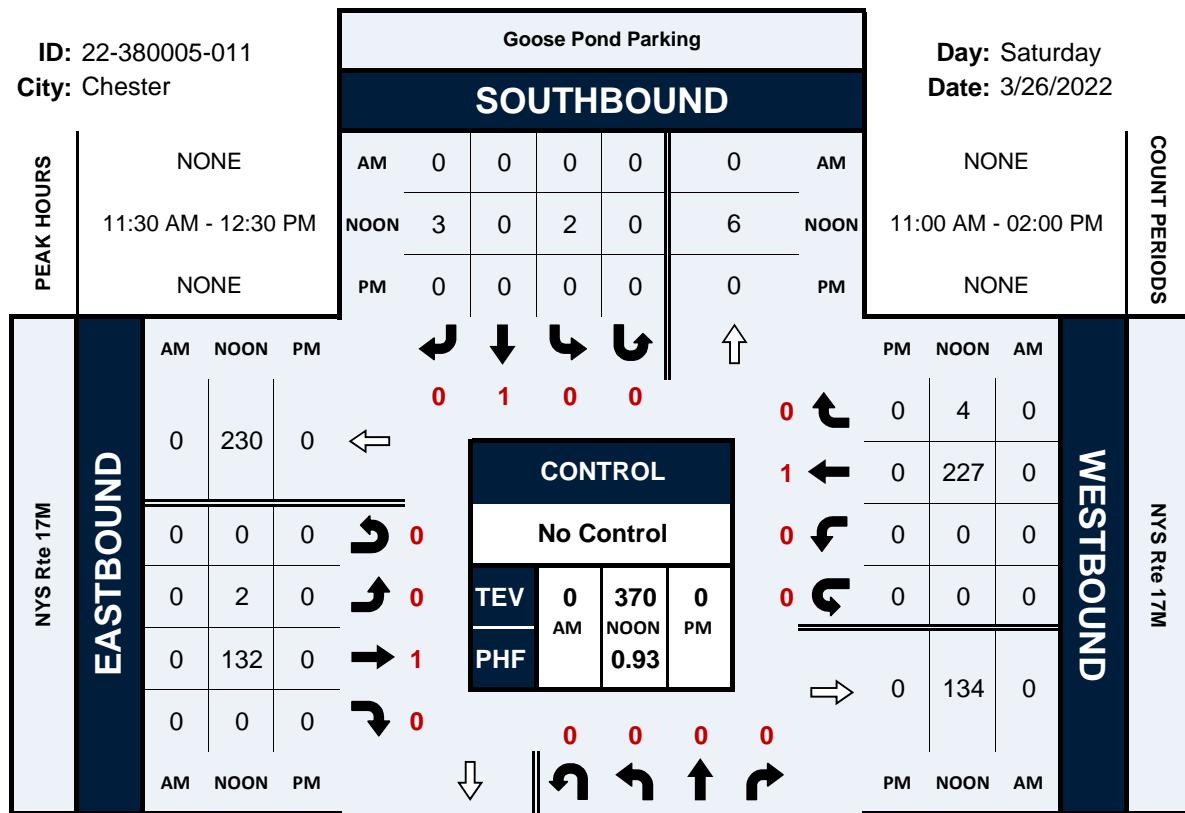
### Data - Pedestrians (Crosswalks)

NS/EW Streets:	Goose Pond Parking		Goose Pond Parking		NYS Rte 17M		NYS Rte 17M		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
NOON	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
11:00 AM	0	0	0	0	0	1	0	2	3
11:15 AM	0	0	0	0	0	2	0	0	2
11:30 AM	0	0	0	0	0	0	2	1	3
11:45 AM	0	0	0	0	0	0	4	2	6
12:00 PM	2	0	0	0	0	1	2	0	5
12:15 PM	2	0	0	0	0	0	2	10	14
12:30 PM	0	0	0	0	0	0	1	0	1
12:45 PM	0	0	0	0	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	12	0	12
1:15 PM	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0
1:45 PM	1	0	0	0	0	0	1	0	2
TOTAL VOLUMES :	EB 5	WB 0	EB 0	WB 0	NB 0	SB 4	NB 24	SB 15	TOTAL 48
APPROACH %'s :	100.00%	0.00%			0.00%	100.00%	61.54%	38.46%	
PEAK HR :	12:00 AM - 01:00 AM		0	0	0	1	10	13	TOTAL 28
PEAK HR VOL :	4	0							
PEAK HR FACTOR :	0.500	0.500			0.250	0.250	0.625	0.325	0.500
							0.479		

**Goose Pond Parking & NYS Rte 17M****Peak Hour Turning Movement Count**

ID: 22-380005-011  
City: Chester

Day: Saturday  
Date: 3/26/2022



## MetroCount Traffic Executive

### Weekly Vehicle Counts (Virtual Week)

#### VirtWeeklyVehicle-140 -- English (ENU)

##### Datasets:

**Site:** [120-269] 400ft from 17M  
**Attribute:** Craigville Rd  
**Direction:** 8 - East bound A>B, West bound B>A. **Lane:** 2  
**Survey Duration:** 21:54 Tuesday, March 1, 2022 => 7:38 Tuesday, March 8, 2022,  
**Zone:**  
**File:** 120-269 0 2022-03-08 0738\_Craigville.EC2 (Plus )  
**Identifier:** BG78EVVB MC56-L5 [MC55] (c)Microcom 19Oct04  
**Algorithm:** Factory default axle (v4.08)  
**Data type:** Axle sensors - Paired (Class/Speed/Count)

##### Profile:

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022 (5.83333)  
**Included classes:** 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13  
**Speed range:** 6 - 99 mph.  
**Direction:** North, East, South, West (bound), P = East, Lane = 0-16  
**Separation:** Headway > 0 sec, Span 0 - 328.084 ft  
**Name:** Default Profile  
**Scheme:** Vehicle classification (Scheme F3)  
**Units:** Non metric (ft, mi, ft/s, mph, lb, ton)  
**In profile:** Vehicles = 5957 / 6073 (98.09%)

## Weekly Vehicle Counts (Virtual Week)

**VirtWeeklyVehicle-140**

**Site:** 120-269.2.3EW

**Description:** 400ft from 17M

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022

**Scheme:** Vehicle classification (Scheme F3)

**Filter:** Cls(1-13) Dir(NESW) Sp(6,99) Headway(>0) Span(0 - 328.084) Lane(0-16)

	Mon	Tue	Wed	Thu	Fri	Sat	Sun	Averages	
								1 - 5	1 - 7
<b>Hour</b>									
0000-0100	6.0	5.0	*	6.0	5.0	7.0	9.0	5.5	6.3
0100-0200	4.0	2.0	*	1.0	8.0	4.0	3.0	3.8	3.7
0200-0300	1.0	5.0	*	0.0	1.0	7.0	3.0	1.8	2.8
0300-0400	2.0	1.0	*	0.0	1.0	1.0	0.0	1.0	0.8
0400-0500	4.0	3.0	*	3.0	3.0	3.0	0.0	3.3	2.7
0500-0600	14.0	12.0	*	19.0	17.0	1.0	4.0	15.5	11.2
0600-0700	39.0	*	*	40.0	42.0	12.0	5.0	40.3	27.6
0700-0800	72.0	*	*	69.0	72.0	19.0	10.0	71.0	48.4
0800-0900	68.0	*	*	89.0	71.0	45.0	26.0	76.0	59.8
0900-1000	70.0	*	*	64.0	65.0	45.0	49.0	66.3	58.6
1000-1100	55.0	*	50.0	51.0	69.0	48.0	49.0	56.3	53.7
1100-1200	64.0	*	63.0	66.0	60.0	69.0	60.0	63.3	63.7
1200-1300	75.0	*	65.0	74.0	67.0	64.0	59.0	70.3	67.3
1300-1400	72.0	*	65.0	76.0	119.0	77.0	67.0	83.0	79.3
1400-1500	99.0	*	88.0	68.0	85.0	53.0	68.0	85.0	76.8
1500-1600	106.0	*	116.0	73.0	86.0	54.0	54.0	95.3	81.5
1600-1700	115.0	*	129.0	122.0	111.0	64.0	62.0	119.3	100.5
1700-1800	146.0	*	133.0	131.0	105.0	45.0	75.0	128.8	105.8
1800-1900	58.0	*	75.0	76.0	58.0	44.0	44.0	66.8	59.2
1900-2000	31.0	*	42.0	51.0	37.0	37.0	44.0	40.3	40.3
2000-2100	32.0	*	36.0	40.0	25.0	28.0	26.0	33.3	31.2
2100-2200	19.0	*	19.0	30.0	28.0	22.0	17.0	24.0	22.5
2200-2300	7.0	*	10.0	11.0	17.0	15.0	10.0	11.3	11.7
2300-2400	8.0	*	10.0	6.0	12.0	12.0	11.0	9.0	9.8
<b>Totals</b>									
0700-1900	1000.0	*	*	959.0	968.0	627.0	623.0	981.1	854.6
0600-2200	1121.0	*	*	1120.0	1100.0	726.0	715.0	1118.9	976.2
0600-0000	1136.0	*	*	1137.0	1129.0	753.0	736.0	1139.2	997.7
0000-0000	1167.0	*	*	1166.0	1164.0	776.0	755.0	1169.9	1025.2
<b>AM Peak</b>	0700 72.0	*	*	0800 89.0	0700 72.0	1100 69.0	1100 60.0		
<b>PM Peak</b>	1700 146.0	*	1700 133.0	1700 131.0	1300 119.0	1300 77.0	1700 75.0		

\* - No data.

## MetroCount Traffic Executive

### Weekly Vehicle Counts (Virtual Week)

#### VirtWeeklyVehicle-141 -- English (ENU)

##### Datasets:

**Site:** [120-269] 400ft from 17M  
**Attribute:** Craigville Rd  
**Direction:** 8 - East bound A>B, West bound B>A. **Lane:** 2  
**Survey Duration:** 21:54 Tuesday, March 1, 2022 => 7:38 Tuesday, March 8, 2022,  
**Zone:**  
**File:** 120-269 0 2022-03-08 0738\_Craigville.EC2 (Plus )  
**Identifier:** BG78EVVB MC56-L5 [MC55] (c)Microcom 19Oct04  
**Algorithm:** Factory default axle (v4.08)  
**Data type:** Axle sensors - Paired (Class/Speed/Count)

##### Profile:

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022 (5.83333)  
**Included classes:** 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13  
**Speed range:** 6 - 99 mph.  
**Direction:** AB , Lane = 0-16  
**Separation:** Headway > 0 sec, Span 0 - 328.084 ft  
**Name:** Default Profile  
**Scheme:** Vehicle classification (Scheme F3)  
**Units:** Non metric (ft, mi, ft/s, mph, lb, ton)  
**In profile:** Vehicles = 3122 / 6073 (51.41%)

## Weekly Vehicle Counts (Virtual Week)

**VirtWeeklyVehicle-141****Site:** 120-269.2.3EW**Description:** 400ft from 17M**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022**Scheme:** Vehicle classification (Scheme F3)**Filter:** Cls(1-13) Dir(AB) Sp(6,99) Headway(>0) Span(0 - 328.084) Lane(0-16)

	<b>Mon</b>	<b>Tue</b>	<b>Wed</b>	<b>Thu</b>	<b>Fri</b>	<b>Sat</b>	<b>Sun</b>	<b>Averages</b>	
								<b>1 - 5</b>	<b>1 - 7</b>
<b>Hour</b>									
0000-0100	5.0	4.0	*	3.0	3.0	3.0	4.0	3.8	3.7
0100-0200	1.0	1.0	*	0.0	4.0	2.0	0.0	1.5	1.3
0200-0300	1.0	2.0	*	0.0	1.0	4.0	2.0	1.0	1.7
0300-0400	1.0	0.0	*	0.0	1.0	0.0	0.0	0.5	0.3
0400-0500	4.0	3.0	*	2.0	2.0	2.0	0.0	2.8	2.2
0500-0600	11.0	10.0	*	14.0	15.0	1.0	2.0	12.5	8.8
0600-0700	24.0	*	*	24.0	24.0	7.0	2.0	24.0	16.2
0700-0800	41.0	*	*	42.0	45.0	11.0	6.0	42.7	29.0
0800-0900	47.0	*	*	53.0	46.0	28.0	16.0	48.7	38.0
0900-1000	39.0	*	*	36.0	38.0	26.0	31.0	37.7	34.0
1000-1100	32.0	*	25.0	27.0	37.0	27.0	26.0	30.3	29.0
1100-1200	33.0	*	37.0	33.0	31.0	37.0	37.0	33.5	34.7
1200-1300	35.0	*	38.0	39.0	35.0	30.0	27.0	36.8	34.0
1300-1400	36.0	*	32.0	37.0	83.0	42.0	32.0	47.0	43.7
1400-1500	55.0	*	42.0	35.0	39.0	22.0	32.0	42.8	37.5
1500-1600	53.0	*	56.0	37.0	44.0	26.0	20.0	47.5	39.3
1600-1700	59.0	*	65.0	54.0	62.0	28.0	31.0	60.0	49.8
1700-1800	84.0	*	70.0	72.0	55.0	24.0	38.0	70.3	57.2
1800-1900	30.0	*	34.0	39.0	27.0	19.0	20.0	32.5	28.2
1900-2000	11.0	*	16.0	20.0	19.0	16.0	20.0	16.5	17.0
2000-2100	16.0	*	11.0	19.0	7.0	13.0	10.0	13.3	12.7
2100-2200	10.0	*	12.0	15.0	15.0	12.0	8.0	13.0	12.0
2200-2300	4.0	*	2.0	6.0	7.0	6.0	4.0	4.8	4.8
2300-2400	3.0	*	6.0	1.0	6.0	4.0	9.0	4.0	4.8
<b>Totals</b>									
0700-1900	544.0	*	*	504.0	542.0	320.0	316.0	529.5	454.3
0600-2200	605.0	*	*	582.0	607.0	368.0	356.0	596.3	512.2
0600-0000	612.0	*	*	589.0	620.0	378.0	369.0	605.0	521.9
0000-0000	635.0	*	*	608.0	646.0	390.0	377.0	627.0	539.9
<b>AM Peak</b>	0800 47.0	*	*	0800 53.0	0800 46.0	1100 37.0	1100 37.0		
<b>PM Peak</b>	1700 84.0	*	1700 70.0	1700 72.0	1300 83.0	1300 42.0	1700 38.0		

\* - No data.

## MetroCount Traffic Executive

### Weekly Vehicle Counts (Virtual Week)

#### VirtWeeklyVehicle-142 -- English (ENU)

##### Datasets:

**Site:** [120-269] 400ft from 17M  
**Attribute:** Craigville Rd  
**Direction:** 8 - East bound A>B, West bound B>A. **Lane:** 2  
**Survey Duration:** 21:54 Tuesday, March 1, 2022 => 7:38 Tuesday, March 8, 2022,  
**Zone:**  
**File:** 120-269 0 2022-03-08 0738\_Craigville.EC2 (Plus )  
**Identifier:** BG78EVVB MC56-L5 [MC55] (c)Microcom 19Oct04  
**Algorithm:** Factory default axle (v4.08)  
**Data type:** Axle sensors - Paired (Class/Speed/Count)

##### Profile:

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022 (5.83333)  
**Included classes:** 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13  
**Speed range:** 6 - 99 mph.  
**Direction:** BA , Lane = 0-16  
**Separation:** Headway > 0 sec, Span 0 - 328.084 ft  
**Name:** Default Profile  
**Scheme:** Vehicle classification (Scheme F3)  
**Units:** Non metric (ft, mi, ft/s, mph, lb, ton)  
**In profile:** Vehicles = 2835 / 6073 (46.68%)

## Weekly Vehicle Counts (Virtual Week)

**VirtWeeklyVehicle-142**

**Site:** 120-269.2.3EW

**Description:** 400ft from 17M

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022

**Scheme:** Vehicle classification (Scheme F3)

**Filter:** Cls(1-13) Dir(BA) Sp(6,99) Headway(>0) Span(0 - 328.084) Lane(0-16)

	Mon	Tue	Wed	Thu	Fri	Sat	Sun	Averages	
								1 - 5	1 - 7
<b>Hour</b>									
0000-0100	1.0	1.0	*	3.0	2.0	4.0	5.0	1.8	2.7
0100-0200	3.0	1.0	*	1.0	4.0	2.0	3.0	2.3	2.3
0200-0300	0.0	3.0	*	0.0	0.0	3.0	1.0	0.8	1.2
0300-0400	1.0	1.0	*	0.0	0.0	1.0	0.0	0.5	0.5
0400-0500	0.0	0.0	*	1.0	1.0	1.0	0.0	0.5	0.5
0500-0600	3.0	2.0	*	5.0	2.0	0.0	2.0	3.0	2.3
0600-0700	15.0	*	*	16.0	18.0	5.0	3.0	16.3	11.4
0700-0800	31.0	*	*	27.0	27.0	8.0	4.0	28.3	19.4
0800-0900	21.0	*	*	36.0	25.0	17.0	10.0	27.3	21.8
0900-1000	31.0	*	*	28.0	27.0	19.0	18.0	28.7	24.6
1000-1100	23.0	*	25.0	24.0	32.0	21.0	23.0	26.0	24.7
1100-1200	31.0	*	26.0	33.0	29.0	32.0	23.0	29.8	29.0
1200-1300	40.0	*	27.0	35.0	32.0	34.0	32.0	33.5	33.3
1300-1400	36.0	*	33.0	39.0	36.0	35.0	35.0	36.0	35.7
1400-1500	44.0	*	46.0	33.0	46.0	31.0	36.0	42.3	39.3
1500-1600	53.0	*	60.0	36.0	42.0	28.0	34.0	47.8	42.2
1600-1700	56.0	*	64.0	68.0	49.0	36.0	31.0	59.3	50.7
1700-1800	62.0	*	63.0	59.0	50.0	21.0	37.0	58.5	48.7
1800-1900	28.0	*	41.0	37.0	31.0	25.0	24.0	34.3	31.0
1900-2000	20.0	*	26.0	31.0	18.0	21.0	24.0	23.8	23.3
2000-2100	16.0	*	25.0	21.0	18.0	15.0	16.0	20.0	18.5
2100-2200	9.0	*	7.0	15.0	13.0	10.0	9.0	11.0	10.5
2200-2300	3.0	*	8.0	5.0	10.0	9.0	6.0	6.5	6.8
2300-2400	5.0	*	4.0	5.0	6.0	8.0	2.0	5.0	5.0
<b>Totals</b>									
0700-1900	456.0	*	*	455.0	426.0	307.0	307.0	451.6	400.3
0600-2200	516.0	*	*	538.0	493.0	358.0	359.0	522.7	464.0
0600-0000	524.0	*	*	548.0	509.0	375.0	367.0	534.2	475.9
0000-0000	532.0	*	*	558.0	518.0	386.0	378.0	542.9	485.4
<b>AM Peak</b>	1100 31.0	*	*	0800 36.0	1000 32.0	1100 32.0	1100 23.0		
<b>PM Peak</b>	1700 62.0	*	1600 64.0	1600 68.0	1700 50.0	1600 36.0	1700 37.0		

\* - No data.

## MetroCount Traffic Executive

### Weekly Vehicle Counts (Virtual Week)

#### VirtWeeklyVehicle-137 -- English (ENU)

**Datasets:**

**Site:** [120-269] Ref Mkr 17M-8301-3139  
**Attribute:** [+51.477222 +0.000000]  
**Direction:** 7 - North bound A>B, South bound B>A. **Lane:** 1  
**Survey Duration:** 21:29 Tuesday, March 1, 2022 => 7:35 Tuesday, March 8, 2022,  
**Zone:**  
**File:** 120-269 0 2022-03-08 0736\_17M.EC1 (Plus )  
**Identifier:** S1328N62 MC56-L5 [MC55] (c)Microcom 19Oct04  
**Algorithm:** Factory default axle (v4.08)  
**Data type:** Axle sensors - Paired (Class/Speed/Count)

**Profile:**

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022 (5.83333)  
**Included classes:** 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13  
**Speed range:** 6 - 99 mph.  
**Direction:** North, East, South, West (bound), P = North, Lane = 0-16  
**Separation:** Headway > 0 sec, Span 0 - 328.084 ft  
**Name:** Default Profile  
**Scheme:** Vehicle classification (Scheme F3)  
**Units:** Non metric (ft, mi, ft/s, mph, lb, ton)  
**In profile:** Vehicles = 27577 / 28075 (98.23%)

## Weekly Vehicle Counts (Virtual Week)

**VirtWeeklyVehicle-137**

**Site:** 120-269.1.2NS

**Description:** Ref Mkr 17M-8301-3139

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022

**Scheme:** Vehicle classification (Scheme F3)

**Filter:** Cls(1-13) Dir(NESW) Sp(6,99) Headway(>0) Span(0 - 328.084) Lane(0-16)

<b>Hour</b>	<b>Mon</b>	<b>Tue</b>	<b>Wed</b>	<b>Thu</b>	<b>Fri</b>	<b>Sat</b>	<b>Sun</b>	<b>Averages</b>		
									1 - 5	1 - 7
0000-0100	25.0	26.0	*	31.0	21.0	35.0	31.0	25.8	28.2	
0100-0200	12.0	9.0	*	6.0	15.0	18.0	21.0	10.5	13.5	
0200-0300	6.0	8.0	*	6.0	12.0	9.0	12.0	8.0	8.8	
0300-0400	5.0	4.0	*	4.0	6.0	11.0	1.0	4.8	5.2	
0400-0500	19.0	12.0	*	23.0	14.0	8.0	4.0	17.0	13.3	
0500-0600	43.0	56.0	*	47.0	59.0	15.0	11.0	51.3	38.5	
0600-0700	128.0	*	*	124.0	122.0	42.0	31.0	124.7	89.4	
0700-0800	330.0	*	*	288.0	270.0	96.0	53.0	296.0	207.4	
0800-0900	344.0	*	*	361.0	353.0	204.0	97.0	352.7	271.8	
0900-1000	320.0	*	*	347.0	329.0	263.0	177.0	332.0	287.2	
1000-1100	335.0	*	356.0	329.0	360.0	318.0	217.0	345.0	319.2	
1100-1200	345.0	*	347.0	368.0	342.0	350.0	234.0	350.5	331.0	
1200-1300	348.0	*	395.0	359.0	349.0	358.0	301.0	362.8	351.7	
1300-1400	381.0	*	383.0	373.0	416.0	382.0	310.0	388.3	374.2	
1400-1500	394.0	*	430.0	428.0	433.0	335.0	322.0	421.3	390.3	
1500-1600	414.0	*	513.0	430.0	443.0	327.0	320.0	450.0	407.8	
1600-1700	455.0	*	486.0	465.0	495.0	282.0	299.0	475.3	413.7	
1700-1800	469.0	*	510.0	466.0	416.0	256.0	253.0	465.3	395.0	
1800-1900	285.0	*	338.0	352.0	297.0	234.0	184.0	318.0	281.7	
1900-2000	188.0	*	228.0	235.0	190.0	201.0	162.0	210.3	200.7	
2000-2100	127.0	*	176.0	143.0	121.0	137.0	99.0	141.8	133.8	
2100-2200	81.0	*	84.0	85.0	90.0	100.0	68.0	85.0	84.7	
2200-2300	44.0	*	58.0	50.0	67.0	58.0	57.0	54.8	55.7	
2300-2400	21.0	*	48.0	27.0	46.0	47.0	28.0	35.5	36.2	
<hr/>										
<b>Totals</b>										
0700-1900	4420.0	*	*	4566.0	4503.0	3405.0	2767.0	4556.9	4030.9	
0600-2200	4944.0	*	*	5153.0	5026.0	3885.0	3127.0	5118.6	4539.5	
0600-0000	5009.0	*	*	5230.0	5139.0	3990.0	3212.0	5208.8	4631.3	
0000-0000	5119.0	*	*	5347.0	5266.0	4086.0	3292.0	5326.1	4738.8	
<b>AM Peak</b>	1100	*	*	1100	1000	1100	1100			
	345.0	*	*	368.0	360.0	350.0	234.0			
<b>PM Peak</b>	1700	*	1500	1700	1600	1300	1400			
	469.0	*	513.0	466.0	495.0	382.0	322.0			

\* - No data.

## MetroCount Traffic Executive

### Weekly Vehicle Counts (Virtual Week)

#### VirtWeeklyVehicle-138 -- English (ENU)

**Datasets:**

**Site:** [120-269] Ref Mkr 17M-8301-3139  
**Attribute:** [+51.477222 +0.000000]  
**Direction:** 7 - North bound A>B, South bound B>A. **Lane:** 1  
**Survey Duration:** 21:29 Tuesday, March 1, 2022 => 7:35 Tuesday, March 8, 2022,  
**Zone:**  
**File:** 120-269 0 2022-03-08 0736\_17M.EC1 (Plus )  
**Identifier:** S1328N62 MC56-L5 [MC55] (c)Microcom 19Oct04  
**Algorithm:** Factory default axle (v4.08)  
**Data type:** Axle sensors - Paired (Class/Speed/Count)

**Profile:**

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022 (5.83333)  
**Included classes:** 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13  
**Speed range:** 6 - 99 mph.  
**Direction:** AB , Lane = 0-16  
**Separation:** Headway > 0 sec, Span 0 - 328.084 ft  
**Name:** Default Profile  
**Scheme:** Vehicle classification (Scheme F3)  
**Units:** Non metric (ft, mi, ft/s, mph, lb, ton)  
**In profile:** Vehicles = 16817 / 28075 (59.90%)

## Weekly Vehicle Counts (Virtual Week)

**VirtWeeklyVehicle-138**

**Site:** 120-269.1.2NS

**Description:** Ref Mkr 17M-8301-3139

**Filter time:** 10:00 Wednesday, March 2, 2022 => 6:00 Tuesday, March 8, 2022

**Scheme:** Vehicle classification (Scheme F3)

**Filter:** Cls(1-13) Dir(AB) Sp(6,99) Headway(>0) Span(0 - 328.084) Lane(0-16)

<b>Hour</b>	<b>Mon</b>	<b>Tue</b>	<b>Wed</b>	<b>Thu</b>	<b>Fri</b>	<b>Sat</b>	<b>Sun</b>	<b>Averages</b>		
									1 - 5	1 - 7
0000-0100	13.0	17.0	*	17.0	13.0	18.0	21.0	15.0	16.5	
0100-0200	7.0	6.0	*	5.0	11.0	10.0	17.0	7.3	9.3	
0200-0300	3.0	5.0	*	4.0	5.0	6.0	6.0	4.3	4.8	
0300-0400	3.0	1.0	*	2.0	2.0	7.0	0.0	2.0	2.5	
0400-0500	6.0	4.0	*	8.0	7.0	5.0	3.0	6.3	5.5	
0500-0600	23.0	26.0	*	20.0	30.0	8.0	5.0	24.8	18.7	
0600-0700	77.0	*	*	74.0	71.0	22.0	17.0	74.0	52.2	
0700-0800	139.0	*	*	149.0	147.0	57.0	30.0	145.0	104.4	
0800-0900	152.0	*	*	194.0	187.0	112.0	51.0	177.7	139.2	
0900-1000	181.0	*	*	197.0	182.0	161.0	88.0	186.7	161.8	
1000-1100	218.0	*	220.0	208.0	<b>218.0</b>	194.0	<b>137.0</b>	216.0	199.2	
1100-1200	<b>223.0</b>	*	224.0	<b>219.0</b>	213.0	<b>206.0</b>	134.0	<b>219.8</b>	<b>203.2</b>	
1200-1300	203.0	*	237.0	224.0	210.0	207.0	192.0	218.5	212.2	
1300-1400	231.0	*	226.0	232.0	232.0	215.0	191.0	230.3	221.2	
1400-1500	237.0	*	270.0	253.0	255.0	<b>217.0</b>	198.0	253.8	238.3	
1500-1600	261.0	*	335.0	268.0	296.0	199.0	<b>201.0</b>	290.0	260.0	
1600-1700	303.0	*	322.0	320.0	<b>340.0</b>	178.0	160.0	<b>321.3</b>	<b>270.5</b>	
1700-1800	<b>322.0</b>	*	<b>356.0</b>	<b>322.0</b>	276.0	151.0	145.0	319.0	262.0	
1800-1900	177.0	*	216.0	216.0	201.0	140.0	98.0	202.5	174.7	
1900-2000	119.0	*	144.0	152.0	117.0	112.0	86.0	133.0	121.7	
2000-2100	92.0	*	130.0	95.0	78.0	87.0	59.0	98.8	90.2	
2100-2200	53.0	*	54.0	61.0	58.0	53.0	41.0	56.5	53.3	
2200-2300	30.0	*	38.0	31.0	49.0	36.0	35.0	37.0	36.5	
2300-2400	11.0	*	23.0	18.0	28.0	31.0	17.0	20.0	21.3	
<hr/>										
<b>Totals</b>										
0700-1900	2647.0	*	*	2802.0	2757.0	2037.0	1625.0	2780.3	2446.6	
0600-2200	2988.0	*	*	3184.0	3081.0	2311.0	1828.0	3142.6	2763.9	
0600-0000	3029.0	*	*	3233.0	3158.0	2378.0	1880.0	3199.6	2821.8	
0000-0000	3084.0	*	*	3289.0	3226.0	2432.0	1932.0	3259.1	2879.1	
<hr/>										
<b>AM Peak</b>	1100	*	*	1100	1000	1100	1000			
	223.0	*	*	219.0	218.0	206.0	137.0			
<hr/>										
<b>PM Peak</b>	1700	*	1700	1700	1600	1400	1500			
	322.0	*	356.0	322.0	340.0	217.0	201.0			

\* - No data.

## MetroCount Traffic Executive

### Weekly Vehicle Counts (Virtual Week)

#### VirtWeeklyVehicle-139 -- English (ENU)

**Datasets:**

**Site:** [120-269] Ref Mkr 17M-8301-3139  
**Attribute:** [+51.477222 +0.000000]  
**Direction:** 7 - North bound A>B, South bound B>A. **Lane:** 1  
**Survey Duration:** 21:29 Tuesday, March 1, 2022 => 7:35 Tuesday, March 8, 2022,  
**Zone:**  
**File:** 120-269 0 2022-03-08 0736\_17M.EC1 (Plus )  
**Identifier:** S1328N62 MC56-L5 [MC55] (c)Microcom 19Oct04  
**Algorithm:** Factory default axle (v4.08)  
**Data type:** Axle sensors - Paired (Class/Speed/Count)

**Profile:**

**Filter time:** 10:00 Wednesday, March 2, 2022 => 7:00 Tuesday, March 8, 2022 (5.875)  
**Included classes:** 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13  
**Speed range:** 6 - 99 mph.  
**Direction:** BA , Lane = 0-16  
**Separation:** Headway > 0 sec, Span 0 - 328.084 ft  
**Name:** Default Profile  
**Scheme:** Vehicle classification (Scheme F3)  
**Units:** Non metric (ft, mi, ft/s, mph, lb, ton)  
**In profile:** Vehicles = 10810 / 28075 (38.50%)

## Weekly Vehicle Counts (Virtual Week)

**VirtWeeklyVehicle-139**

**Site:** 120-269.1.2NS

**Description:** Ref Mkr 17M-8301-3139

**Filter time:** 10:00 Wednesday, March 2, 2022 => 7:00 Tuesday, March 8, 2022

**Scheme:** Vehicle classification (Scheme F3)

**Filter:** Cls(1-13) Dir(BA) Sp(6,99) Headway(>0) Span(0 - 328.084) Lane(0-16)

<b>Hour</b>	<b>Mon</b>	<b>Tue</b>	<b>Wed</b>	<b>Thu</b>	<b>Fri</b>	<b>Sat</b>	<b>Sun</b>	<b>Averages</b>	
								1 - 5	1 - 7
0000-0100	12.0	9.0	*	14.0	8.0	17.0	10.0	10.8	11.7
0100-0200	5.0	3.0	*	1.0	4.0	8.0	4.0	3.3	4.2
0200-0300	3.0	3.0	*	2.0	7.0	3.0	6.0	3.8	4.0
0300-0400	2.0	3.0	*	2.0	4.0	4.0	1.0	2.8	2.7
0400-0500	13.0	8.0	*	15.0	7.0	3.0	1.0	10.8	7.8
0500-0600	20.0	30.0	*	27.0	29.0	7.0	6.0	26.5	19.8
0600-0700	51.0	50.0	*	50.0	51.0	20.0	14.0	50.5	39.3
0700-0800	191.0	*	*	139.0	123.0	39.0	23.0	151.0	103.0
0800-0900	192.0	*	*	167.0	166.0	92.0	46.0	175.0	132.6
0900-1000	139.0	*	*	150.0	147.0	102.0	89.0	145.3	125.4
1000-1100	117.0	*	136.0	121.0	142.0	124.0	80.0	129.0	120.0
1100-1200	122.0	*	123.0	149.0	129.0	144.0	100.0	130.8	127.8
1200-1300	145.0	*	158.0	135.0	139.0	151.0	109.0	144.3	139.5
1300-1400	150.0	*	157.0	141.0	184.0	167.0	119.0	158.0	153.0
1400-1500	157.0	*	160.0	175.0	178.0	118.0	124.0	167.5	152.0
1500-1600	153.0	*	178.0	162.0	147.0	128.0	119.0	160.0	147.8
1600-1700	152.0	*	164.0	145.0	155.0	104.0	139.0	154.0	143.2
1700-1800	147.0	*	154.0	144.0	140.0	105.0	108.0	146.3	133.0
1800-1900	108.0	*	122.0	136.0	96.0	94.0	86.0	115.5	107.0
1900-2000	69.0	*	84.0	83.0	73.0	89.0	76.0	77.3	79.0
2000-2100	35.0	*	46.0	48.0	43.0	50.0	40.0	43.0	43.7
2100-2200	28.0	*	30.0	24.0	32.0	47.0	27.0	28.5	31.3
2200-2300	14.0	*	20.0	19.0	18.0	22.0	22.0	17.8	19.2
2300-2400	10.0	*	25.0	9.0	18.0	16.0	11.0	15.5	14.8
<hr/>									
<b>Totals</b>									
0700-1900	1773.0	*	*	1764.0	1746.0	1368.0	1142.0	1776.6	1584.3
0600-2200	1956.0	*	*	1969.0	1945.0	1574.0	1299.0	1975.8	1777.7
0600-0000	1980.0	*	*	1997.0	1981.0	1612.0	1332.0	2009.1	1811.7
0000-0000	2035.0	*	*	2058.0	2040.0	1654.0	1360.0	2066.8	1861.8
<hr/>									
<b>AM Peak</b>	0800	*	*	0800	0800	1100	1100		
	192.0	*	*	167.0	166.0	144.0	100.0		
<hr/>									
<b>PM Peak</b>	1400	*	1500	1400	1300	1300	1600		
	157.0	*	178.0	175.0	184.0	167.0	139.0		

\* - No data.

**ATTACHMENT D**  
**NYSDOT SDV APPROVAL LETTER**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK



# **Department of Transportation**

**ANDREW M. CUOMO**  
Governor

**MARIE THERESE DOMINGUEZ**  
Commissioner

**LANCE MacMILLAN, P.E.**  
Regional Director

July 14, 2021

**Mr. Frank A. Filiciotto, PE  
Branch Manager  
Creighton Manning  
145 Main Street, 3<sup>rd</sup> Floor  
Ossining, NY 10562**

Re: T21-113

Dear Mr. Filiciotto:

Thank you for your letter to the New York State Department of Transportation (NYSDOT) dated June 4, 2021 requesting the following roads be designated as access highways for Special Dimension Vehicles (53' tractor trailers) within the Towns of Chester and Blooming Grove:

- Route 17M from CR 13 to CR 51 in the Towns of Chester and Blooming Grove, a distance of approximately 2.6 miles; and
  - CR 51 from Route 17M to the warehouse site on CR 51, a distance of approximately 3000 feet.

Please know that on September 1, 2021 it will be legal to drive special dimension vehicles on this segment on Route 17M.

As previously mentioned, the distance of 3000 feet on CR 51 from the intersection of Route 17M is within one-road mile from a qualifying highway (Route 17) and has automatic access.

Law enforcement officials are being notified of this change by copy of this letter.

Thank you for your interest in traffic safety and for bringing your concerns to our attention. If you require further information on this request, please contact the Regional Traffic and Safety Group at (845) 437-3320.

Sincerely,

Sincerely,  
  
Gayle Sudder  
Transportation Analyst

cc: D. Carey, Office of Traffic Safety and Mobility, Traffic Operations Bureau, POD 53  
S. Quadri, Resident Engineer, Residency 8-4  
R. Gaupman, Resident Engineer, Residency 8-5  
Sgt. M. Belgiovene, New York State Police, Troop F  
Chief D. Doellinger, Town of Chester Police Department  
Chief A. Melchiorre, Town of Blooming Grove Police Department

**ATTACHMENT E  
LOS REPORTS**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK

## **LOS Definitions**

The following is an excerpt from the Highway Capacity Manual, 6<sup>th</sup> Edition (HCM).

### **Level of Service for Signalized Intersections**

Level of Service (LOS) can be characterized for the entire intersection, each intersection approach, and each lane group. Control delay alone is used to characterize LOS for the entire intersection or an approach. Control delay *and* volume-to-capacity (v/c) ratio are used to characterize LOS for a lane group. Delay quantifies the increase in travel time due to traffic signal control. It is also a surrogate measure of driver discomfort and fuel consumption. The v/c ratio quantifies the degree to which a phase's capacity is utilized by a lane group. The following paragraphs describe each LOS.

**LOS A** describes operations with a control delay of 10 s/veh or less and a v/c ratio no greater than 1.0. This level is typically assigned when the v/c ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

**LOS B** describes operations with control delay between 10 and 20 s/veh and a v/c ratio no greater than 1.0. This level is typically assigned when the v/c ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

**LOS C** describes operations with control delay between 20 and 35 s/veh and a v/c ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual *cycle failures* (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

**LOS D** describes operations with control delay between 35 and 55 s/veh and a v/c ratio no greater than 1.0. This level is typically assigned when the v/c ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

**LOS E** describes operations with control delay between 55 and 80 s/veh and a v/c ratio no greater than 1.0. This level is typically assigned when the v/c ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

**LOS F** describes operations with control delay exceeding 80 s/veh or a v/c ratio greater than 1.0. This level is typically assigned when the v/c ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

A lane group can incur a delay less than 80 s/veh when the v/c ratio exceeds 1.0. This condition typically occurs when the cycle length is short, the signal progression is favorable, or both. As a result, both the delay and v/c ratio are considered when lane group LOS is established. A ratio of 1.0 or more indicates that cycle capacity is fully utilized and represents failure from a capacity perspective (just as delay in excess of 80 s/veh represents failure from a delay perspective).

Average control delay and queue length at roundabout controlled intersections are calculated using SIDRA Intersection. The physical geometry such as entry lane width and approach flare, and traffic volume at the roundabout are factors that influence the intersection's performance. The average delay reported using SIDRA Intersection is based on the signalized HCM Method of Delay for Level-of-Service.

### **Level of Service Criteria for Unsignalized Intersections**

Level of service (LOS) for Two-Way Stop-Controlled (TWSC) intersections is determined by the computed or measured control delay. For motor vehicles, LOS is determined for each minor-street movement (or shared movement) as well as major-street left turns by using criteria given in Exhibit 20-2. LOS is not defined for the intersection as a whole or for major-street approaches for three primary reasons: (a) major-street through vehicles are assumed to experience zero delay; (b) the disproportionate number of major-street through vehicles at a typical TWSC intersection skews the weighted average of all movements, resulting in a very low overall average delay for all vehicles; and (c) the resulting low delay can mask important LOS deficiencies for minor movements. LOS F is assigned to the movement if the volume-to-capacity (v/c) ratio for the movement exceeds 1.0, regardless of the control delay.

The LOS criteria for TWSC intersections are somewhat different from the criteria used in Chapter 18 for signalized intersections, primarily because user perceptions differ among transportation facility types. The expectation is that a signalized intersection is designed to carry higher traffic volumes and will present greater delay than an unsignalized intersection. Unsignalized intersections are also associated with more uncertainty for users, as delays are less predictable than they are at signals, which can reduce users' delay tolerance.

The LOS criteria for All-Way Stop-Controlled (AWSC) intersections are given in Exhibit 21-8. LOS F is assigned if the v/c ratio of a lane exceeds 1.0, regardless of the control delay. For assessment of LOS at the approach and intersection levels, LOS is based solely on control delay.

**Exhibits 20-2/21-8:  
Level-of-Service Criteria for Stop Controlled Intersections**

Control Delay (s/veh)	LOS by Volume-to-Capacity Ratio	
	v/c $\leq$ 1.0	v/c $\geq$ 1.0
10.0	A	F
>10.0 and $\leq$ 15.0	B	F
>15.0 and $\leq$ 25.0	C	F
>25.0 and $\leq$ 35.0	D	F
>35.0 and $\leq$ 50.0	E	F
>50.0	F	F

Intersection						
Int Delay, s/veh	1.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	10	7	0	42	43	0
Future Vol, veh/h	10	7	0	42	43	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	14	17	0	13	10	0
Mvmt Flow	11	8	0	47	48	0
Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	95	48	-	0	-	0
Stage 1	48	-	-	-	-	-
Stage 2	47	-	-	-	-	-
Critical Hdwy	6.54	6.37	-	-	-	-
Critical Hdwy Stg 1	5.54	-	-	-	-	-
Critical Hdwy Stg 2	5.54	-	-	-	-	-
Follow-up Hdwy	3.626	3.453	-	-	-	-
Pot Cap-1 Maneuver	876	980	0	-	-	0
Stage 1	945	-	0	-	-	0
Stage 2	946	-	0	-	-	0
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver	876	980	-	-	-	-
Mov Cap-2 Maneuver	876	-	-	-	-	-
Stage 1	945	-	-	-	-	-
Stage 2	946	-	-	-	-	-
Approach	EB	NB	SB			
HCM Control Delay, s	9	0	0			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	EBLn1	SBT			
Capacity (veh/h)	-	916	-			
HCM Lane V/C Ratio	-	0.021	-			
HCM Control Delay (s)	-	9	-			
HCM Lane LOS	-	A	-			
HCM 95th %tile Q(veh)	-	0.1	-			

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	12	100	163	30	33	17
Future Vol, veh/h	12	100	163	30	33	17
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	6	7	17	9	12
Mvmt Flow	13	110	179	33	36	19
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	212	0	-	0	332	196
Stage 1	-	-	-	-	196	-
Stage 2	-	-	-	-	136	-
Critical Hdwy	4.1	-	-	-	6.49	6.32
Critical Hdwy Stg 1	-	-	-	-	5.49	-
Critical Hdwy Stg 2	-	-	-	-	5.49	-
Follow-up Hdwy	2.2	-	-	-	3.581	3.408
Pot Cap-1 Maneuver	1370	-	-	-	649	820
Stage 1	-	-	-	-	821	-
Stage 2	-	-	-	-	874	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1370	-	-	-	643	820
Mov Cap-2 Maneuver	-	-	-	-	643	-
Stage 1	-	-	-	-	813	-
Stage 2	-	-	-	-	874	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.8	0	10.6			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1370	-	-	-	694	
HCM Lane V/C Ratio	0.01	-	-	-	0.079	
HCM Control Delay (s)	7.7	0	-	-	10.6	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.3	

Intersection

Int Delay, s/veh 3.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	228	112	0	0	174	0	0	0	0	0	0	0
Future Vol, veh/h	228	112	0	0	174	0	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	9	5	0	0	8	0	2	2	2	2	2	2
Mvmt Flow	253	124	0	0	193	0	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	193	0	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.19	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.281	-	-
Pot Cap-1 Maneuver	1339	-	0
Stage 1	-	0	0
Stage 2	-	0	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1339	-	0
Mov Cap-2 Maneuver	-	-	0
Stage 1	-	-	0
Stage 2	-	-	0

Approach	EB	WB	NE
HCM Control Delay, s	5.6	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1339	-
HCM Lane V/C Ratio	-	0.189	-
HCM Control Delay (s)	0	8.3	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.7	-

**Intersection**

Int Delay, s/veh 5.6

**Movement** WBL WBR NBT NBR SBL SBT

Lane Configurations	WBL	WBR	NBT	NBR	SBL	SBT
Traffic Vol, veh/h	5	44	31	21	146	38
Future Vol, veh/h	5	44	31	21	146	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	25	11	19	5	8	18
Mvmt Flow	6	54	38	26	178	46

**Major/Minor** Minor1 Major1 Major2

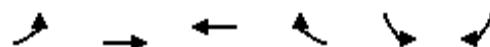
Conflicting Flow All	453	51	0	0	64	0
Stage 1	51	-	-	-	-	-
Stage 2	402	-	-	-	-	-
Critical Hdwy	6.65	6.31	-	-	4.18	-
Critical Hdwy Stg 1	5.65	-	-	-	-	-
Critical Hdwy Stg 2	5.65	-	-	-	-	-
Follow-up Hdwy	3.725	3.399	-	-	2.272	-
Pot Cap-1 Maneuver	524	992	-	-	1501	-
Stage 1	916	-	-	-	-	-
Stage 2	629	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	460	992	-	-	1501	-
Mov Cap-2 Maneuver	460	-	-	-	-	-
Stage 1	916	-	-	-	-	-
Stage 2	552	-	-	-	-	-

**Approach** WB NB SB

HCM Control Delay, s 9.4 0 6.1

HCM LOS A

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	887	1501	-
HCM Lane V/C Ratio	-	-	0.067	0.119	-
HCM Control Delay (s)	-	-	9.4	7.7	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.2	0.4	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	15	333	174	0	15	136
Future Volume (vph)	15	333	174	0	15	136
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.88	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1763	1810		1463	
Flt Permitted		0.99	1.00		1.00	
Satd. Flow (perm)		1741	1810		1463	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	17	383	200	0	17	156
RTOR Reduction (vph)	0	0	0	0	124	0
Lane Group Flow (vph)	0	400	200	0	49	0
Heavy Vehicles (%)	20%	7%	5%	0%	0%	15%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		81.4	35.9		23.9	
Effective Green, g (s)		81.4	35.9		23.9	
Actuated g/C Ratio		0.71	0.31		0.21	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)		1229	563		303	
v/s Ratio Prot			c0.11		c0.03	
v/s Ratio Perm			c0.23			
v/c Ratio		0.33	0.36		0.16	
Uniform Delay, d1		6.5	30.7		37.5	
Progression Factor		0.15	1.00		1.00	
Incremental Delay, d2		0.4	1.1		0.3	
Delay (s)		1.4	31.8		37.7	
Level of Service		A	C		D	
Approach Delay (s)		1.4	31.8		37.7	
Approach LOS		A	C		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		17.4		HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio		0.31				
Actuated Cycle Length (s)		115.3		Sum of lost time (s)		15.0
Intersection Capacity Utilization		47.3%		ICU Level of Service		A
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120-269; Craigville Logistics Warehouse

Existing\_AM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↙	↖	↗
Traffic Volume (vph)	181	277	116	194	365	167
Future Volume (vph)	181	277	116	194	365	167
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1759	1495		1690	1703	1509
Flt Permitted	1.00	1.00		0.80	0.95	1.00
Satd. Flow (perm)	1759	1495		1374	1703	1509
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	208	318	133	223	420	192
RTOR Reduction (vph)	0	219	0	0	0	34
Lane Group Flow (vph)	208	99	0	356	420	158
Heavy Vehicles (%)	8%	8%	16%	7%	6%	7%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	35.9	35.9		64.8	40.5	40.5
Effective Green, g (s)	35.9	35.9		64.8	40.5	40.5
Actuated g/C Ratio	0.31	0.31		0.56	0.35	0.35
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	547	465		772	598	530
v/s Ratio Prot	0.12			c0.25		
v/s Ratio Perm		0.07		c0.26		0.10
v/c Ratio	0.38	0.21		0.46	0.70	0.30
Uniform Delay, d1	31.0	29.3		14.9	32.2	27.1
Progression Factor	1.00	1.00		0.43	1.00	1.00
Incremental Delay, d2	1.2	0.6		1.2	6.8	1.4
Delay (s)	32.3	29.9		7.5	39.0	28.5
Level of Service	C	C		A	D	C
Approach Delay (s)	30.8			7.5	35.7	
Approach LOS	C			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		27.3		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.58				
Actuated Cycle Length (s)		115.3		Sum of lost time (s)		15.0
Intersection Capacity Utilization		59.3%		ICU Level of Service		B
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

Existing\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	63	176	134	362	187	10	100	110	236	46	206	79
Future Volume (veh/h)	63	176	134	362	187	10	100	110	236	46	206	79
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			0.99	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No		No		No	
Adj Sat Flow, veh/h/ln	1752	1811	1781	1841	1781	1574	1811	1841	1767	1752	1752	1737
Adj Flow Rate, veh/h	66	185	141	381	197	11	105	116	248	48	217	83
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	10	6	8	4	8	22	6	4	9	10	10	11
Cap, veh/h	82	338	243	532	476	27	135	506	411	57	279	107
Arrive On Green	0.05	0.18	0.18	0.16	0.28	0.28	0.08	0.27	0.27	0.03	0.23	0.23
Sat Flow, veh/h	1668	1904	1368	3401	1670	93	1725	1841	1497	1668	1207	462
Grp Volume(v), veh/h	66	166	160	381	0	208	105	116	248	48	0	300
Grp Sat Flow(s), veh/h/ln	1668	1721	1551	1700	0	1764	1725	1841	1497	1668	0	1669
Q Serve(g_s), s	2.2	4.9	5.3	6.0	0.0	5.4	3.3	2.7	8.1	1.6	0.0	9.4
Cycle Q Clear(g_c), s	2.2	4.9	5.3	6.0	0.0	5.4	3.3	2.7	8.1	1.6	0.0	9.4
Prop In Lane	1.00			1.00			0.05	1.00		1.00	1.00	0.28
Lane Grp Cap(c), veh/h	82	305	275	532	0	502	135	506	411	57	0	385
V/C Ratio(X)	0.81	0.54	0.58	0.72	0.00	0.41	0.78	0.23	0.60	0.84	0.00	0.78
Avail Cap(c_a), veh/h	447	768	693	911	0	1260	308	822	669	298	0	745
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	26.4	21.0	21.1	22.4	0.0	16.2	25.3	15.7	17.6	26.9	0.0	20.2
Incr Delay (d2), s/veh	16.7	1.5	1.9	1.8	0.0	0.5	9.4	0.2	1.4	26.6	0.0	3.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.2	1.9	1.8	2.2	0.0	1.9	1.6	1.0	2.7	1.0	0.0	3.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	43.1	22.5	23.0	24.3	0.0	16.8	34.7	15.9	19.1	53.4	0.0	23.6
LnGrp LOS	D	C	C	C	A	B	C	B	B	D	A	C
Approach Vol, veh/h						589			469			348
Approach Delay, s/veh						21.6			21.8			27.7
Approach LOS						C			C			C
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R <sub>c</sub> ), s	7.7	20.9	6.9	20.4	13.8	14.9	9.4	17.9				
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Q Clear Time (g_c+l1), s	4.2	7.4	3.6	10.1	8.0	7.3	5.3	11.4				
Green Ext Time (p_c), s	0.1	1.1	0.0	1.2	0.8	1.6	0.1	1.5				
Intersection Summary												
HCM 6th Ctrl Delay				23.8								
HCM 6th LOS				C								

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120-269; Craigville Logistics Warehouse Existing\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑↑	↑↑	
Traffic Volume (veh/h)	0	0	0	130	0	58	81	388	0	0	283	419
Future Volume (veh/h)	0	0	0	130	0	58	81	388	0	0	283	419
Initial Q (Q <sub>b</sub> ), veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		No
Adj Sat Flow, veh/h/ln				1648	1900	1826	1663	1796	0	0	1752	1826
Adj Flow Rate, veh/h				144	0	64	90	431	0	0	314	466
Peak Hour Factor				0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %				17	0	5	16	7	0	0	10	5
Cap, veh/h				235	0	201	401	1125	0	0	707	631
Arrive On Green				0.13	0.00	0.13	0.08	0.63	0.00	0.00	0.42	0.42
Sat Flow, veh/h				1810	0	1547	1584	1796	0	0	1752	1485
Grp Volume(v), veh/h				144	0	64	90	431	0	0	314	466
Grp Sat Flow(s), veh/h/ln				1810	0	1547	1584	1796	0	0	1664	1485
Q Serve(g_s), s				3.1	0.0	1.5	1.1	4.8	0.0	0.0	5.5	10.8
Cycle Q Clear(g_c), s				3.1	0.0	1.5	1.1	4.8	0.0	0.0	5.5	10.8
Prop In Lane				1.00		1.00	1.00	1.00	0.00	0.00		1.00
Lane Grp Cap(c), veh/h				235	0	201	401	1125	0	0	707	631
V/C Ratio(X)				0.61	0.00	0.32	0.22	0.38	0.00	0.00	0.44	0.74
Avail Cap(c_a), veh/h				882	0	754	1047	1314	0	0	1217	1086
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)				1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				16.9	0.0	16.2	6.7	3.8	0.0	0.0	8.4	9.9
Incr Delay (d2), s/veh				2.6	0.0	0.9	0.3	0.2	0.0	0.0	0.4	1.7
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln				1.2	0.0	0.1	0.2	0.6	0.0	0.0	1.3	2.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				19.5	0.0	17.1	7.0	4.0	0.0	0.0	8.8	11.6
LnGrp LOS				B	A	B	A	A	A	A	A	B
Approach Vol, veh/h						208			521		780	
Approach Delay, s/veh						18.7			4.5		10.5	
Approach LOS						B			A		B	
Timer - Assigned Phs	2		4	5	6							
Phs Duration (G+Y+R <sub>c</sub> ), s	30.7		10.3	8.3	22.4							
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0							
Max Green Setting (Gmax), s	30.0		20.0	20.0	30.0							
Max Q Clear Time (g <sub>c+l1</sub> ), s	6.8		5.1	3.1	12.8							
Green Ext Time (p <sub>c</sub> ), s	2.5		0.8	0.2	4.6							
Intersection Summary												
HCM 6th Ctrl Delay			9.5									
HCM 6th LOS			A									

HCM 6th Signalized Intersection **SUMMARY** Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120-269; Craigville Logistics Warehouse Existing\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	255	1	189	0	0	0	0	214	257	132	281	0
Future Volume (veh/h)	255	1	189	0	0	0	0	214	257	132	281	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No		No		
Adj Sat Flow, veh/h/ln	1796	1870	1722				0	1737	1722	1781	1693	0
Adj Flow Rate, veh/h	283	1	210				0	238	286	147	312	0
Peak Hour Factor	0.90	0.90	0.90				0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	7	2	12				0	11	12	8	14	0
Cap, veh/h	422	2	390				0	481	404	509	871	0
Arrive On Green	0.25	0.25	0.25				0.00	0.28	0.28	0.12	0.51	0.00
Sat Flow, veh/h	1711	8	1579				0	1737	1459	1697	1693	0
Grp Volume(v), veh/h	283	0	211				0	238	286	147	312	0
Grp Sat Flow(s), veh/h/ln	1711	0	1586				0	1737	1459	1697	1693	0
Q Serve(g_s), s	6.3	0.0	4.8				0.0	4.8	7.4	2.2	4.6	0.0
Cycle Q Clear(g_c), s	6.3	0.0	4.8				0.0	4.8	7.4	2.2	4.6	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	422	0	391				0	481	404	509	871	0
V/C Ratio(X)	0.67	0.00	0.54				0.00	0.49	0.71	0.29	0.36	0.00
Avail Cap(c_a), veh/h	1224	0	1135				0	1243	1044	1117	1211	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	14.3	0.0	13.7				0.0	12.7	13.6	7.8	6.1	0.0
Incr Delay (d2), s/veh	1.9	0.0	1.2				0.0	0.8	2.3	0.3	0.2	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.1	0.0	1.5				0.0	1.5	2.1	0.5	0.9	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	16.1	0.0	14.9				0.0	13.5	15.9	8.1	6.3	0.0
LnGrp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	494							524			459	
Approach Delay, s/veh	15.6							14.8			6.9	
Approach LOS	B							B			A	
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+R <sub>c</sub> ), s	10.0	16.6	15.3	26.6								
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	20.0	30.0	30.0	30.0								
Max Q Clear Time (g <sub>c+l1</sub> ), s	4.2	9.4	8.3	6.6								
Green Ext Time (p <sub>c</sub> ), s	0.3	2.2	2.1	1.7								
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay			12.6									
HCM 6th LOS			B									

**Intersection**

Int Delay, s/veh 6.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	29	133	1	10	179	28	0	51	9	87	11	54
Future Vol, veh/h	29	133	1	10	179	28	0	51	9	87	11	54
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	10	10	0	0	7	0	0	6	0	6	9	6
Mvmt Flow	33	151	1	11	203	32	0	58	10	99	13	61

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	235	0	0	152	0	0	496	475	152	493	459	219
Stage 1	-	-	-	-	-	-	218	218	-	241	241	-
Stage 2	-	-	-	-	-	-	278	257	-	252	218	-
Critical Hdwy	4.2	-	-	4.1	-	-	7.1	6.56	6.2	7.96	7.39	6.66
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.56	-	6.96	6.39	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.56	-	6.96	6.39	-
Follow-up Hdwy	2.29	-	-	2.2	-	-	3.5	4.054	3.3	3.554	4.081	3.354
Pot Cap-1 Maneuver	1287	-	-	1441	-	-	487	483	900	430	441	791
Stage 1	-	-	-	-	-	-	789	715	-	714	657	-
Stage 2	-	-	-	-	-	-	733	688	-	703	676	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1287	-	-	1441	-	-	427	465	900	374	425	791
Mov Cap-2 Maneuver	-	-	-	-	-	-	427	465	-	374	425	-
Stage 1	-	-	-	-	-	-	767	695	-	694	651	-
Stage 2	-	-	-	-	-	-	657	682	-	619	657	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	1.4	0.3			13.3			17.2			
HCM LOS					B			C			

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	501	1287	-	-	1441	-	-	465
HCM Lane V/C Ratio	0.136	0.026	-	-	0.008	-	-	0.371
HCM Control Delay (s)	13.3	7.9	0	-	7.5	0	-	17.2
HCM Lane LOS	B	A	A	-	A	A	-	C
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0	-	-	1.7

**Intersection**

Int Delay, s/veh 7.7

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		A	B		
Traffic Vol, veh/h	78	21	5	318	757	74
Future Vol, veh/h	78	21	5	318	757	74
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	6	-	-	0	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	22	19	20	14	5	5
Mvmt Flow	86	23	5	349	832	81

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1232	873	913	0	-	0
Stage 1	873	-	-	-	-	-
Stage 2	359	-	-	-	-	-
Critical Hdwy	7.82	6.99	4.3	-	-	-
Critical Hdwy Stg 1	6.82	-	-	-	-	-
Critical Hdwy Stg 2	6.82	-	-	-	-	-
Follow-up Hdwy	3.698	3.471	2.38	-	-	-
Pot Cap-1 Maneuver	118	282	677	-	-	-
Stage 1	282	-	-	-	-	-
Stage 2	590	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	117	282	677	-	-	-
Mov Cap-2 Maneuver	117	-	-	-	-	-
Stage 1	279	-	-	-	-	-
Stage 2	590	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	97.3	0.2	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	677	-	134	-	-
HCM Lane V/C Ratio	0.008	-	0.812	-	-
HCM Control Delay (s)	10.4	0	97.3	-	-
HCM Lane LOS	B	A	F	-	-
HCM 95th %tile Q(veh)	0	-	5	-	-

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	23	7	0	67	63	0
Future Vol, veh/h	23	7	0	67	63	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	0	0	4	2	0
Mvmt Flow	26	8	0	77	72	0
Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	149	72	-	0	-	0
Stage 1	72	-	-	-	-	-
Stage 2	77	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	848	996	0	-	-	0
Stage 1	956	-	0	-	-	0
Stage 2	951	-	0	-	-	0
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver	848	996	-	-	-	-
Mov Cap-2 Maneuver	848	-	-	-	-	-
Stage 1	956	-	-	-	-	-
Stage 2	951	-	-	-	-	-
Approach	EB	NB	SB			
HCM Control Delay, s	9.3	0	0			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	EBLn1	SBT			
Capacity (veh/h)	-	878	-			
HCM Lane V/C Ratio	-	0.039	-			
HCM Control Delay (s)	-	9.3	-			
HCM Lane LOS	-	A	-			
HCM 95th %tile Q(veh)	-	0.1	-			

Intersection						
Int Delay, s/veh	1.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	6	122	272	61	27	43
Future Vol, veh/h	6	122	272	61	27	43
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	2	4	5	4	0
Mvmt Flow	7	136	302	68	30	48
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	370	0	-	0	486	336
Stage 1	-	-	-	-	336	-
Stage 2	-	-	-	-	150	-
Critical Hdwy	4.1	-	-	-	6.44	6.2
Critical Hdwy Stg 1	-	-	-	-	5.44	-
Critical Hdwy Stg 2	-	-	-	-	5.44	-
Follow-up Hdwy	2.2	-	-	-	3.536	3.3
Pot Cap-1 Maneuver	1200	-	-	-	537	711
Stage 1	-	-	-	-	719	-
Stage 2	-	-	-	-	873	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1200	-	-	-	534	711
Mov Cap-2 Maneuver	-	-	-	-	534	-
Stage 1	-	-	-	-	715	-
Stage 2	-	-	-	-	873	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.4	0	11.5			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1200	-	-	-	630	
HCM Lane V/C Ratio	0.006	-	-	-	0.123	
HCM Control Delay (s)	8	0	-	-	11.5	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.4	

Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	159	126	0	0	315	1	0	0	0	0	0	0
Future Vol, veh/h	159	126	0	0	315	1	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	84	84	84	84	84	84	84	84	84	84	84	84
Heavy Vehicles, %	3	2	0	0	4	0	2	2	2	2	2	2
Mvmt Flow	189	150	0	0	375	1	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	376	0	-
Stage 1	-	-	528
Stage 2	-	-	376
Critical Hdwy	4.13	-	6.52
Critical Hdwy Stg 1	-	-	5.52
Critical Hdwy Stg 2	-	-	5.52
Follow-up Hdwy	2.227	-	4.018
Pot Cap-1 Maneuver	1177	-	0
Stage 1	-	0	528
Stage 2	-	0	616
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1177	-	0
Mov Cap-2 Maneuver	-	-	0
Stage 1	-	-	0
Stage 2	-	-	0

Approach	EB	WB	NE
HCM Control Delay, s	4.8	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1177	-
HCM Lane V/C Ratio	-	0.161	-
HCM Control Delay (s)	0	8.6	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.6	-

**Intersection**

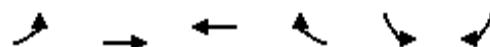
Int Delay, s/veh 7.3

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B			A	
Traffic Vol, veh/h	37	176	86	4	92	26
Future Vol, veh/h	37	176	86	4	92	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	3	5	25	10	8
Mvmt Flow	45	215	105	5	112	32

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	364	108	0	0	110
Stage 1	108	-	-	-	-
Stage 2	256	-	-	-	-
Critical Hdwy	6.4	6.23	-	-	4.2
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.327	-	-	2.29
Pot Cap-1 Maneuver	639	943	-	-	1432
Stage 1	921	-	-	-	-
Stage 2	791	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	588	943	-	-	1432
Mov Cap-2 Maneuver	588	-	-	-	-
Stage 1	921	-	-	-	-
Stage 2	728	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.1	0	6
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	853	1432	-
HCM Lane V/C Ratio	-	-	0.305	0.078	-
HCM Control Delay (s)	-	-	11.1	7.7	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	1.3	0.3	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	31	304	332	0	9	340
Future Volume (vph)	31	304	332	0	9	340
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.87	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1820	1810		1529	
Flt Permitted		0.94	1.00		1.00	
Satd. Flow (perm)		1727	1810		1529	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	32	310	339	0	9	347
RTOR Reduction (vph)	0	0	0	0	246	0
Lane Group Flow (vph)	0	342	339	0	110	0
Heavy Vehicles (%)	3%	4%	5%	0%	0%	8%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		85.0	40.0		39.1	
Effective Green, g (s)		85.0	40.0		39.1	
Actuated g/C Ratio		0.63	0.30		0.29	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)		1094	539		445	
v/s Ratio Prot			c0.19		c0.07	
v/s Ratio Perm		c0.20				
v/c Ratio		0.31	0.63		0.25	
Uniform Delay, d1		11.2	40.6		36.3	
Progression Factor		0.17	1.00		1.00	
Incremental Delay, d2		0.4	3.9		0.3	
Delay (s)		2.4	44.5		36.6	
Level of Service		A	D		D	
Approach Delay (s)		2.4	44.5		36.6	
Approach LOS		A	D		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		27.9		HCM 2000 Level of Service	C	
HCM 2000 Volume to Capacity ratio		0.40				
Actuated Cycle Length (s)		134.1		Sum of lost time (s)	15.0	
Intersection Capacity Utilization		69.2%		ICU Level of Service	C	
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120;296; Craigville Logistics Warehouse

Existing \_PM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↖	↘	↗
Traffic Volume (vph)	175	451	269	403	351	160
Future Volume (vph)	175	451	269	403	351	160
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1810	1583		1757	1752	1495
Flt Permitted	1.00	1.00		0.79	0.95	1.00
Satd. Flow (perm)	1810	1583		1417	1752	1495
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	179	460	274	411	358	163
RTOR Reduction (vph)	0	323	0	0	0	36
Lane Group Flow (vph)	179	137	0	685	358	127
Heavy Vehicles (%)	5%	2%	9%	4%	3%	8%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	40.0	40.0		84.1	40.0	40.0
Effective Green, g (s)	40.0	40.0		84.1	40.0	40.0
Actuated g/C Ratio	0.30	0.30		0.63	0.30	0.30
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	539	472		888	522	445
v/s Ratio Prot	0.10			c0.20		
v/s Ratio Perm		0.09		c0.48		0.08
v/c Ratio	0.33	0.29		0.77	0.69	0.28
Uniform Delay, d1	36.6	36.2		18.1	41.5	36.1
Progression Factor	1.00	1.00		0.78	1.00	1.00
Incremental Delay, d2	1.0	1.0		4.4	7.2	1.6
Delay (s)	37.7	37.1		18.5	48.7	37.7
Level of Service	D	D		B	D	D
Approach Delay (s)	37.3			18.5	45.2	
Approach LOS	D			B	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		32.5		HCM 2000 Level of Service	C	
HCM 2000 Volume to Capacity ratio		0.77				
Actuated Cycle Length (s)		134.1		Sum of lost time (s)	15.0	
Intersection Capacity Utilization		78.0%		ICU Level of Service	D	
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120;296; Craigville Logistics Warehouse

Existing \_PM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	112	307	210	377	355	22	261	180	250	69	151	111
Future Volume (veh/h)	112	307	210	377	355	22	261	180	250	69	151	111
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1900	1885	1841	1826	1870	1900	1870	1885	1841	1841	1856	1900
Adj Flow Rate, veh/h	114	313	214	385	362	22	266	184	255	70	154	113
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	1	4	5	2	0	2	1	4	4	3	0
Cap, veh/h	149	436	291	503	486	30	262	554	458	90	197	144
Arrive On Green	0.08	0.21	0.21	0.15	0.28	0.28	0.15	0.29	0.29	0.05	0.20	0.20
Sat Flow, veh/h	1810	2057	1372	3374	1745	106	1781	1885	1558	1753	994	729
Grp Volume(v), veh/h	114	272	255	385	0	384	266	184	255	70	0	267
Grp Sat Flow(s), veh/h/ln	1810	1791	1638	1687	0	1851	1781	1885	1558	1753	0	1723
Q Serve(g_s), s	4.2	9.6	9.9	7.5	0.0	12.8	10.0	5.2	9.4	2.7	0.0	10.0
Cycle Q Clear(g_c), s	4.2	9.6	9.9	7.5	0.0	12.8	10.0	5.2	9.4	2.7	0.0	10.0
Prop In Lane	1.00			0.84	1.00		0.06	1.00		1.00	1.00	0.42
Lane Grp Cap(c), veh/h	149	379	347	503	0	515	262	554	458	90	0	341
V/C Ratio(X)	0.76	0.72	0.74	0.77	0.00	0.75	1.02	0.33	0.56	0.78	0.00	0.78
Avail Cap(c_a), veh/h	399	658	602	744	0	1089	262	693	573	258	0	633
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	30.6	24.9	25.0	27.8	0.0	22.3	29.0	18.8	20.3	31.9	0.0	25.9
Incr Delay (d2), s/veh	7.8	2.5	3.0	2.8	0.0	2.2	59.7	0.3	1.1	13.7	0.0	3.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.0	4.0	3.8	3.0	0.0	5.3	8.3	2.1	3.3	1.4	0.0	4.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	38.4	27.4	28.1	30.6	0.0	24.5	88.7	19.1	21.3	45.6	0.0	29.8
LnGrp LOS	D	C	C	C	A	C	F	B	C	D	A	C
Approach Vol, veh/h						769			705			337
Approach Delay, s/veh						27.5			46.2			33.1
Approach LOS			C			C			D			C
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R <sub>c</sub> ), s	10.6	23.9	8.5	25.0	15.1	19.4	15.0	18.5				
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Q Clear Time (g_c+l1), s	6.2	14.8	4.7	11.4	9.5	11.9	12.0	12.0				
Green Ext Time (p_c), s	0.2	2.2	0.1	1.5	0.7	2.5	0.0	1.3				
Intersection Summary												
HCM 6th Ctrl Delay				34.2								
HCM 6th LOS				C								

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse Existing \_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑	↑	
Traffic Volume (veh/h)	0	0	0	244	0	163	222	528	0	0	313	425
Future Volume (veh/h)	0	0	0	244	0	163	222	528	0	0	313	425
Initial Q (Q <sub>b</sub> ), veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No		No		No		No		No
Adj Sat Flow, veh/h/ln				1707	1900	1885	1811	1856	0	0	1841	1826
Adj Flow Rate, veh/h				249	0	166	227	539	0	0	319	434
Peak Hour Factor				0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %				13	0	1	6	3	0	0	4	5
Cap, veh/h				361	0	319	447	1105	0	0	649	579
Arrive On Green				0.20	0.00	0.20	0.12	0.60	0.00	0.00	0.37	0.37
Sat Flow, veh/h				1810	0	1598	1725	1856	0	0	1841	1560
Grp Volume(v), veh/h				249	0	166	227	539	0	0	319	434
Grp Sat Flow(s), veh/h/ln				1810	0	1598	1725	1856	0	0	1749	1560
Q Serve(g_s), s				6.2	0.0	4.5	3.4	8.1	0.0	0.0	6.9	11.8
Cycle Q Clear(g_c), s				6.2	0.0	4.5	3.4	8.1	0.0	0.0	6.9	11.8
Prop In Lane				1.00		1.00	1.00	1.00	0.00	0.00	1.00	1.00
Lane Grp Cap(c), veh/h				361	0	319	447	1105	0	0	649	579
V/C Ratio(X)				0.69	0.00	0.52	0.51	0.49	0.00	0.00	0.49	0.75
Avail Cap(c_a), veh/h				741	0	654	942	1140	0	0	1074	958
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)				1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				18.1	0.0	17.5	9.0	5.6	0.0	0.0	11.8	13.4
Incr Delay (d2), s/veh				2.4	0.0	1.3	0.9	0.3	0.0	0.0	0.6	2.0
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln				2.5	0.0	0.1	0.9	1.7	0.0	0.0	2.1	3.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				20.5	0.0	18.8	9.9	6.0	0.0	0.0	12.4	15.4
LnGrp LOS				C	A	B	A	A	A	A	B	B
Approach Vol, veh/h						415		766			753	
Approach Delay, s/veh						19.8		7.1			14.1	
Approach LOS						B		A			B	
Timer - Assigned Phs		2		4		5		6				
Phs Duration (G+Y+R <sub>c</sub> ), s		34.1		14.7		11.0		23.1				
Change Period (Y+R <sub>c</sub> ), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		30.0		20.0		20.0		30.0				
Max Q Clear Time (g_c+l1), s		10.1		8.2		5.4		13.8				
Green Ext Time (p_c), s		3.2		1.5		0.5		4.3				
Intersection Summary												
HCM 6th Ctrl Delay				12.6								
HCM 6th LOS				B								

HCM 6th Signalized Intersection Summary Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse Existing \_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	311	0	204	0	0	0	0	439	193	108	449	0
Future Volume (veh/h)	311	0	204	0	0	0	0	439	193	108	449	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No		No		
Adj Sat Flow, veh/h/ln	1856	1900	1693				0	1826	1752	1885	1752	0
Adj Flow Rate, veh/h	324	0	212				0	457	201	112	468	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	3	0	14				0	5	10	1	10	0
Cap, veh/h	450	0	410				0	603	490	419	942	0
Arrive On Green	0.25	0.00	0.25				0.00	0.33	0.33	0.10	0.54	0.00
Sat Flow, veh/h	1767	0	1610				0	1826	1485	1795	1752	0
Grp Volume(v), veh/h	324	0	212				0	457	201	112	468	0
Grp Sat Flow(s), veh/h/ln	1767	0	1610				0	1826	1485	1795	1752	0
Q Serve(g_s), s	8.1	0.0	5.4				0.0	10.8	5.1	1.7	8.1	0.0
Cycle Q Clear(g_c), s	8.1	0.0	5.4				0.0	10.8	5.1	1.7	8.1	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	450	0	410				0	603	490	419	942	0
V/C Ratio(X)	0.72	0.00	0.52				0.00	0.76	0.41	0.27	0.50	0.00
Avail Cap(c_a), veh/h	1101	0	1003				0	1137	925	978	1091	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	16.4	0.0	15.4				0.0	14.4	12.5	9.0	7.0	0.0
Incr Delay (d2), s/veh	2.2	0.0	1.0				0.0	2.0	0.6	0.3	0.4	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.0	0.0	1.8				0.0	3.8	1.4	0.5	1.9	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	18.6	0.0	16.4				0.0	16.4	13.0	9.4	7.4	0.0
LnGrp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	536							658			580	
Approach Delay, s/veh	17.7							15.4			7.8	
Approach LOS	B							B			A	
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+R <sub>c</sub> ), s	10.0	20.9	17.3	30.9								
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	20.0	30.0	30.0	30.0								
Max Q Clear Time (g <sub>c+l1</sub> ), s	3.7	12.8	10.1	10.1								
Green Ext Time (p <sub>c</sub> ), s	0.2	3.1	2.2	2.7								
Intersection Summary												
HCM 6th Ctrl Delay			13.6									
HCM 6th LOS			B									

**Intersection**

Int Delay, s/veh 10.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	42	156	3	17	298	79	1	30	19	103	38	56
Future Vol, veh/h	42	156	3	17	298	79	1	30	19	103	38	56
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	3	0	6	4	4	0	0	0	1	5	5
Mvmt Flow	47	173	3	19	331	88	1	33	21	114	42	62

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	419	0	0	176	0	0	734	726	175	709	683	375
Stage 1	-	-	-	-	-	-	269	269	-	413	413	-
Stage 2	-	-	-	-	-	-	465	457	-	296	270	-
Critical Hdwy	4.12	-	-	4.16	-	-	7.1	6.5	6.2	7.91	7.35	6.65
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.91	6.35	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.91	6.35	-
Follow-up Hdwy	2.218	-	-	2.254	-	-	3.5	4	3.3	3.509	4.045	3.345
Pot Cap-1 Maneuver	1140	-	-	1376	-	-	338	354	874	299	316	638
Stage 1	-	-	-	-	-	-	741	690	-	564	537	-
Stage 2	-	-	-	-	-	-	581	571	-	669	641	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1140	-	-	1376	-	-	259	332	874	257	296	638
Mov Cap-2 Maneuver	-	-	-	-	-	-	259	332	-	257	296	-
Stage 1	-	-	-	-	-	-	707	658	-	538	527	-
Stage 2	-	-	-	-	-	-	474	561	-	591	612	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	1.7	0.3			14.6			37.7			
HCM LOS					B			E			
<hr/>											
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	431	1140	-	-	1376	-	-	319			
HCM Lane V/C Ratio	0.129	0.041	-	-	0.014	-	-	0.686			
HCM Control Delay (s)	14.6	8.3	0	-	7.7	0	-	37.7			
HCM Lane LOS	B	A	A	-	A	A	-	E			
HCM 95th %tile Q(veh)	0.4	0.1	-	-	0	-	-	4.8			

**Intersection**

Int Delay, s/veh 78.7

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		A	B		
Traffic Vol, veh/h	151	30	16	921	478	70
Future Vol, veh/h	151	30	16	921	478	70
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	6	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	3	0	6	2	5	4
Mvmt Flow	161	32	17	980	509	74

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	1560	546	583	0	-	0
Stage 1	546	-	-	-	-	-
Stage 2	1014	-	-	-	-	-
Critical Hdwy	7.63	6.8	4.16	-	-	-
Critical Hdwy Stg 1	6.63	-	-	-	-	-
Critical Hdwy Stg 2	6.63	-	-	-	-	-
Follow-up Hdwy	3.527	3.3	2.254	-	-	-
Pot Cap-1 Maneuver	~ 73	494	972	-	-	-
Stage 1	482	-	-	-	-	-
Stage 2	249	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	~ 70	494	972	-	-	-
Mov Cap-2 Maneuver	~ 70	-	-	-	-	-
Stage 1	464	-	-	-	-	-
Stage 2	249	-	-	-	-	-

**Approach** EB NB SB

HCM Control Delay, \$\\$ 724.3 0.1 0

HCM LOS F

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	972	-	82	-	-
HCM Lane V/C Ratio	0.018	-	2.348	-	-
HCM Control Delay (s)	8.8	\$\\$ 724.3	-	-	-
HCM Lane LOS	A	A	F	-	-
HCM 95th %tile Q(veh)	0.1	-	17.9	-	-

**Notes**

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	3	4	0	33	26	0
Future Vol, veh/h	3	4	0	33	26	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	4	5	0	40	31	0
Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	71	31	-	0	-	0
Stage 1	31	-	-	-	-	-
Stage 2	40	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	938	1049	0	-	-	0
Stage 1	997	-	0	-	-	0
Stage 2	988	-	0	-	-	0
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver	938	1049	-	-	-	-
Mov Cap-2 Maneuver	938	-	-	-	-	-
Stage 1	997	-	-	-	-	-
Stage 2	988	-	-	-	-	-
Approach	EB	NB	SB			
HCM Control Delay, s	8.6	0	0			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	EBLn1	SBT			
Capacity (veh/h)	-	998	-			
HCM Lane V/C Ratio	-	0.008	-			
HCM Control Delay (s)	-	8.6	-			
HCM Lane LOS	-	A	-			
HCM 95th %tile Q(veh)	-	0	-			

Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	112	182	29	21	9
Future Vol, veh/h	4	112	182	29	21	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	3	3	0	0	0
Mvmt Flow	4	124	202	32	23	10
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	234	0	-	0	350	218
Stage 1	-	-	-	-	218	-
Stage 2	-	-	-	-	132	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	1345	-	-	-	651	827
Stage 1	-	-	-	-	823	-
Stage 2	-	-	-	-	899	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1345	-	-	-	649	827
Mov Cap-2 Maneuver	-	-	-	-	649	-
Stage 1	-	-	-	-	821	-
Stage 2	-	-	-	-	899	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.3	0	10.4			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1345	-	-	-	694	
HCM Lane V/C Ratio	0.003	-	-	-	0.048	
HCM Control Delay (s)	7.7	0	-	-	10.4	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.2	

Intersection

Int Delay, s/veh 2.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	161	121	0	0	191	1	0	0	0	0	0	0
Future Vol, veh/h	161	121	0	0	191	1	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	1	2	0	0	4	0	2	2	2	2	2	2
Mvmt Flow	173	130	0	0	205	1	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	206	0	-
Stage 1	-	-	476
Stage 2	-	-	206
Critical Hdwy	4.11	-	6.52
Critical Hdwy Stg 1	-	-	5.52
Critical Hdwy Stg 2	-	-	5.52
Follow-up Hdwy	2.209	-	4.018
Pot Cap-1 Maneuver	1371	-	372
Stage 1	-	0	557
Stage 2	-	0	731
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1371	-	0
Mov Cap-2 Maneuver	-	-	0
Stage 1	-	-	0
Stage 2	-	-	0

Approach	EB	WB	NE
HCM Control Delay, s	4.6	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1371	-
HCM Lane V/C Ratio	-	0.126	-
HCM Control Delay (s)	0	8	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.4	-

**Intersection**

Int Delay, s/veh 5.6

**Movement** WBL WBR NBT NBR SBL SBT

Lane Configurations	WBL	WBR	NBT	NBR	SBL	SBT
Traffic Vol, veh/h	5	57	30	6	60	21
Future Vol, veh/h	5	57	30	6	60	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	0	4	0	0	2	0
Mvmt Flow	6	68	36	7	71	25

**Major/Minor** Minor1 Major1 Major2

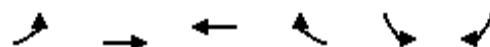
Conflicting Flow All	207	40	0	0	43	0
Stage 1	40	-	-	-	-	-
Stage 2	167	-	-	-	-	-
Critical Hdwy	6.4	6.24	-	-	4.12	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.336	-	-	2.218	-
Pot Cap-1 Maneuver	786	1026	-	-	1566	-
Stage 1	988	-	-	-	-	-
Stage 2	867	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	750	1026	-	-	1566	-
Mov Cap-2 Maneuver	750	-	-	-	-	-
Stage 1	988	-	-	-	-	-
Stage 2	827	-	-	-	-	-

**Approach** WB NB SB

HCM Control Delay, s 8.9 0 5.5

HCM LOS A

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	996	1566	-
HCM Lane V/C Ratio	-	-	0.074	0.046	-
HCM Control Delay (s)	-	-	8.9	7.4	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.2	0.1	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	45	298	211	3	12	188
Future Volume (vph)	45	298	211	3	12	188
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.87	
Flt Protected		0.99	1.00		1.00	
Satd. Flow (prot)		1851	1842		1639	
Flt Permitted		0.93	1.00		1.00	
Satd. Flow (perm)		1741	1842		1639	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	47	310	220	3	12	196
RTOR Reduction (vph)	0	0	1	0	153	0
Lane Group Flow (vph)	0	357	222	0	56	0
Heavy Vehicles (%)	2%	2%	3%	0%	0%	1%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		82.6	37.2		26.2	
Effective Green, g (s)		82.6	37.2		26.2	
Actuated g/C Ratio		0.70	0.31		0.22	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1210	576		361		
v/s Ratio Prot		c0.12		c0.03		
v/s Ratio Perm		c0.21				
v/c Ratio		0.30	0.39		0.16	
Uniform Delay, d1		6.9	31.9		37.4	
Progression Factor		0.14	1.00		1.00	
Incremental Delay, d2		0.4	1.2		0.2	
Delay (s)		1.4	33.1		37.6	
Level of Service		A	C		D	
Approach Delay (s)		1.4	33.1		37.6	
Approach LOS		A	C		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		19.9		HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio		0.30				
Actuated Cycle Length (s)		118.8		Sum of lost time (s)		15.0
Intersection Capacity Utilization		54.2%		ICU Level of Service		A
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120;296; Craigville Logistics Warehouse

Existing \_SAT Midday Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↙	↖	↗
Traffic Volume (vph)	218	407	138	261	403	125
Future Volume (vph)	218	407	138	261	403	125
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1881	1599		1831	1787	1583
Flt Permitted	1.00	1.00		0.80	0.95	1.00
Satd. Flow (perm)	1881	1599		1491	1787	1583
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	227	424	144	272	420	130
RTOR Reduction (vph)	0	291	0	0	0	23
Lane Group Flow (vph)	227	133	0	416	420	107
Heavy Vehicles (%)	1%	1%	2%	2%	1%	2%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	37.2	37.2		68.4	40.4	40.4
Effective Green, g (s)	37.2	37.2		68.4	40.4	40.4
Actuated g/C Ratio	0.31	0.31		0.58	0.34	0.34
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	589	500		858	607	538
v/s Ratio Prot	0.12			c0.24		
v/s Ratio Perm		0.08		c0.28		0.07
v/c Ratio	0.39	0.27		0.48	0.69	0.20
Uniform Delay, d1	31.9	30.6		14.8	33.8	27.7
Progression Factor	1.00	1.00		0.45	1.00	1.00
Incremental Delay, d2	1.2	0.8		1.2	6.4	0.8
Delay (s)	33.1	31.4		7.8	40.2	28.6
Level of Service	C	C		A	D	C
Approach Delay (s)	32.0			7.8	37.4	
Approach LOS	C			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		27.6		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.59				
Actuated Cycle Length (s)		118.8		Sum of lost time (s)		15.0
Intersection Capacity Utilization		67.7%		ICU Level of Service		C
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120;296; Craigville Logistics Warehouse Existing\_SAT Midday Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	122	312	208	309	331	24	251	140	257	56	171	140
Future Volume (veh/h)	122	312	208	309	331	24	251	140	257	56	171	140
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No		No		No	
Adj Sat Flow, veh/h/ln	1900	1900	1856	1870	1870	1781	1900	1870	1856	1900	1870	1900
Adj Flow Rate, veh/h	131	335	224	332	356	26	270	151	276	60	184	151
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	3	2	2	8	0	2	3	0	2	0
Cap, veh/h	169	455	298	447	436	32	254	617	519	77	221	181
Arrive On Green	0.09	0.22	0.22	0.13	0.25	0.25	0.14	0.33	0.33	0.04	0.23	0.23
Sat Flow, veh/h	1810	2090	1369	3456	1722	126	1810	1870	1572	1810	950	780
Grp Volume(v), veh/h	131	288	271	332	0	382	270	151	276	60	0	335
Grp Sat Flow(s), veh/h/ln	1810	1805	1654	1728	0	1848	1810	1870	1572	1810	0	1730
Q Serve(g_s), s	5.0	10.6	10.9	6.6	0.0	13.9	10.0	4.2	10.2	2.3	0.0	13.1
Cycle Q Clear(g_c), s	5.0	10.6	10.9	6.6	0.0	13.9	10.0	4.2	10.2	2.3	0.0	13.1
Prop In Lane	1.00		0.83	1.00		0.07	1.00		1.00	1.00		0.45
Lane Grp Cap(c), veh/h	169	393	360	447	0	468	254	617	519	77	0	402
V/C Ratio(X)	0.77	0.73	0.75	0.74	0.00	0.82	1.06	0.24	0.53	0.78	0.00	0.83
Avail Cap(c_a), veh/h	380	633	580	727	0	1036	254	655	551	254	0	606
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	31.6	26.0	26.1	29.9	0.0	25.1	30.7	17.4	19.4	33.8	0.0	26.1
Incr Delay (d2), s/veh	7.3	2.7	3.2	2.5	0.0	3.5	74.6	0.2	0.9	15.1	0.0	6.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.4	4.5	4.2	2.7	0.0	6.0	9.3	1.7	3.6	1.3	0.0	5.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	38.9	28.7	29.3	32.4	0.0	28.6	105.2	17.6	20.3	48.9	0.0	32.2
LnGrp LOS	D	C	C	C	A	C	F	B	C	D	A	C
Approach Vol, veh/h		690			714			697			395	
Approach Delay, s/veh		30.8			30.4			52.6			34.7	
Approach LOS		C			C			D			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R <sub>c</sub> ), s	11.7	23.1	8.0	28.5	14.2	20.5	15.0	21.6				
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Q Clear Time (g_c+l1), s	7.0	15.9	4.3	12.2	8.6	12.9	12.0	15.1				
Green Ext Time (p_c), s	0.2	2.2	0.0	1.4	0.6	2.6	0.0	1.4				
Intersection Summary												
HCM 6th Ctrl Delay			37.4									
HCM 6th LOS			D									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse Existing\_SAT Midday Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	108	1	133	93	515	0	0	337	351
Future Volume (veh/h)	0	0	0	108	1	133	93	515	0	0	337	351
Initial Q (Q <sub>b</sub> ), veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		No
Adj Sat Flow, veh/h/ln				1841	1900	1900	1841	1870	0	0	1856	1870
Adj Flow Rate, veh/h				117	1	145	101	560	0	0	366	382
Peak Hour Factor				0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %				4	0	0	4	2	0	0	3	2
Cap, veh/h				270	2	242	460	1102	0	0	656	585
Arrive On Green				0.15	0.15	0.15	0.09	0.59	0.00	0.00	0.37	0.37
Sat Flow, veh/h				1795	15	1610	1753	1870	0	0	1856	1572
Grp Volume(v), veh/h				118	0	145	101	560	0	0	366	382
Grp Sat Flow(s), veh/h/ln				1810	0	1610	1753	1870	0	0	1763	1572
Q Serve(g_s), s				2.3	0.0	3.2	1.1	6.7	0.0	0.0	6.3	7.7
Cycle Q Clear(g_c), s				2.3	0.0	3.2	1.1	6.7	0.0	0.0	6.3	7.7
Prop In Lane				0.99		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				272	0	242	460	1102	0	0	656	585
V/C Ratio(X)				0.43	0.00	0.60	0.22	0.51	0.00	0.00	0.56	0.65
Avail Cap(c_a), veh/h				942	0	838	1220	1461	0	0	1377	1228
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)				1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				14.8	0.0	15.2	6.3	4.6	0.0	0.0	9.6	10.0
Incr Delay (d2), s/veh				1.1	0.0	2.4	0.2	0.4	0.0	0.0	0.7	1.2
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln				0.8	0.0	0.2	0.2	0.9	0.0	0.0	1.7	1.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				15.9	0.0	17.6	6.5	5.0	0.0	0.0	10.3	11.2
LnGrp LOS				B	A	B	A	A	A	A	B	B
Approach Vol, veh/h						263			661		748	
Approach Delay, s/veh						16.9			5.2		10.8	
Approach LOS						B			A		B	
Timer - Assigned Phs	2		4	5	6							
Phs Duration (G+Y+R <sub>c</sub> ), s	27.6		10.8	8.3	19.3							
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0							
Max Green Setting (Gmax), s	30.0		20.0	20.0	30.0							
Max Q Clear Time (g_c+l1), s	8.7		5.2	3.1	9.7							
Green Ext Time (p_c), s	3.4		0.9	0.2	4.6							
Intersection Summary												
HCM 6th Ctrl Delay			9.5									
HCM 6th LOS			A									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse Existing\_SAT Midday Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	292	1	136	0	0	0	0	316	196	143	302	0
Future Volume (veh/h)	292	1	136	0	0	0	0	316	196	143	302	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No		No		
Adj Sat Flow, veh/h/ln	1870	1900	1841				0	1856	1811	1856	1856	0
Adj Flow Rate, veh/h	321	1	149				0	347	215	157	332	0
Peak Hour Factor	0.91	0.91	0.91				0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	0	4				0	3	6	3	3	0
Cap, veh/h	451	3	405				0	511	423	469	948	0
Arrive On Green	0.25	0.25	0.25				0.00	0.28	0.28	0.12	0.51	0.00
Sat Flow, veh/h	1781	11	1601				0	1856	1535	1767	1856	0
Grp Volume(v), veh/h	321	0	150				0	347	215	157	332	0
Grp Sat Flow(s), veh/h/ln	1781	0	1612				0	1856	1535	1767	1856	0
Q Serve(g_s), s	7.0	0.0	3.2				0.0	7.1	5.0	2.3	4.5	0.0
Cycle Q Clear(g_c), s	7.0	0.0	3.2				0.0	7.1	5.0	2.3	4.5	0.0
Prop In Lane	1.00		0.99				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	451	0	408				0	511	423	469	948	0
V/C Ratio(X)	0.71	0.00	0.37				0.00	0.68	0.51	0.33	0.35	0.00
Avail Cap(c_a), veh/h	1261	0	1141				0	1313	1086	1095	1313	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	14.4	0.0	13.0				0.0	13.7	12.9	8.5	6.2	0.0
Incr Delay (d2), s/veh	2.1	0.0	0.6				0.0	1.6	0.9	0.4	0.2	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.5	0.0	1.0				0.0	2.5	1.4	0.6	1.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	16.5	0.0	13.6				0.0	15.3	13.9	8.9	6.4	0.0
LnGrp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	471							562			489	
Approach Delay, s/veh	15.6							14.7			7.2	
Approach LOS	B							B			A	
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+R <sub>c</sub> ), s	10.0	16.7	15.7	26.7								
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	20.0	30.0	30.0	30.0								
Max Q Clear Time (g <sub>c+l1</sub> ), s	4.3	9.1	9.0	6.5								
Green Ext Time (p <sub>c</sub> ), s	0.3	2.6	1.8	1.8								
Intersection Summary												
HCM 6th Ctrl Delay			12.6									
HCM 6th LOS			B									

**Intersection**

Int Delay, s/veh 4.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	17	125	1	6	178	35	0	22	4	89	19	25
Future Vol, veh/h	17	125	1	6	178	35	0	22	4	89	19	25
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	2	0	17	2	0	0	0	25	1	0	4
Mvmt Flow	19	139	1	7	198	39	0	24	4	99	21	28

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	237	0	0	140	0	0	434	429	140	424	410	218
Stage 1	-	-	-	-	-	-	178	178	-	232	232	-
Stage 2	-	-	-	-	-	-	256	251	-	192	178	-
Critical Hdwy	4.1	-	-	4.27	-	-	7.1	6.5	6.45	7.91	7.3	6.64
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Follow-up Hdwy	2.2	-	-	2.353	-	-	3.5	4	3.525	3.509	4	3.336
Pot Cap-1 Maneuver	1342	-	-	1356	-	-	536	521	850	493	488	797
Stage 1	-	-	-	-	-	-	828	756	-	734	680	-
Stage 2	-	-	-	-	-	-	753	703	-	778	726	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1342	-	-	1356	-	-	492	510	850	465	478	797
Mov Cap-2 Maneuver	-	-	-	-	-	-	492	510	-	465	478	-
Stage 1	-	-	-	-	-	-	816	745	-	723	676	-
Stage 2	-	-	-	-	-	-	700	699	-	737	715	-

Approach	EB	WB			NB	SB		
HCM Control Delay, s	0.9	0.2			12	15		
HCM LOS					B	C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	543	1342	-	-	1356	-	-	507
HCM Lane V/C Ratio	0.053	0.014	-	-	0.005	-	-	0.291
HCM Control Delay (s)	12	7.7	0	-	7.7	0	-	15
HCM Lane LOS	B	A	A	-	A	A	-	C
HCM 95th %tile Q(veh)	0.2	0	-	-	0	-	-	1.2

Intersection						
Int Delay, s/veh	2.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		A	B		
Traffic Vol, veh/h	79	5	5	363	523	40
Future Vol, veh/h	79	5	5	363	523	40
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	6	-	-	0	0	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	3	0	0	1	1	3
Mvmt Flow	82	5	5	378	545	42
Major/Minor	Minor2	Major1		Major2		
Conflicting Flow All	954	566	587	0	-	0
Stage 1	566	-	-	-	-	-
Stage 2	388	-	-	-	-	-
Critical Hdwy	7.63	6.8	4.1	-	-	-
Critical Hdwy Stg 1	6.63	-	-	-	-	-
Critical Hdwy Stg 2	6.63	-	-	-	-	-
Follow-up Hdwy	3.527	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	208	480	998	-	-	-
Stage 1	469	-	-	-	-	-
Stage 2	600	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	207	480	998	-	-	-
Mov Cap-2 Maneuver	207	-	-	-	-	-
Stage 1	466	-	-	-	-	-
Stage 2	600	-	-	-	-	-
Approach	EB	NB		SB		
HCM Control Delay, s	33	0.1		0		
HCM LOS	D					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	998	-	214	-	-	
HCM Lane V/C Ratio	0.005	-	0.409	-	-	
HCM Control Delay (s)	8.6	0	33	-	-	
HCM Lane LOS	A	A	D	-	-	
HCM 95th %tile Q(veh)	0	-	1.9	-	-	

Intersection						
Int Delay, s/veh	3.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	56	7	0	61	51	0
Future Vol, veh/h	56	7	0	61	51	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	14	17	0	13	10	0
Mvmt Flow	62	8	0	68	57	0
Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	125	57	-	0	-	0
Stage 1	57	-	-	-	-	-
Stage 2	68	-	-	-	-	-
Critical Hdwy	6.54	6.37	-	-	-	-
Critical Hdwy Stg 1	5.54	-	-	-	-	-
Critical Hdwy Stg 2	5.54	-	-	-	-	-
Follow-up Hdwy	3.626	3.453	-	-	-	-
Pot Cap-1 Maneuver	842	969	0	-	-	0
Stage 1	936	-	0	-	-	0
Stage 2	925	-	0	-	-	0
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver	842	969	-	-	-	-
Mov Cap-2 Maneuver	842	-	-	-	-	-
Stage 1	936	-	-	-	-	-
Stage 2	925	-	-	-	-	-
Approach	EB	NB	SB			
HCM Control Delay, s	9.6	0	0			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	EBLn1	SBT			
Capacity (veh/h)	-	854	-			
HCM Lane V/C Ratio	-	0.082	-			
HCM Control Delay (s)	-	9.6	-			
HCM Lane LOS	-	A	-			
HCM 95th %tile Q(veh)	-	0.3	-			

Intersection						
Int Delay, s/veh	2.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	27	119	172	35	36	24
Future Vol, veh/h	27	119	172	35	36	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	6	7	17	9	12
Mvmt Flow	30	131	189	38	40	26
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	227	0	-	0	399	208
Stage 1	-	-	-	-	208	-
Stage 2	-	-	-	-	191	-
Critical Hdwy	4.1	-	-	-	6.49	6.32
Critical Hdwy Stg 1	-	-	-	-	5.49	-
Critical Hdwy Stg 2	-	-	-	-	5.49	-
Follow-up Hdwy	2.2	-	-	-	3.581	3.408
Pot Cap-1 Maneuver	1353	-	-	-	593	808
Stage 1	-	-	-	-	810	-
Stage 2	-	-	-	-	825	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1353	-	-	-	579	808
Mov Cap-2 Maneuver	-	-	-	-	579	-
Stage 1	-	-	-	-	791	-
Stage 2	-	-	-	-	825	-
Approach	EB	WB	SB			
HCM Control Delay, s	1.4	0	11.1			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1353	-	-	-	653	
HCM Lane V/C Ratio	0.022	-	-	-	0.101	
HCM Control Delay (s)	7.7	0	-	-	11.1	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0.1	-	-	-	0.3	

Intersection

Int Delay, s/veh 3.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	239	146	0	0	183	7	0	0	0	0	0	0
Future Vol, veh/h	239	146	0	0	183	7	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	9	5	0	0	8	0	2	2	2	2	2	2
Mvmt Flow	266	162	0	0	203	8	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	211	0	-
Stage 1	-	-	694
Stage 2	-	-	211
Critical Hdwy	4.19	-	6.52
Critical Hdwy Stg 1	-	-	5.52
Critical Hdwy Stg 2	-	-	5.52
Follow-up Hdwy	2.281	-	4.018
Pot Cap-1 Maneuver	1319	-	0
Stage 1	-	0	444
Stage 2	-	0	728
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1319	-	0
Mov Cap-2 Maneuver	-	-	0
Stage 1	-	-	0
Stage 2	-	-	0

Approach	EB	WB	NE
HCM Control Delay, s	5.2	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1319	-
HCM Lane V/C Ratio	-	0.201	-
HCM Control Delay (s)	0	8.4	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.8	-

**Intersection**

Int Delay, s/veh 5.6

Movement	WBL	WBR	NBT	NBR	SBL	SBT
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Lane Configurations	W	B		A		
Traffic Vol, veh/h	9	45	32	23	149	43
Future Vol, veh/h	9	45	32	23	149	43
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	25	11	19	5	8	18
Mvmt Flow	11	55	39	28	182	52

Major/Minor	Minor1	Major1	Major2
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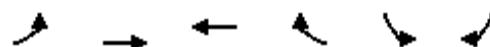
Conflicting Flow All	469	53	0	0	67	0
Stage 1	53	-	-	-	-	-
Stage 2	416	-	-	-	-	-
Critical Hdwy	6.65	6.31	-	-	4.18	-
Critical Hdwy Stg 1	5.65	-	-	-	-	-
Critical Hdwy Stg 2	5.65	-	-	-	-	-
Follow-up Hdwy	3.725	3.399	-	-	2.272	-
Pot Cap-1 Maneuver	513	990	-	-	1497	-
Stage 1	914	-	-	-	-	-
Stage 2	619	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	449	990	-	-	1497	-
Mov Cap-2 Maneuver	449	-	-	-	-	-
Stage 1	914	-	-	-	-	-
Stage 2	542	-	-	-	-	-

Approach	WB	NB	SB
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HCM Control Delay, s 9.7 0 6

HCM LOS A

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	824	1497	-
HCM Lane V/C Ratio	-	-	0.08	0.121	-
HCM Control Delay (s)	-	-	9.7	7.7	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.4	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	15	377	184	0	15	179
Future Volume (vph)	15	377	184	0	15	179
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.88	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1764	1810		1455	
Flt Permitted		0.99	1.00		1.00	
Satd. Flow (perm)		1743	1810		1455	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	17	433	211	0	17	206
RTOR Reduction (vph)	0	0	0	0	156	0
Lane Group Flow (vph)	0	450	211	0	68	0
Heavy Vehicles (%)	20%	7%	5%	0%	0%	15%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		83.3	38.0		30.3	
Effective Green, g (s)		83.3	38.0		30.3	
Actuated g/C Ratio		0.67	0.31		0.25	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1174	556		356		
v/s Ratio Prot		0.12		c0.05		
v/s Ratio Perm		c0.26				
v/c Ratio		0.38	0.38		0.19	
Uniform Delay, d1		8.9	33.6		36.9	
Progression Factor		0.13	1.00		1.00	
Incremental Delay, d2		0.5	1.2		0.3	
Delay (s)		1.7	34.8		37.2	
Level of Service		A	C		D	
Approach Delay (s)		1.7	34.8		37.2	
Approach LOS		A	C		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		18.5		HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio		0.35				
Actuated Cycle Length (s)		123.6		Sum of lost time (s)		15.0
Intersection Capacity Utilization		52.2%		ICU Level of Service		A
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120-269; Craigville Logistics Warehouse

No-Build\_AM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↖	↖	↗
Traffic Volume (vph)	216	333	159	204	385	176
Future Volume (vph)	216	333	159	204	385	176
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1759	1495		1676	1703	1509
Flt Permitted	1.00	1.00		0.74	0.95	1.00
Satd. Flow (perm)	1759	1495		1274	1703	1509
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	248	383	183	234	443	202
RTOR Reduction (vph)	0	265	0	0	0	35
Lane Group Flow (vph)	248	118	0	417	443	167
Heavy Vehicles (%)	8%	8%	16%	7%	6%	7%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	38.0	38.0		73.3	40.3	40.3
Effective Green, g (s)	38.0	38.0		73.3	40.3	40.3
Actuated g/C Ratio	0.31	0.31		0.59	0.33	0.33
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	540	459		755	555	492
v/s Ratio Prot	0.14			c0.26		
v/s Ratio Perm		0.08		c0.33		0.11
v/c Ratio	0.46	0.26		0.55	0.80	0.34
Uniform Delay, d1	34.5	32.2		15.2	37.9	31.6
Progression Factor	1.00	1.00		0.50	1.00	1.00
Incremental Delay, d2	1.7	0.8		1.8	11.4	1.9
Delay (s)	36.3	33.0		9.4	49.3	33.4
Level of Service	D	C		A	D	C
Approach Delay (s)	34.3			9.4	44.4	
Approach LOS	C			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		32.0		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.67				
Actuated Cycle Length (s)		123.6		Sum of lost time (s)		15.0
Intersection Capacity Utilization		64.7%		ICU Level of Service		C
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

No-Build\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑↓		↑↑	↑↓		↑	↑	↑	↑	↑↓	
Traffic Volume (veh/h)	64	186	137	380	194	10	102	133	317	47	216	81
Future Volume (veh/h)	64	186	137	380	194	10	102	133	317	47	216	81
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00		0.99	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1752	1811	1781	1841	1781	1574	1811	1841	1767	1752	1752	1737
Adj Flow Rate, veh/h	67	196	144	400	204	11	107	140	334	49	227	85
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	10	6	8	4	8	22	6	4	9	10	10	11
Cap, veh/h	83	347	241	545	485	26	137	516	420	59	287	107
Arrive On Green	0.05	0.18	0.18	0.16	0.29	0.29	0.08	0.28	0.28	0.04	0.24	0.24
Sat Flow, veh/h	1668	1932	1344	3401	1674	90	1725	1841	1497	1668	1215	455
Grp Volume(v), veh/h	67	173	167	400	0	215	107	140	334	49	0	312
Grp Sat Flow(s), veh/h/ln	1668	1721	1556	1700	0	1764	1725	1841	1497	1668	0	1670
Q Serve(g_s), s	2.3	5.3	5.7	6.5	0.0	5.7	3.5	3.4	12.0	1.7	0.0	10.2
Cycle Q Clear(g_c), s	2.3	5.3	5.7	6.5	0.0	5.7	3.5	3.4	12.0	1.7	0.0	10.2
Prop In Lane	1.00			1.00		0.05	1.00		1.00	1.00		0.27
Lane Grp Cap(c), veh/h	83	309	279	545	0	511	137	516	420	59	0	394
V/C Ratio(X)	0.80	0.56	0.60	0.73	0.00	0.42	0.78	0.27	0.80	0.84	0.00	0.79
Avail Cap(c_a), veh/h	431	741	670	879	0	1216	297	793	645	288	0	719
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.3	21.7	21.9	23.2	0.0	16.7	26.2	16.3	19.3	27.8	0.0	20.8
Incr Delay (d2), s/veh	16.2	1.6	2.0	1.9	0.0	0.6	9.2	0.3	3.9	25.4	0.0	3.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	1.2	2.0	2.0	2.5	0.0	2.1	1.7	1.3	4.2	1.1	0.0	4.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	43.5	23.3	23.9	25.1	0.0	17.2	35.4	16.5	23.2	53.2	0.0	24.4
LnGrp LOS	D	C	C	C	A	B	D	B	C	D	A	C
Approach Vol, veh/h		407			615			581			361	
Approach Delay, s/veh		26.9			22.4			23.9			28.3	
Approach LOS		C			C			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R <sub>c</sub> ), s	7.9	21.8	7.0	21.3	14.3	15.4	9.6	18.7				
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Q Clear Time (g_c+l1), s	4.3	7.7	3.7	14.0	8.5	7.7	5.5	12.2				
Green Ext Time (p_c), s	0.1	1.2	0.0	1.5	0.8	1.7	0.1	1.5				
Intersection Summary												
HCM 6th Ctrl Delay			24.8									
HCM 6th LOS			C									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120-269; Craigville Logistics Warehouse

No-Build\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑	↑	
Traffic Volume (veh/h)	0	0	0	158	0	59	125	493	0	0	301	432
Future Volume (veh/h)	0	0	0	158	0	59	125	493	0	0	301	432
Initial Q (Q <sub>b</sub> ), veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No		No		No		No	
Adj Sat Flow, veh/h/ln				1648	1900	1826	1663	1796	0	0	1752	1826
Adj Flow Rate, veh/h				176	0	66	139	548	0	0	334	480
Peak Hour Factor				0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %				17	0	5	16	7	0	0	10	5
Cap, veh/h				270	0	231	396	1128	0	0	705	629
Arrive On Green				0.15	0.00	0.15	0.09	0.63	0.00	0.00	0.42	0.42
Sat Flow, veh/h				1810	0	1547	1584	1796	0	0	1752	1485
Grp Volume(v), veh/h				176	0	66	139	548	0	0	334	480
Grp Sat Flow(s), veh/h/ln				1810	0	1547	1584	1796	0	0	1664	1485
Q Serve(g_s), s				4.1	0.0	1.7	1.9	7.3	0.0	0.0	6.5	12.4
Cycle Q Clear(g_c), s				4.1	0.0	1.7	1.9	7.3	0.0	0.0	6.5	12.4
Prop In Lane				1.00		1.00	1.00	1.00	0.00	0.00	1.00	
Lane Grp Cap(c), veh/h				270	0	231	396	1128	0	0	705	629
V/C Ratio(X)				0.65	0.00	0.29	0.35	0.49	0.00	0.00	0.47	0.76
Avail Cap(c_a), veh/h				805	0	689	953	1199	0	0	1111	991
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)				1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				18.0	0.0	17.0	7.7	4.5	0.0	0.0	9.3	11.0
Incr Delay (d2), s/veh				2.6	0.0	0.7	0.5	0.3	0.0	0.0	0.5	2.0
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln				1.7	0.0	0.0	0.4	1.1	0.0	0.0	1.7	3.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				20.7	0.0	17.7	8.3	4.8	0.0	0.0	9.8	13.0
LnGrp LOS				C	A	B	A	A	A	A	A	B
Approach Vol, veh/h					242			687			814	
Approach Delay, s/veh					19.8			5.5			11.7	
Approach LOS					B			A			B	
Timer - Assigned Phs	2		4	5	6							
Phs Duration (G+Y+R <sub>c</sub> ), s	33.2		11.7	9.2	24.0							
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0							
Max Green Setting (Gmax), s	30.0		20.0	20.0	30.0							
Max Q Clear Time (g <sub>c+l1</sub> ), s	9.3		6.1	3.9	14.4							
Green Ext Time (p <sub>c</sub> ), s	3.3		0.9	0.3	4.7							
Intersection Summary												
HCM 6th Ctrl Delay			10.4									
HCM 6th LOS			B									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120-269; Craigville Logistics Warehouse

No-Build\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	315	1	206	0	0	0	0	302	341	135	324	0
Future Volume (veh/h)	315	1	206	0	0	0	0	302	341	135	324	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No		No		
Adj Sat Flow, veh/h/ln	1796	1870	1722				0	1737	1722	1781	1693	0
Adj Flow Rate, veh/h	350	1	229				0	336	379	150	360	0
Peak Hour Factor	0.90	0.90	0.90				0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	7	2	12				0	11	12	8	14	0
Cap, veh/h	468	2	432				0	578	485	430	896	0
Arrive On Green	0.27	0.27	0.27				0.00	0.33	0.33	0.10	0.53	0.00
Sat Flow, veh/h	1711	7	1579				0	1737	1459	1697	1693	0
Grp Volume(v), veh/h	350	0	230				0	336	379	150	360	0
Grp Sat Flow(s), veh/h/ln	1711	0	1586				0	1737	1459	1697	1693	0
Q Serve(g_s), s	9.5	0.0	6.3				0.0	8.1	11.9	2.6	6.5	0.0
Cycle Q Clear(g_c), s	9.5	0.0	6.3				0.0	8.1	11.9	2.6	6.5	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00	1.00	0.00
Lane Grp Cap(c), veh/h	468	0	434				0	578	485	430	896	0
V/C Ratio(X)	0.75	0.00	0.53				0.00	0.58	0.78	0.35	0.40	0.00
Avail Cap(c_a), veh/h	1011	0	937				0	1027	863	931	1000	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	16.8	0.0	15.7				0.0	14.0	15.3	9.1	7.1	0.0
Incr Delay (d2), s/veh	2.4	0.0	1.0				0.0	0.9	2.8	0.5	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.5	0.0	2.0				0.0	2.7	3.5	0.7	1.6	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	19.3	0.0	16.7				0.0	14.9	18.0	9.6	7.4	0.0
LnGrp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	580							715			510	
Approach Delay, s/veh	18.2							16.6			8.1	
Approach LOS	B							B			A	
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+R <sub>c</sub> ), s	10.0	21.9	18.9	31.9								
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	20.0	30.0	30.0	30.0								
Max Q Clear Time (g <sub>c+l1</sub> ), s	4.6	13.9	11.5	8.5								
Green Ext Time (p <sub>c</sub> ), s	0.3	3.0	2.4	2.0								
Intersection Summary												
HCM 6th Ctrl Delay			14.7									
HCM 6th LOS			B									

HCM 6th Signalized Intersection Summary  
120-269; Craigville Logistics Warehouse

50: NYS Route 208 & Museum Village Rd  
No-Build\_AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	88	82	191	385	895	112
Future Volume (veh/h)	88	82	191	385	895	112
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No	No		
Adj Sat Flow, veh/h/ln	1550	1595	1598	1687	1982	1982
Adj Flow Rate, veh/h	97	90	210	423	984	123
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	22	19	20	14	5	5
Cap, veh/h	130	199	349	1384	1429	1211
Arrive On Green	0.09	0.09	0.06	0.82	0.72	0.72
Sat Flow, veh/h	1477	1352	1522	1687	1982	1680
Grp Volume(v), veh/h	97	90	210	423	984	123
Grp Sat Flow(s), veh/h/ln	1477	1352	1522	1687	1982	1680
Q Serve(g_s), s	7.0	6.7	3.6	6.6	30.2	2.4
Cycle Q Clear(g_c), s	7.0	6.7	3.6	6.6	30.2	2.4
Prop In Lane	1.00	1.00	1.00			1.00
Lane Grp Cap(c), veh/h	130	199	349	1384	1429	1211
V/C Ratio(X)	0.74	0.45	0.60	0.31	0.69	0.10
Avail Cap(c_a), veh/h	673	696	683	1384	1429	1211
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	48.8	42.8	11.7	2.4	8.5	4.6
Incr Delay (d2), s/veh	3.1	0.6	1.7	0.6	2.7	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.7	5.1	2.2	1.0	9.9	0.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	51.9	43.4	13.4	2.9	11.2	4.8
LnGrp LOS	D	D	B	A	B	A
Approach Vol, veh/h	187			633	1107	
Approach Delay, s/veh	47.8			6.4	10.5	
Approach LOS	D			A	B	
Timer - Assigned Phs	2			4	5	6
Phs Duration (G+Y+R <sub>c</sub> ), s	95.0			14.7	10.9	84.1
Change Period (Y+R <sub>c</sub> ), s	5.0			5.0	4.5	5.0
Max Green Setting (Gmax), s	90.0			50.0	30.5	55.0
Max Q Clear Time (g_c+l1), s	8.6			9.0	5.6	32.2
Green Ext Time (p_c), s	1.1			0.7	0.9	3.9
Intersection Summary						
HCM 6th Ctrl Delay				12.8		
HCM 6th LOS				B		

Intersection																			
Int Delay, s/veh	8																		
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR							
Lane Configurations																			
Traffic Vol, veh/h	31	154	1	10	193	29	0	85	9	89	23	55							
Future Vol, veh/h	31	154	1	10	193	29	0	85	9	89	23	55							
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0							
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop							
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None							
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-							
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-							
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-							
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88							
Heavy Vehicles, %	10	10	0	0	7	0	0	6	0	6	9	6							
Mvmt Flow	35	175	1	11	219	33	0	97	10	101	26	63							
Major/Minor																			
Major1		Major2			Minor1			Minor2											
Conflicting Flow All	252	0	0	176	0	0	548	520	176	557	504	236							
Stage 1	-	-	-	-	-	-	246	246	-	258	258	-							
Stage 2	-	-	-	-	-	-	302	274	-	299	246	-							
Critical Hdwy	4.2	-	-	4.1	-	-	7.1	6.56	6.2	7.96	7.39	6.66							
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.56	-	6.96	6.39	-							
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.56	-	6.96	6.39	-							
Follow-up Hdwy	2.29	-	-	2.2	-	-	3.5	4.054	3.3	3.554	4.081	3.354							
Pot Cap-1 Maneuver	1268	-	-	1412	-	-	450	455	872	384	412	773							
Stage 1	-	-	-	-	-	-	762	695	-	697	644	-							
Stage 2	-	-	-	-	-	-	712	676	-	656	653	-							
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-							
Mov Cap-1 Maneuver	1268	-	-	1412	-	-	381	437	872	306	396	773							
Mov Cap-2 Maneuver	-	-	-	-	-	-	381	437	-	306	396	-							
Stage 1	-	-	-	-	-	-	738	673	-	675	638	-							
Stage 2	-	-	-	-	-	-	622	670	-	538	633	-							
Approach																			
EB			WB			NB			SB										
HCM Control Delay, s	1.3		0.3			15.2			22										
HCM LOS	C						C												
Minor Lane/Major Mvmt																			
NBLn1		EBL	EBT	EBR	WBL	WBT	WBR	SBLn1											
Capacity (veh/h)	459	1268	-	-	1412	-	-	398											
HCM Lane V/C Ratio	0.233	0.028	-	-	0.008	-	-	0.477											
HCM Control Delay (s)	15.2	7.9	0	-	7.6	0	-	22											
HCM Lane LOS	C	A	A	-	A	A	-	C											
HCM 95th %tile Q(veh)	0.9	0.1	-	-	0	-	-	2.5											

Intersection						
Int Delay, s/veh	1.7					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	34	7	0	73	116	0
Future Vol, veh/h	34	7	0	73	116	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	0	0	4	2	0
Mvmt Flow	39	8	0	84	133	0
Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	217	133	-	0	-	0
Stage 1	133	-	-	-	-	-
Stage 2	84	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	-	-
Pot Cap-1 Maneuver	776	922	0	-	-	0
Stage 1	898	-	0	-	-	0
Stage 2	944	-	0	-	-	0
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver	776	922	-	-	-	-
Mov Cap-2 Maneuver	776	-	-	-	-	-
Stage 1	898	-	-	-	-	-
Stage 2	944	-	-	-	-	-
Approach	EB	NB	SB			
HCM Control Delay, s	9.8	0	0			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	EBLn1	SBT			
Capacity (veh/h)	-	798	-			
HCM Lane V/C Ratio	-	0.059	-			
HCM Control Delay (s)	-	9.8	-			
HCM Lane LOS	-	A	-			
HCM 95th %tile Q(veh)	-	0.2	-			

Intersection						
Int Delay, s/veh	2.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	139	299	63	39	85
Future Vol, veh/h	10	139	299	63	39	85
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	2	4	5	4	0
Mvmt Flow	11	154	332	70	43	94
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	402	0	-	0	543	367
Stage 1	-	-	-	-	367	-
Stage 2	-	-	-	-	176	-
Critical Hdwy	4.1	-	-	-	6.44	6.2
Critical Hdwy Stg 1	-	-	-	-	5.44	-
Critical Hdwy Stg 2	-	-	-	-	5.44	-
Follow-up Hdwy	2.2	-	-	-	3.536	3.3
Pot Cap-1 Maneuver	1168	-	-	-	497	683
Stage 1	-	-	-	-	696	-
Stage 2	-	-	-	-	850	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1168	-	-	-	492	683
Mov Cap-2 Maneuver	-	-	-	-	492	-
Stage 1	-	-	-	-	689	-
Stage 2	-	-	-	-	850	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.5	0	12.6			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	1168	-	-	-	609	-
HCM Lane V/C Ratio	0.01	-	-	-	0.226	-
HCM Control Delay (s)	8.1	0	-	-	12.6	-
HCM Lane LOS	A	A	-	-	B	-
HCM 95th %tile Q(veh)	0	-	-	-	0.9	-

Intersection													
Int Delay, s/veh	2.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR	
Lane Configurations													
Traffic Vol, veh/h	197	148	0	0	343	42	0	0	0	0	0	0	
Future Vol, veh/h	197	148	0	0	343	42	0	0	0	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	84	84	84	84	84	84	84	84	84	84	84	84	
Heavy Vehicles, %	3	2	0	0	4	0	2	2	2	2	2	2	
Mvmt Flow	235	176	0	0	408	50	0	0	0	0	0	0	
Major/Minor													
Major1		Major2			Minor1								
Conflicting Flow All	458	0	-	-	-	0	-	1104	-				
Stage 1	-	-	-	-	-	-	-	646	-				
Stage 2	-	-	-	-	-	-	-	458	-				
Critical Hdwy	4.13	-	-	-	-	-	-	6.52	-				
Critical Hdwy Stg 1	-	-	-	-	-	-	-	5.52	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	-	5.52	-				
Follow-up Hdwy	2.227	-	-	-	-	-	-	4.018	-				
Pot Cap-1 Maneuver	1098	-	0	0	-	-	0	211	0				
Stage 1	-	-	0	0	-	-	0	467	0				
Stage 2	-	-	0	0	-	-	0	567	0				
Platoon blocked, %	-												
Mov Cap-1 Maneuver	1098	-	-	-	-	-	-	0	-				
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	0	-				
Stage 1	-	-	-	-	-	-	-	0	-				
Stage 2	-	-	-	-	-	-	-	0	-				
Approach													
EB			WB			NE							
HCM Control Delay, s	5.2			0			0						
HCM LOS							A						
Minor Lane/Major Mvmt													
Capacity (veh/h)	-	1098	-	-	-								
HCM Lane V/C Ratio	-	0.214	-	-	-								
HCM Control Delay (s)	0	9.2	0	-	-								
HCM Lane LOS	A	A	A	-	-								
HCM 95th %tile Q(veh)	-	0.8	-	-	-								

**Intersection**

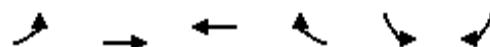
Int Delay, s/veh 7.2

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B			A	
Traffic Vol, veh/h	39	180	91	16	94	29
Future Vol, veh/h	39	180	91	16	94	29
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	3	5	25	10	8
Mvmt Flow	48	220	111	20	115	35

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	386	121	0	0	131
Stage 1	121	-	-	-	-
Stage 2	265	-	-	-	-
Critical Hdwy	6.4	6.23	-	-	4.2
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.327	-	-	2.29
Pot Cap-1 Maneuver	621	928	-	-	1406
Stage 1	909	-	-	-	-
Stage 2	784	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	569	928	-	-	1406
Mov Cap-2 Maneuver	569	-	-	-	-
Stage 1	909	-	-	-	-
Stage 2	719	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.3	0	6
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	834	1406	-
HCM Lane V/C Ratio	-	-	0.32	0.082	-
HCM Control Delay (s)	-	-	11.3	7.8	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	1.4	0.3	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	31	364	361	0	9	353
Future Volume (vph)	31	364	361	0	9	353
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.87	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1821	1810		1528	
Flt Permitted		0.95	1.00		1.00	
Satd. Flow (perm)		1736	1810		1528	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	32	371	368	0	9	360
RTOR Reduction (vph)	0	0	0	0	253	0
Lane Group Flow (vph)	0	403	368	0	116	0
Heavy Vehicles (%)	3%	4%	5%	0%	0%	8%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		85.0	40.0		40.0	
Effective Green, g (s)		85.0	40.0		40.0	
Actuated g/C Ratio		0.63	0.30		0.30	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)		1093	536		452	
v/s Ratio Prot			c0.20		c0.08	
v/s Ratio Perm		c0.23				
v/c Ratio		0.37	0.69		0.26	
Uniform Delay, d1		12.1	42.0		36.2	
Progression Factor		0.16	1.00		1.00	
Incremental Delay, d2		0.6	5.5		0.3	
Delay (s)		2.4	47.5		36.5	
Level of Service		A	D		D	
Approach Delay (s)		2.4	47.5		36.5	
Approach LOS		A	D		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		28.0		HCM 2000 Level of Service	C	
HCM 2000 Volume to Capacity ratio		0.44				
Actuated Cycle Length (s)		135.0		Sum of lost time (s)	15.0	
Intersection Capacity Utilization		74.7%		ICU Level of Service	D	
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120;296; Craigville Logistics Warehouse

No-Build\_PM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↖	↘	↗
Traffic Volume (vph)	197	471	280	434	405	198
Future Volume (vph)	197	471	280	434	405	198
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1810	1583		1759	1752	1495
Flt Permitted	1.00	1.00		0.78	0.95	1.00
Satd. Flow (perm)	1810	1583		1407	1752	1495
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	201	481	286	443	413	202
RTOR Reduction (vph)	0	338	0	0	0	39
Lane Group Flow (vph)	201	143	0	729	413	163
Heavy Vehicles (%)	5%	2%	9%	4%	3%	8%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	40.0	40.0		85.0	40.0	40.0
Effective Green, g (s)	40.0	40.0		85.0	40.0	40.0
Actuated g/C Ratio	0.30	0.30		0.63	0.30	0.30
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	536	469		885	519	442
v/s Ratio Prot	0.11			c0.24		
v/s Ratio Perm		0.09		c0.52		0.11
v/c Ratio	0.38	0.30		0.82	0.80	0.37
Uniform Delay, d1	37.6	36.7		19.2	43.7	37.5
Progression Factor	1.00	1.00		0.88	1.00	1.00
Incremental Delay, d2	1.2	1.0		6.0	12.0	2.4
Delay (s)	38.8	37.8		22.8	55.7	39.9
Level of Service	D	D		C	E	D
Approach Delay (s)	38.1			22.8	50.5	
Approach LOS	D			C	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		36.4		HCM 2000 Level of Service		D
HCM 2000 Volume to Capacity ratio		0.85				
Actuated Cycle Length (s)		135.0		Sum of lost time (s)		15.0
Intersection Capacity Utilization		83.6%		ICU Level of Service		E
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120;296; Craigville Logistics Warehouse

No-Build\_PM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	114	321	214	444	372	22	266	196	277	70	178	113
Future Volume (veh/h)	114	321	214	444	372	22	266	196	277	70	178	113
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00			1.00	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1885	1841	1826	1870	1900	1870	1885	1841	1841	1856	1900
Adj Flow Rate, veh/h	116	328	218	453	380	22	271	200	283	71	182	115
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	1	4	5	2	0	2	1	4	4	3	0
Cap, veh/h	151	443	288	557	516	30	244	558	461	91	224	142
Arrive On Green	0.08	0.21	0.21	0.17	0.29	0.29	0.14	0.30	0.30	0.05	0.21	0.21
Sat Flow, veh/h	1810	2080	1353	3374	1751	101	1781	1885	1558	1753	1062	671
Grp Volume(v), veh/h	116	281	265	453	0	402	271	200	283	71	0	297
Grp Sat Flow(s), veh/h/ln	1810	1791	1642	1687	0	1852	1781	1885	1558	1753	0	1734
Q Serve(g_s), s	4.6	10.7	11.0	9.5	0.0	14.3	10.0	6.1	11.4	2.9	0.0	11.9
Cycle Q Clear(g_c), s	4.6	10.7	11.0	9.5	0.0	14.3	10.0	6.1	11.4	2.9	0.0	11.9
Prop In Lane	1.00			0.82	1.00		0.05	1.00		1.00	1.00	0.39
Lane Grp Cap(c), veh/h	151	381	349	557	0	545	244	558	461	91	0	366
V/C Ratio(X)	0.77	0.74	0.76	0.81	0.00	0.74	1.11	0.36	0.61	0.78	0.00	0.81
Avail Cap(c_a), veh/h	372	613	562	693	0	1015	244	646	534	240	0	594
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	32.8	26.8	27.0	29.4	0.0	23.2	31.5	20.2	22.1	34.2	0.0	27.4
Incr Delay (d2), s/veh	7.9	2.8	3.4	6.0	0.0	2.0	90.4	0.4	1.6	13.3	0.0	4.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.2	4.5	4.3	4.0	0.0	5.9	10.1	2.5	4.1	1.5	0.0	5.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	40.7	29.6	30.3	35.4	0.0	25.2	121.9	20.6	23.7	47.5	0.0	31.8
LnGrp LOS	D	C	C	D	A	C	F	C	C	D	A	C
Approach Vol, veh/h		662			855			754			368	
Approach Delay, s/veh		31.9			30.6			58.2			34.8	
Approach LOS		C			C			E			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R <sub>c</sub> ), s	11.1	26.5	8.8	26.6	17.1	20.5	15.0	20.4				
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Q Clear Time (g <sub>c+l1</sub> ), s	6.6	16.3	4.9	13.4	11.5	13.0	12.0	13.9				
Green Ext Time (p <sub>c</sub> ), s	0.2	2.3	0.1	1.6	0.6	2.5	0.0	1.3				
Intersection Summary												
HCM 6th Ctrl Delay			39.4									
HCM 6th LOS			D									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse

No-Build\_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑	↑	
Traffic Volume (veh/h)	0	0	0	338	0	166	252	573	0	0	367	469
Future Volume (veh/h)	0	0	0	338	0	166	252	573	0	0	367	469
Initial Q (Q <sub>b</sub> ), veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach					No		No		No		No	
Adj Sat Flow, veh/h/ln				1707	1900	1885	1811	1856	0	0	1841	1826
Adj Flow Rate, veh/h				345	0	169	257	585	0	0	374	479
Peak Hour Factor				0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %				13	0	1	6	3	0	0	4	5
Cap, veh/h				434	0	384	404	1095	0	0	665	593
Arrive On Green				0.24	0.00	0.24	0.12	0.59	0.00	0.00	0.38	0.38
Sat Flow, veh/h				1810	0	1598	1725	1856	0	0	1841	1560
Grp Volume(v), veh/h				345	0	169	257	585	0	0	374	479
Grp Sat Flow(s), veh/h/ln				1810	0	1598	1725	1856	0	0	1749	1560
Q Serve(g_s), s				10.5	0.0	5.3	4.7	11.1	0.0	0.0	9.9	16.2
Cycle Q Clear(g_c), s				10.5	0.0	5.3	4.7	11.1	0.0	0.0	9.9	16.2
Prop In Lane				1.00		1.00	1.00	1.00	0.00	0.00	1.00	
Lane Grp Cap(c), veh/h				434	0	384	404	1095	0	0	665	593
V/C Ratio(X)				0.79	0.00	0.44	0.64	0.53	0.00	0.00	0.56	0.81
Avail Cap(c_a), veh/h				615	0	543	775	1095	0	0	892	795
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)				1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				21.0	0.0	19.0	11.6	7.2	0.0	0.0	14.4	16.3
Incr Delay (d2), s/veh				4.7	0.0	0.8	1.7	0.5	0.0	0.0	0.7	4.6
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln				4.5	0.0	0.1	1.4	3.0	0.0	0.0	3.4	5.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				25.7	0.0	19.8	13.3	7.7	0.0	0.0	15.1	20.9
LnGrp LOS				C	A	B	B	A	A	A	B	C
Approach Vol, veh/h						514			842			853
Approach Delay, s/veh						23.8			9.4			18.3
Approach LOS						C			A			B
Timer - Assigned Phs	2		4	5	6							
Phs Duration (G+Y+R <sub>c</sub> ), s	39.7		19.1	12.3	27.4							
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0							
Max Green Setting (Gmax), s	30.0		20.0	20.0	30.0							
Max Q Clear Time (g <sub>c+l1</sub> ), s	13.1		12.5	6.7	18.2							
Green Ext Time (p <sub>c</sub> ), s	3.3		1.6	0.6	4.2							
Intersection Summary												
HCM 6th Ctrl Delay				16.2								
HCM 6th LOS				B								

HCM 6th Signalized Intersection **SUMMARY** Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse

No-Build\_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	326	0	256	0	0	0	0	499	244	110	595	0
Future Volume (veh/h)	326	0	256	0	0	0	0	499	244	110	595	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No		No		
Adj Sat Flow, veh/h/ln	1856	1900	1693				0	1826	1752	1885	1752	0
Adj Flow Rate, veh/h	340	0	267				0	520	254	115	620	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	3	0	14				0	5	10	1	10	0
Cap, veh/h	464	0	423				0	656	534	379	961	0
Arrive On Green	0.26	0.00	0.26				0.00	0.36	0.36	0.09	0.55	0.00
Sat Flow, veh/h	1767	0	1610				0	1826	1485	1795	1752	0
Grp Volume(v), veh/h	340	0	267				0	520	254	115	620	0
Grp Sat Flow(s), veh/h/ln	1767	0	1610				0	1826	1485	1795	1752	0
Q Serve(g_s), s	9.3	0.0	7.8				0.0	13.5	7.0	1.8	13.1	0.0
Cycle Q Clear(g_c), s	9.3	0.0	7.8				0.0	13.5	7.0	1.8	13.1	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	464	0	423				0	656	534	379	961	0
V/C Ratio(X)	0.73	0.00	0.63				0.00	0.79	0.48	0.30	0.65	0.00
Avail Cap(c_a), veh/h	1002	0	913				0	1035	841	888	993	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	17.8	0.0	17.2				0.0	15.2	13.1	10.0	8.4	0.0
Incr Delay (d2), s/veh	2.3	0.0	1.6				0.0	2.2	0.7	0.4	1.4	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.5	0.0	2.7				0.0	4.8	2.0	0.6	3.5	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	20.1	0.0	18.8				0.0	17.4	13.8	10.5	9.7	0.0
LnGrp LOS	C	A	B				A	B	B	B	A	A
Approach Vol, veh/h	607							774			735	
Approach Delay, s/veh	19.5							16.2			9.9	
Approach LOS		B						B			A	
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+R <sub>c</sub> ), s	10.0	24.0	18.9	34.0								
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	20.0	30.0	30.0	30.0								
Max Q Clear Time (g <sub>c+l1</sub> ), s	3.8	15.5	11.3	15.1								
Green Ext Time (p <sub>c</sub> ), s	0.2	3.5	2.6	3.4								
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay			15.0									
HCM 6th LOS			B									

HCM 6th Signalized Intersection Summary  
120;296; Craigville Logistics Warehouse

50: NYS Route 208 & Museum Village Rd  
No-Build\_PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	178	225	175	1116	613	102
Future Volume (veh/h)	178	225	175	1116	613	102
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No	No		
Adj Sat Flow, veh/h/ln	1832	1876	1805	1864	1982	1997
Adj Flow Rate, veh/h	189	239	186	1187	652	109
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	3	0	6	2	5	4
Cap, veh/h	288	348	489	1412	1319	1126
Arrive On Green	0.17	0.17	0.05	0.76	0.67	0.67
Sat Flow, veh/h	1745	1590	1719	1864	1982	1693
Grp Volume(v), veh/h	189	239	186	1187	652	109
Grp Sat Flow(s), veh/h/ln	1745	1590	1719	1864	1982	1693
Q Serve(g_s), s	13.1	17.9	4.2	55.0	21.2	3.0
Cycle Q Clear(g_c), s	13.1	17.9	4.2	55.0	21.2	3.0
Prop In Lane	1.00	1.00	1.00			1.00
Lane Grp Cap(c), veh/h	288	348	489	1412	1319	1126
V/C Ratio(X)	0.66	0.69	0.38	0.84	0.49	0.10
Avail Cap(c_a), veh/h	566	601	795	1412	1319	1126
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	50.5	46.5	8.0	10.5	10.8	7.7
Incr Delay (d2), s/veh	0.9	0.9	0.5	6.2	1.3	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	5.8	15.4	1.2	17.8	8.3	1.0
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	51.5	47.4	8.5	16.7	12.1	7.9
LnGrp LOS	D	D	A	B	B	A
Approach Vol, veh/h	428			1373	761	
Approach Delay, s/veh	49.2			15.6	11.5	
Approach LOS	D			B	B	
Timer - Assigned Phs	2		4	5	6	
Phs Duration (G+Y+Rc), s	103.0		26.4	11.9	91.1	
Change Period (Y+Rc), s	5.0		5.0	5.0	5.0	
Max Green Setting (Gmax), s	98.0		42.0	30.0	63.0	
Max Q Clear Time (g_c+l1), s	57.0		19.9	6.2	23.2	
Green Ext Time (p_c), s	5.4		1.5	0.8	2.3	
Intersection Summary						
HCM 6th Ctrl Delay		20.0				
HCM 6th LOS			B			
Notes						
User approved volume balancing among the lanes for turning movement.						

**Intersection**

Int Delay, s/veh 25.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	45	176	3	17	315	81	1	56	19	105	80	57
Future Vol, veh/h	45	176	3	17	315	81	1	56	19	105	80	57
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	3	0	6	4	4	0	0	0	1	5	5
Mvmt Flow	50	196	3	19	350	90	1	62	21	117	89	63

Major/Minor	Major1	Major2		Minor1		Minor2		
Conflicting Flow All	440	0	0	199	0	0	807	776
Stage 1	-	-	-	-	-	-	298	298
Stage 2	-	-	-	-	-	-	509	478
Critical Hdwy	4.12	-	-	4.16	-	-	7.1	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5
Follow-up Hdwy	2.218	-	-	2.254	-	-	3.5	4
Pot Cap-1 Maneuver	1120	-	-	1350	-	-	302	331
Stage 1	-	-	-	-	-	-	715	671
Stage 2	-	-	-	-	-	-	550	559
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1120	-	-	1350	-	-	193	308
Mov Cap-2 Maneuver	-	-	-	-	-	-	193	308
Stage 1	-	-	-	-	-	-	679	637
Stage 2	-	-	-	-	-	-	401	548

Approach	EB	WB		NB		SB		
HCM Control Delay, s	1.7	0.3		17.9		91.3		
HCM LOS				C		F		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	363	1120	-	-	1350	-	-	273
HCM Lane V/C Ratio	0.233	0.045	-	-	0.014	-	-	0.985
HCM Control Delay (s)	17.9	8.4	0	-	7.7	0	-	91.3
HCM Lane LOS	C	A	A	-	A	A	-	F
HCM 95th %tile Q(veh)	0.9	0.1	-	-	0	-	-	9.8

Intersection									
Int Delay, s/veh	1								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	W			↑	↑				
Traffic Vol, veh/h	5	4	0	37	30	0			
Future Vol, veh/h	5	4	0	37	30	0			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	-	-	-	-	-			
Veh in Median Storage, #	0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	83	83	83	83	83	83			
Heavy Vehicles, %	0	0	0	0	0	0			
Mvmt Flow	6	5	0	45	36	0			
Major/Minor	Minor2	Major1		Major2					
Conflicting Flow All	81	36	-	0	-	0			
Stage 1	36	-	-	-	-	-			
Stage 2	45	-	-	-	-	-			
Critical Hdwy	6.4	6.2	-	-	-	-			
Critical Hdwy Stg 1	5.4	-	-	-	-	-			
Critical Hdwy Stg 2	5.4	-	-	-	-	-			
Follow-up Hdwy	3.5	3.3	-	-	-	-			
Pot Cap-1 Maneuver	926	1042	0	-	-	0			
Stage 1	992	-	0	-	-	0			
Stage 2	983	-	0	-	-	0			
Platoon blocked, %				-	-	-			
Mov Cap-1 Maneuver	926	1042	-	-	-	-			
Mov Cap-2 Maneuver	926	-	-	-	-	-			
Stage 1	992	-	-	-	-	-			
Stage 2	983	-	-	-	-	-			
Approach	EB	NB		SB					
HCM Control Delay, s	8.7	0		0					
HCM LOS	A								
Minor Lane/Major Mvmt	NBT	EBLn1	SBT						
Capacity (veh/h)	-	974	-						
HCM Lane V/C Ratio	-	0.011	-						
HCM Control Delay (s)	-	8.7	-						
HCM Lane LOS	-	A	-						
HCM 95th %tile Q(veh)	-	0	-						

Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	6	132	204	31	23	10
Future Vol, veh/h	6	132	204	31	23	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	3	3	0	0	0
Mvmt Flow	7	147	227	34	26	11
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	261	0	-	0	405	244
Stage 1	-	-	-	-	244	-
Stage 2	-	-	-	-	161	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	1315	-	-	-	606	800
Stage 1	-	-	-	-	801	-
Stage 2	-	-	-	-	873	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1315	-	-	-	602	800
Mov Cap-2 Maneuver	-	-	-	-	602	-
Stage 1	-	-	-	-	796	-
Stage 2	-	-	-	-	873	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.3	0	10.9			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1315	-	-	-	651	
HCM Lane V/C Ratio	0.005	-	-	-	0.056	
HCM Control Delay (s)	7.8	0	-	-	10.9	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.2	

Intersection

Int Delay, s/veh 2.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	169	143	0	0	214	1	0	0	0	0	0	0
Future Vol, veh/h	169	143	0	0	214	1	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	1	2	0	0	4	0	2	2	2	2	2	2
Mvmt Flow	182	154	0	0	230	1	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	231	0	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.11	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.209	-	-
Pot Cap-1 Maneuver	1343	-	-
Stage 1	-	0	0
Stage 2	-	0	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1343	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NE
HCM Control Delay, s	4.4	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1343	-
HCM Lane V/C Ratio	-	0.135	-
HCM Control Delay (s)	0	8.1	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.5	-

**Intersection**

Int Delay, s/veh 5.5

**Movement** WBL WBR NBT NBR SBL SBT

Lane Configurations	WBL	WBR	NBT	NBR	SBL	SBT
Traffic Vol, veh/h	6	58	31	6	61	22
Future Vol, veh/h	6	58	31	6	61	22
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	0	4	0	0	2	0
Mvmt Flow	7	69	37	7	73	26

**Major/Minor** Minor1 Major1 Major2

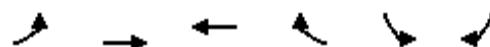
Conflicting Flow All	213	41	0	0	44	0
Stage 1	41	-	-	-	-	-
Stage 2	172	-	-	-	-	-
Critical Hdwy	6.4	6.24	-	-	4.12	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.336	-	-	2.218	-
Pot Cap-1 Maneuver	780	1024	-	-	1564	-
Stage 1	987	-	-	-	-	-
Stage 2	863	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	743	1024	-	-	1564	-
Mov Cap-2 Maneuver	743	-	-	-	-	-
Stage 1	987	-	-	-	-	-
Stage 2	822	-	-	-	-	-

**Approach** WB NB SB

HCM Control Delay, s 8.9 0 5.4

HCM LOS A

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	989	1564	-
HCM Lane V/C Ratio	-	-	0.077	0.046	-
HCM Control Delay (s)	-	-	8.9	7.4	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.2	0.1	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	46	330	234	11	12	196
Future Volume (vph)	46	330	234	11	12	196
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	0.99		0.87	
Flt Protected		0.99	1.00		1.00	
Satd. Flow (prot)		1851	1836		1639	
Flt Permitted		0.93	1.00		1.00	
Satd. Flow (perm)		1739	1836		1639	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	48	344	244	11	12	204
RTOR Reduction (vph)	0	0	1	0	157	0
Lane Group Flow (vph)	0	392	254	0	61	0
Heavy Vehicles (%)	2%	2%	3%	0%	0%	1%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		83.9	38.6		28.5	
Effective Green, g (s)		83.9	38.6		28.5	
Actuated g/C Ratio		0.69	0.32		0.23	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)		1192	579		381	
v/s Ratio Prot			c0.14		c0.04	
v/s Ratio Perm			c0.23			
v/c Ratio		0.33	0.44		0.16	
Uniform Delay, d1		7.8	33.3		37.4	
Progression Factor		0.13	1.00		1.00	
Incremental Delay, d2		0.4	1.5		0.2	
Delay (s)		1.5	34.8		37.6	
Level of Service		A	C		D	
Approach Delay (s)		1.5	34.8		37.6	
Approach LOS		A	C		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		20.4		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.33				
Actuated Cycle Length (s)		122.4		Sum of lost time (s)		15.0
Intersection Capacity Utilization		58.2%		ICU Level of Service		B
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120;296; Craigville Logistics Warehouse

No-Build\_SAT Midday Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↖	↖	↗
Traffic Volume (vph)	244	419	145	285	425	132
Future Volume (vph)	244	419	145	285	425	132
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1881	1599		1832	1787	1583
Flt Permitted	1.00	1.00		0.79	0.95	1.00
Satd. Flow (perm)	1881	1599		1477	1787	1583
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	254	436	151	297	443	138
RTOR Reduction (vph)	0	299	0	0	0	23
Lane Group Flow (vph)	254	137	0	448	443	115
Heavy Vehicles (%)	1%	1%	2%	2%	1%	2%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	38.6	38.6		72.1	40.3	40.3
Effective Green, g (s)	38.6	38.6		72.1	40.3	40.3
Actuated g/C Ratio	0.32	0.32		0.59	0.33	0.33
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	593	504		870	588	521
v/s Ratio Prot	0.14			c0.25		
v/s Ratio Perm		0.09		c0.30		0.07
v/c Ratio	0.43	0.27		0.51	0.75	0.22
Uniform Delay, d1	33.2	31.4		14.8	36.6	29.7
Progression Factor	1.00	1.00		0.45	1.00	1.00
Incremental Delay, d2	1.4	0.8		1.3	8.7	1.0
Delay (s)	34.6	32.2		7.9	45.3	30.7
Level of Service	C	C		A	D	C
Approach Delay (s)	33.1			7.9	41.8	
Approach LOS	C			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		29.5		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.63				
Actuated Cycle Length (s)		122.4		Sum of lost time (s)		15.0
Intersection Capacity Utilization		71.9%		ICU Level of Service		C
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120;296; Craigville Logistics Warehouse

No-Build\_SAT Midday Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	124	328	212	339	347	24	256	159	284	57	192	143
Future Volume (veh/h)	124	328	212	339	347	24	256	159	284	57	192	143
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1900	1900	1856	1870	1870	1781	1900	1870	1856	1900	1870	1900
Adj Flow Rate, veh/h	133	353	228	365	373	26	275	171	305	61	206	154
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	3	2	2	8	0	2	3	0	2	0
Cap, veh/h	171	466	296	473	454	32	241	623	523	79	242	181
Arrive On Green	0.09	0.22	0.22	0.14	0.26	0.26	0.13	0.33	0.33	0.04	0.24	0.24
Sat Flow, veh/h	1810	2119	1344	3456	1728	120	1810	1870	1572	1810	994	743
Grp Volume(v), veh/h	133	300	281	365	0	399	275	171	305	61	0	360
Grp Sat Flow(s), veh/h/ln	1810	1805	1658	1728	0	1849	1810	1870	1572	1810	0	1737
Q Serve(g_s), s	5.4	11.7	11.9	7.6	0.0	15.2	10.0	5.0	12.0	2.5	0.0	14.8
Cycle Q Clear(g_c), s	5.4	11.7	11.9	7.6	0.0	15.2	10.0	5.0	12.0	2.5	0.0	14.8
Prop In Lane	1.00		0.81	1.00		0.07	1.00		1.00	1.00		0.43
Lane Grp Cap(c), veh/h	171	397	365	473	0	485	241	623	523	79	0	422
V/C Ratio(X)	0.78	0.75	0.77	0.77	0.00	0.82	1.14	0.27	0.58	0.77	0.00	0.85
Avail Cap(c_a), veh/h	362	602	553	691	0	986	241	623	524	241	0	579
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	33.2	27.4	27.5	31.2	0.0	26.0	32.5	18.4	20.7	35.5	0.0	27.1
Incr Delay (d2), s/veh	7.4	2.9	3.7	3.2	0.0	3.6	101.0	0.2	1.6	14.7	0.0	8.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.6	5.0	4.7	3.2	0.0	6.6	10.9	2.0	4.4	1.4	0.0	6.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	40.6	30.3	31.2	34.5	0.0	29.6	133.5	18.6	22.4	50.2	0.0	36.0
LnGrp LOS	D	C	C	C	A	C	F	B	C	D	A	D
Approach Vol, veh/h		714			764			751			421	
Approach Delay, s/veh		32.6			31.9			62.2			38.0	
Approach LOS		C			C			E			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R <sub>c</sub> ), s	12.1	24.7	8.3	30.0	15.3	21.5	15.0	23.2				
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Q Clear Time (g_c+l1), s	7.4	17.2	4.5	14.0	9.6	13.9	12.0	16.8				
Green Ext Time (p_c), s	0.2	2.3	0.0	1.5	0.6	2.6	0.0	1.4				
Intersection Summary												
HCM 6th Ctrl Delay			41.6									
HCM 6th LOS			D									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse No-Build\_SAT Midday Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	179	1	136	125	563	0	0	380	363
Future Volume (veh/h)	0	0	0	179	1	136	125	563	0	0	380	363
Initial Q (Q <sub>b</sub> ), veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		No
Adj Sat Flow, veh/h/ln				1841	1900	1900	1841	1870	0	0	1856	1870
Adj Flow Rate, veh/h				195	1	148	136	612	0	0	413	395
Peak Hour Factor				0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %				4	0	0	4	2	0	0	3	2
Cap, veh/h				316	2	282	446	1098	0	0	659	588
Arrive On Green				0.18	0.18	0.18	0.09	0.59	0.00	0.00	0.37	0.37
Sat Flow, veh/h				1801	9	1610	1753	1870	0	0	1856	1572
Grp Volume(v), veh/h				196	0	148	136	612	0	0	413	395
Grp Sat Flow(s), veh/h/ln				1810	0	1610	1753	1870	0	0	1763	1572
Q Serve(g_s), s				4.2	0.0	3.5	1.7	8.5	0.0	0.0	8.1	8.9
Cycle Q Clear(g_c), s				4.2	0.0	3.5	1.7	8.5	0.0	0.0	8.1	8.9
Prop In Lane				0.99		1.00	1.00		0.00	0.00		1.00
Lane Grp Cap(c), veh/h				318	0	282	446	1098	0	0	659	588
V/C Ratio(X)				0.62	0.00	0.52	0.31	0.56	0.00	0.00	0.63	0.67
Avail Cap(c_a), veh/h				859	0	764	1111	1331	0	0	1255	1119
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)				1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh				16.1	0.0	15.8	7.1	5.3	0.0	0.0	10.8	11.0
Incr Delay (d2), s/veh				2.0	0.0	1.5	0.4	0.4	0.0	0.0	1.0	1.3
Initial Q Delay(d3), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln				1.6	0.0	0.1	0.4	1.5	0.0	0.0	2.3	2.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh				18.0	0.0	17.3	7.5	5.8	0.0	0.0	11.8	12.4
LnGrp LOS				B	A	B	A	A	A	A	B	B
Approach Vol, veh/h						344		748			808	
Approach Delay, s/veh						17.7		6.1			12.1	
Approach LOS						B		A			B	
Timer - Assigned Phs	2		4	5	6							
Phs Duration (G+Y+R <sub>c</sub> ), s	29.7		12.4	9.0	20.7							
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0							
Max Green Setting (Gmax), s	30.0		20.0	20.0	30.0							
Max Q Clear Time (g <sub>c+l1</sub> ), s	10.5		6.2	3.7	10.9							
Green Ext Time (p <sub>c</sub> ), s	3.7		1.3	0.3	4.9							
Intersection Summary												
HCM 6th Ctrl Delay			10.7									
HCM 6th LOS			B									

HCM 6th Signalized Intersection **SUMMARY** Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse

No-Build\_SAT Midday Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	304	1	176	0	0	0	0	384	257	146	413	0
Future Volume (veh/h)	304	1	176	0	0	0	0	384	257	146	413	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No						No			No	
Adj Sat Flow, veh/h/ln	1870	1900	1841				0	1856	1811	1856	1856	0
Adj Flow Rate, veh/h	334	1	193				0	422	282	160	454	0
Peak Hour Factor	0.91	0.91	0.91				0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	0	4				0	3	6	3	3	0
Cap, veh/h	461	2	415				0	583	482	425	979	0
Arrive On Green	0.26	0.26	0.26				0.00	0.31	0.31	0.11	0.53	0.00
Sat Flow, veh/h	1781	8	1603				0	1856	1535	1767	1856	0
Grp Volume(v), veh/h	334	0	194				0	422	282	160	454	0
Grp Sat Flow(s), veh/h/ln	1781	0	1611				0	1856	1535	1767	1856	0
Q Serve(g_s), s	8.0	0.0	4.8				0.0	9.5	7.2	2.5	7.2	0.0
Cycle Q Clear(g_c), s	8.0	0.0	4.8				0.0	9.5	7.2	2.5	7.2	0.0
Prop In Lane	1.00		0.99				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	461	0	417				0	583	482	425	979	0
V/C Ratio(X)	0.72	0.00	0.47				0.00	0.72	0.58	0.38	0.46	0.00
Avail Cap(c_a), veh/h	1141	0	1032				0	1188	983	991	1188	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	15.8	0.0	14.6				0.0	14.3	13.5	9.2	6.9	0.0
Incr Delay (d2), s/veh	2.2	0.0	0.8				0.0	1.7	1.1	0.6	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.0	0.0	1.5				0.0	3.3	2.1	0.7	1.8	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	18.0	0.0	15.4				0.0	16.0	14.6	9.7	7.3	0.0
LnGrp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	528							704			614	
Approach Delay, s/veh	17.1							15.4			7.9	
Approach LOS	B							B			A	
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+R <sub>c</sub> ), s	10.0	19.7	17.1	29.7								
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	20.0	30.0	30.0	30.0								
Max Q Clear Time (g <sub>c+l1</sub> ), s	4.5	11.5	10.0	9.2								
Green Ext Time (p <sub>c</sub> ), s	0.3	3.3	2.1	2.6								
<b>Intersection Summary</b>												
HCM 6th Ctrl Delay			13.4									
HCM 6th LOS			B									

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	101	166	227	449	600	86
Future Volume (veh/h)	101	166	227	449	600	86
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No	No		
Adj Sat Flow, veh/h/ln	1832	1876	1894	1879	2042	2012
Adj Flow Rate, veh/h	105	173	236	468	625	90
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	3	0	0	1	1	3
Cap, veh/h	218	296	579	1483	1399	1168
Arrive On Green	0.13	0.13	0.06	0.79	0.68	0.68
Sat Flow, veh/h	1745	1590	1804	1879	2042	1705
Grp Volume(v), veh/h	105	173	236	468	625	90
Grp Sat Flow(s), veh/h/ln	1745	1590	1804	1879	2042	1705
Q Serve(g_s), s	6.5	11.6	4.2	8.2	16.2	2.0
Cycle Q Clear(g_c), s	6.5	11.6	4.2	8.2	16.2	2.0
Prop In Lane	1.00	1.00	1.00		1.00	
Lane Grp Cap(c), veh/h	218	296	579	1483	1399	1168
V/C Ratio(X)	0.48	0.58	0.41	0.32	0.45	0.08
Avail Cap(c_a), veh/h	718	752	933	1483	1399	1168
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.5	43.3	5.8	3.5	8.3	6.1
Incr Delay (d2), s/veh	0.6	0.7	0.5	0.6	1.0	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.9	10.2	1.1	2.0	6.0	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	48.1	44.0	6.3	4.0	9.4	6.2
LnGrp LOS	D	D	A	A	A	A
Approach Vol, veh/h	278			704	715	
Approach Delay, s/veh	45.5			4.8	9.0	
Approach LOS	D			A	A	
Timer - Assigned Phs	2		4	5	6	
Phs Duration (G+Y+R <sub>c</sub> ), s	97.0		19.6	12.1	84.9	
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0	
Max Green Setting (Gmax), s	92.0		48.0	30.0	57.0	
Max Q Clear Time (g_c+l1), s	10.2		13.6	6.2	18.2	
Green Ext Time (p_c), s	1.3		1.0	1.0	2.1	
Intersection Summary						
HCM 6th Ctrl Delay			13.2			
HCM 6th LOS			B			

Intersection

Int Delay, s/veh 6.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	18	139	1	6	192	36	0	58	4	91	51	26
Future Vol, veh/h	18	139	1	6	192	36	0	58	4	91	51	26
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	3	0	17	2	0	0	0	25	1	0	4
Mvmt Flow	20	154	1	7	213	40	0	64	4	101	57	29

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	253	0	0	155	0	0	485	462	155	476	442	233
Stage 1	-	-	-	-	-	-	195	195	-	247	247	-
Stage 2	-	-	-	-	-	-	290	267	-	229	195	-
Critical Hdwy	4.1	-	-	4.27	-	-	7.1	6.5	6.45	7.91	7.3	6.64
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Follow-up Hdwy	2.2	-	-	2.353	-	-	3.5	4	3.525	3.509	4	3.336
Pot Cap-1 Maneuver	1324	-	-	1339	-	-	496	500	834	451	465	781
Stage 1	-	-	-	-	-	-	811	743	-	719	668	-
Stage 2	-	-	-	-	-	-	722	692	-	738	712	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1324	-	-	1339	-	-	425	489	834	396	454	781
Mov Cap-2 Maneuver	-	-	-	-	-	-	425	489	-	396	454	-
Stage 1	-	-	-	-	-	-	797	730	-	707	664	-
Stage 2	-	-	-	-	-	-	632	688	-	658	700	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	0.9	0.2			13.3			18.7			
HCM LOS					B			C			
<hr/>											
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBLn1		
Capacity (veh/h)	502	1324	-	-	1339	-	-	447			
HCM Lane V/C Ratio	0.137	0.015	-	-	0.005	-	-	0.418			
HCM Control Delay (s)	13.3	7.8	0	-	7.7	0	-	18.7			
HCM Lane LOS	B	A	A	-	A	A	-	C			
HCM 95th %tile Q(veh)	0.5	0	-	-	0	-	-	2			

**Intersection**

Int Delay, s/veh 3.4

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	79	7	0	96	71	0
Future Vol, veh/h	79	7	0	96	71	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	14	0	4	12	0
Mvmt Flow	88	8	0	107	79	0

Major/Minor	Minor2	Major1	Major2	
Conflicting Flow All	186	79	-	0
Stage 1	79	-	-	-
Stage 2	107	-	-	-
Critical Hdwy	6.44	6.34	-	-
Critical Hdwy Stg 1	5.44	-	-	-
Critical Hdwy Stg 2	5.44	-	-	-
Follow-up Hdwy	3.536	3.426	-	-
Pot Cap-1 Maneuver	799	949	0	-
Stage 1	939	-	0	0
Stage 2	912	-	0	0
Platoon blocked, %		-	-	
Mov Cap-1 Maneuver	799	949	-	-
Mov Cap-2 Maneuver	799	-	-	-
Stage 1	939	-	-	-
Stage 2	912	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT
Capacity (veh/h)	-	809	-
HCM Lane V/C Ratio	-	0.118	-
HCM Control Delay (s)	-	10	-
HCM Lane LOS	-	B	-
HCM 95th %tile Q(veh)	-	0.4	-

Intersection						
Int Delay, s/veh	2.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	48	119	172	48	44	34
Future Vol, veh/h	48	119	172	48	44	34
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	3	5	7	5	5	21
Mvmt Flow	53	131	189	53	48	37
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	242	0	-	0	453	216
Stage 1	-	-	-	-	216	-
Stage 2	-	-	-	-	237	-
Critical Hdwy	4.13	-	-	-	6.45	6.41
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	2.227	-	-	-	3.545	3.489
Pot Cap-1 Maneuver	1319	-	-	-	559	778
Stage 1	-	-	-	-	813	-
Stage 2	-	-	-	-	795	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1319	-	-	-	535	778
Mov Cap-2 Maneuver	-	-	-	-	535	-
Stage 1	-	-	-	-	778	-
Stage 2	-	-	-	-	795	-
Approach	EB	WB	SB			
HCM Control Delay, s	2.3	0	11.7			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1319	-	-	-	619	
HCM Lane V/C Ratio	0.04	-	-	-	0.138	
HCM Control Delay (s)	7.8	0	-	-	11.7	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0.1	-	-	-	0.5	

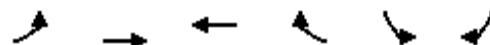
Intersection

Int Delay, s/veh 3.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	239	167	0	0	190	10	0	0	0	0	0	0
Future Vol, veh/h	239	167	0	0	190	10	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	8	4	0	0	9	36	2	2	2	2	2	2
Mvmt Flow	266	186	0	0	211	11	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	222	0	- - - 0 - 940 -
Stage 1	-	-	- - - 718 -
Stage 2	-	-	- - - 222 -
Critical Hdwy	4.18	- - - - -	- 6.52 -
Critical Hdwy Stg 1	-	- - - - -	- 5.52 -
Critical Hdwy Stg 2	-	- - - - -	- 5.52 -
Follow-up Hdwy	2.272	- - - - -	- 4.018 -
Pot Cap-1 Maneuver	1312	- 0 0 - - 0 264 0	
Stage 1	-	- 0 0 - - 0 433 0	
Stage 2	-	- 0 0 - - 0 720 0	
Platoon blocked, %	-	- - -	
Mov Cap-1 Maneuver	1312	- - - - -	- 0 -
Mov Cap-2 Maneuver	-	- - - - -	- 0 -
Stage 1	-	- - - - -	- 0 -
Stage 2	-	- - - - -	- 0 -

Approach	EB	WB	NE
HCM Control Delay, s	5	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1312	- - -
HCM Lane V/C Ratio	-	0.202	- - -
HCM Control Delay (s)	0	8.4	0 - -
HCM Lane LOS	A	A	A - -
HCM 95th %tile Q(veh)	-	0.8	- - -



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	15	398	191	0	15	179
Future Volume (vph)	15	398	191	0	15	179
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.88	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1781	1776		1504	
Flt Permitted		0.99	1.00		1.00	
Satd. Flow (perm)		1760	1776		1504	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	17	457	220	0	17	206
RTOR Reduction (vph)	0	0	0	0	155	0
Lane Group Flow (vph)	0	474	220	0	68	0
Heavy Vehicles (%)	20%	6%	7%	0%	0%	11%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		84.7	39.5		31.5	
Effective Green, g (s)		84.7	39.5		31.5	
Actuated g/C Ratio		0.67	0.31		0.25	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1181	555		375		
v/s Ratio Prot			0.12	c0.05		
v/s Ratio Perm		c0.27				
v/c Ratio		0.40	0.40		0.18	
Uniform Delay, d1		9.3	34.0		37.2	
Progression Factor		0.10	1.00		1.00	
Incremental Delay, d2		0.6	1.3		0.2	
Delay (s)		1.5	35.3		37.5	
Level of Service		A	D		D	
Approach Delay (s)		1.5	35.3		37.5	
Approach LOS		A	D		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		18.4		HCM 2000 Level of Service	B	
HCM 2000 Volume to Capacity ratio		0.36				
Actuated Cycle Length (s)		126.2		Sum of lost time (s)	15.0	
Intersection Capacity Utilization		53.3%		ICU Level of Service	A	
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120-269; Craigville Logistics Warehouse

Build\_AM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↑		↑	↑	↑
Traffic Volume (vph)	273	333	160	210	380	179
Future Volume (vph)	273	333	160	210	380	179
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1776	1524		1702	1719	1524
Flt Permitted	1.00	1.00		0.72	0.95	1.00
Satd. Flow (perm)	1776	1524		1248	1719	1524
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	314	383	184	241	437	206
RTOR Reduction (vph)	0	263	0	0	0	37
Lane Group Flow (vph)	314	120	0	425	437	169
Heavy Vehicles (%)	7%	6%	11%	8%	5%	6%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	39.5	39.5		76.0	40.2	40.2
Effective Green, g (s)	39.5	39.5		76.0	40.2	40.2
Actuated g/C Ratio	0.31	0.31		0.60	0.32	0.32
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	555	477		751	547	485
v/s Ratio Prot	0.18			c0.25		
v/s Ratio Perm		0.08		c0.34		0.11
v/c Ratio	0.57	0.25		0.57	0.80	0.35
Uniform Delay, d1	36.2	32.3		15.1	39.3	33.0
Progression Factor	1.00	1.00		0.50	1.00	1.00
Incremental Delay, d2	2.7	0.8		1.9	11.6	2.0
Delay (s)	38.9	33.1		9.5	50.9	34.9
Level of Service	D	C		A	D	C
Approach Delay (s)	35.7			9.5	45.8	
Approach LOS	D			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		33.1		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.67				
Actuated Cycle Length (s)		126.2		Sum of lost time (s)		15.0
Intersection Capacity Utilization		67.8%		ICU Level of Service		C
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

Build\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	64	192	137	384	196	10	102	133	329	47	216	81
Future Volume (veh/h)	64	192	137	384	196	10	102	133	329	47	216	81
Number	1	6	16	5	2	12	7	4	14	3	8	18
Initial Q, veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00			0.99	1.00		0.99	1.00		1.00	1.00	1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No				No			No		No		No
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1767	1841	1781	1826	1796	1604	1811	1856	1811	1767	1767	1737
Adj Flow Rate, veh/h	67	202	144	404	206	11	107	140	346	49	227	85
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	9	4	8	5	7	20	6	3	6	9	9	11
Opposing Right Turn Influence	Yes			Yes			Yes			Yes		
Cap, veh/h	84	357	242	547	493	26	137	518	428	59	287	108
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.05	0.18	0.18	0.16	0.29	0.29	0.08	0.28	0.28	0.03	0.23	0.23
Unsig. Movement Delay												
Ln Grp Delay, s/veh	43.2	23.3	23.9	25.2	0.0	17.1	35.5	16.6	23.8	52.7	0.0	24.5
Ln Grp LOS	D	C	C	C	A	B	D	B	C	D	A	C
Approach Vol, veh/h	413				621			593			361	
Approach Delay, s/veh	26.8				22.3			24.2			28.3	
Approach LOS	C				C			C			C	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Case No	2.0	4.0	2.0	3.0	2.0	4.0	2.0	4.0				
Phs Duration (G+Y+Rc), s	7.9	22.0	7.0	21.2	14.4	15.4	9.6	18.6				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Allow Headway (MAH), s	3.8	5.0	3.8	4.3	3.7	5.2	3.7	5.4				
Max Q Clear (g_c+l1), s	4.3	7.7	3.7	14.2	8.6	7.7	5.5	12.1				
Green Ext Time (g_e), s	0.1	1.2	0.0	1.5	0.8	1.7	0.1	1.5				
Prob of Phs Call (p_c)	0.66	1.00	0.55	1.00	1.00	1.00	0.82	1.00				
Prob of Max Out (p_x)	0.00	0.00	0.04	0.09	0.22	0.01	0.47	0.07				
Left-Turn Movement Data												
Assigned Mvmt	1		3		5		7					
Mvmt Sat Flow, veh/h	1682		1682		3374		1725					
Through Movement Data												
Assigned Mvmt	2		4		6		8					
Mvmt Sat Flow, veh/h	1689		1856		1988		1225					
Right-Turn Movement Data												
Assigned Mvmt	12		14		16		18					
Mvmt Sat Flow, veh/h	90		1535		1346		459					
Left Lane Group Data												
Assigned Mvmt	1	0	3	0	5	0	7	0				
Lane Assignment	L (Prot)		L (Prot)		L (Prot)		L (Prot)					

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

Build\_AM Peak Hour

	1	0	1	0	2	0	1	0
Grp Vol (v), veh/h	67	0	49	0	404	0	107	0
Grp Sat Flow (s), veh/h/ln	1682	0	1682	0	1687	0	1725	0
Q Serve Time (g_s), s	2.3	0.0	1.7	0.0	6.6	0.0	3.5	0.0
Cycle Q Clear Time (g_c), s	2.3	0.0	1.7	0.0	6.6	0.0	3.5	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Lane Grp Cap (c), veh/h	84	0	59	0	547	0	137	0
V/C Ratio (X)	0.80	0.00	0.83	0.00	0.74	0.00	0.78	0.00
Avail Cap (c_a), veh/h	434	0	289	0	870	0	297	0
Upstream Filter (l)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	27.3	0.0	27.9	0.0	23.2	0.0	26.3	0.0
Incr Delay (d2), s/veh	15.9	0.0	24.8	0.0	2.0	0.0	9.2	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	43.2	0.0	52.7	0.0	25.2	0.0	35.5	0.0
1st-Term Q (Q1), veh/ln	0.8	0.0	0.6	0.0	2.3	0.0	1.3	0.0
2nd-Term Q (Q2), veh/ln	0.4	0.0	0.4	0.0	0.2	0.0	0.4	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	1.2	0.0	1.0	0.0	2.5	0.0	1.7	0.0
%ile Storage Ratio (RQ%)	0.20	0.00	0.09	0.00	0.32	0.00	0.20	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	8
Lane Assignment				T		T		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	140	0	176	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1856	0	1749	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	3.4	0.0	5.3	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	3.4	0.0	5.3	0.0	0.0
Lane Grp Cap (c), veh/h	0	0	0	518	0	314	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.27	0.00	0.56	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	798	0	752	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	16.3	0.0	21.8	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.3	0.0	1.6	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	16.6	0.0	23.3	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	1.3	0.0	1.9	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

Build\_AM Peak Hour

	3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	1.3	0.0	2.1	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.06	0.00	0.09	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	18
Lane Assignment		T+R		R		T+R		T+R
Lanes in Grp	0	1	0	1	0	1	0	1
Grp Vol (v), veh/h	0	217	0	346	0	170	0	312
Grp Sat Flow (s), veh/h/ln	0	1779	0	1535	0	1585	0	1684
Q Serve Time (g_s), s	0.0	5.7	0.0	12.2	0.0	5.7	0.0	10.1
Cycle Q Clear Time (g_c), s	0.0	5.7	0.0	12.2	0.0	5.7	0.0	10.1
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.05	0.00	1.00	0.00	0.85	0.00	0.27
Lane Grp Cap (c), veh/h	0	520	0	428	0	285	0	395
V/C Ratio (X)	0.00	0.42	0.00	0.81	0.00	0.60	0.00	0.79
Avail Cap (c_a), veh/h	0	1224	0	660	0	682	0	724
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d1), s/veh	0.0	16.6	0.0	19.5	0.0	21.9	0.0	20.9
Incr Delay (d2), s/veh	0.0	0.5	0.0	4.3	0.0	2.0	0.0	3.6
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	17.1	0.0	23.8	0.0	23.9	0.0	24.5
1st-Term Q (Q1), veh/ln	0.0	2.0	0.0	3.9	0.0	1.9	0.0	3.6
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.5	0.0	0.2	0.0	0.4
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00
%ile Back of Q (50%), veh/ln	0.0	2.1	0.0	4.4	0.0	2.0	0.0	4.0
%ile Storage Ratio (RQ%)	0.00	0.17	0.00	0.22	0.00	0.09	0.00	0.35
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay			24.9					
HCM 6th LOS			C					

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
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Build\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑	↑	↑
Traffic Volume (veh/h)	0	0	0	158	0	59	125	505	0	0	301	436
Future Volume (veh/h)	0	0	0	158	0	59	125	505	0	0	301	436
Number				7	4	14	5	2	12	1	6	16
Initial Q, veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)				1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		No
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln				1693	1900	1826	1752	1826	0	0	1767	1826
Adj Flow Rate, veh/h				176	0	66	139	561	0	0	334	484
Peak Hour Factor				0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %				14	0	5	10	5	0	0	9	5
Opposing Right Turn Influence				Yes			Yes			No		
Cap, veh/h				270	0	231	407	1145	0	0	712	636
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green				0.15	0.00	0.15	0.09	0.63	0.00	0.00	0.42	0.42
Unsig. Movement Delay												
Ln Grp Delay, s/veh				20.6	0.0	17.6	8.2	4.8	0.0	0.0	9.7	12.9
Ln Grp LOS				C	A	B	A	A	A	A	A	B
Approach Vol, veh/h				242				700			818	
Approach Delay, s/veh				19.8				5.5			11.6	
Approach LOS					B			A			B	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs				2		4	5	6				
Case No				4.0		11.0	1.2	8.0				
Phs Duration (G+Y+Rc), s				33.1		11.7	9.1	24.0				
Change Period (Y+Rc), s				5.0		5.0	5.0	5.0				
Max Green (Gmax), s				30.0		20.0	20.0	30.0				
Max Allow Headway (MAH), s				5.0		4.9	3.8	5.2				
Max Q Clear (g_c+l1), s				9.4		6.1	3.8	14.3				
Green Ext Time (g_e), s				3.4		0.9	0.3	4.7				
Prob of Phs Call (p_c)				1.00		0.95	1.00	1.00				
Prob of Max Out (p_x)				0.03		0.01	0.00	0.22				
Left-Turn Movement Data												
Assigned Mvmt					7	5	1					
Mvmt Sat Flow, veh/h					1810	1668	0					
Through Movement Data												
Assigned Mvmt				2		4		6				
Mvmt Sat Flow, veh/h				1826		0		1767				
Right-Turn Movement Data												
Assigned Mvmt				12		14		16				
Mvmt Sat Flow, veh/h				0		1547		1497				
Left Lane Group Data												
Assigned Mvmt	0	0	0	7	5	1	0	0				
Lane Assignment							L+TL (Pr/Pm)					

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
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	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	176	139	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1810	1668	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	4.1	1.8	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	4.1	1.8	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	626	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	21.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	6.7	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	4.1	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	19.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	270	407	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.65	0.34	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	808	999	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	17.9	7.7	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	2.6	0.5	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	20.6	8.2	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	1.5	0.3	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.2	0.1	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	1.7	0.4	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.11	0.06	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	561	0	0	0	334	0	0
Grp Sat Flow (s), veh/h/ln	0	1826	0	0	0	1678	0	0
Q Serve Time (g_s), s	0.0	7.4	0.0	0.0	0.0	6.4	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	7.4	0.0	0.0	0.0	6.4	0.0	0.0
Lane Grp Cap (c), veh/h	0	1145	0	0	0	712	0	0
V/C Ratio (X)	0.00	0.49	0.00	0.00	0.00	0.47	0.00	0.00
Avail Cap (c_a), veh/h	0	1223	0	0	0	1124	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	4.5	0.0	0.0	0.0	9.3	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.3	0.0	0.0	0.0	0.5	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	4.8	0.0	0.0	0.0	9.7	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	1.0	0.0	0.0	0.0	1.6	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.0	0.0	0.1	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
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	3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	1.1	0.0	0.0	0.0	1.7	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.10	0.00	0.00	0.00	0.09	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment				R		T+R		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	66	0	484	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1547	0	1497	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	1.7	0.0	12.3	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	1.7	0.0	12.3	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	231	0	636	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.29	0.00	0.76	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	691	0	1003	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	16.9	0.0	11.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.7	0.0	1.9	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	17.6	0.0	12.9	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	1.5	0.0	2.7	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.3	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	1.6	0.0	3.1	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.11	0.00	0.16	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay				10.3				
HCM 6th LOS				B				

HCM 6th Signalized Intersection Cap\_NY-17 SB On-Ramp & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120-269; Craigville Logistics Warehouse

Build\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	323	1	206	0	0	0	0	306	341	135	324	0
Future Volume (veh/h)	323	1	206	0	0	0	0	306	341	135	324	0
Number	7	4	14				5	2	12	1	6	16
Initial Q, veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00		1.00				1.00		1.00	1.00	1.00	1.00
Parking Bus Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No						No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1811	1870	1737				0	1796	1767	1796	1722	0
Adj Flow Rate, veh/h	359	1	229				0	340	379	150	360	0
Peak Hour Factor	0.90	0.90	0.90				0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	6	2	11				0	7	9	7	12	0
Opposing Right Turn Influence	Yes						No			Yes		
Cap, veh/h	478	2	438				0	588	490	431	904	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.28	0.28	0.28				0.00	0.33	0.33	0.10	0.53	0.00
Unsig. Movement Delay												
Ln Grp Delay, s/veh	19.1	0.0	16.4				0.0	15.0	18.0	9.6	7.5	0.0
Ln Grp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	589							719			510	
Approach Delay, s/veh	18.0							16.6			8.1	
Approach LOS	B							B			A	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2		4		6						
Case No	1.2	7.0		10.0		4.0						
Phs Duration (G+Y+Rc), s	10.0	21.6		19.0		31.6						
Change Period (Y+Rc), s	5.0	5.0		5.0		5.0						
Max Green (Gmax), s	20.0	30.0		30.0		30.0						
Max Allow Headway (MAH), s	3.7	4.5		4.4		5.0						
Max Q Clear (g_c+l1), s	4.6	13.5		11.6		8.4						
Green Ext Time (g_e), s	0.3	3.0		2.4		2.0						
Prob of Phs Call (p_c)	1.00	1.00		1.00		1.00						
Prob of Max Out (p_x)	0.00	0.06		0.02		0.00						
Left-Turn Movement Data												
Assigned Mvmt	1	5		7								
Mvmt Sat Flow, veh/h	1711	0		1725								
Through Movement Data												
Assigned Mvmt	2		4		6							
Mvmt Sat Flow, veh/h	1796		7		1722							
Right-Turn Movement Data												
Assigned Mvmt	12		14		16							
Mvmt Sat Flow, veh/h	1497		1579		0							
Left Lane Group Data												
Assigned Mvmt	1	5	0	7	0	0	0	0				
Lane Assignment	L (Pr/Pm)			L								

HCM 6th Signalized Intersection Capital Rythm & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120-269; Craigville Logistics Warehouse Build\_AM Peak Hour

Middle Lane Group Data

Traffic Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T			T			
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	340	0	0	0	360	0	0
Grp Sat Flow (s), veh/h/ln	0	1796	0	0	0	1722	0	0
Q Serve Time (g_s), s	0.0	7.9	0.0	0.0	0.0	6.4	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	7.9	0.0	0.0	0.0	6.4	0.0	0.0
Lane Grp Cap (c), veh/h	0	588	0	0	0	904	0	0
V/C Ratio (X)	0.00	0.58	0.00	0.00	0.00	0.40	0.00	0.00
Avail Cap (c_a), veh/h	0	1065	0	0	0	1021	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	14.1	0.0	0.0	0.0	7.2	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.9	0.0	0.0	0.0	0.3	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	15.0	0.0	0.0	0.0	7.5	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	2.6	0.0	0.0	0.0	1.5	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.0	0.0	0.1	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120-269; Craigville Logistics Warehouse

	NY-17 SB On-Ramp	NY-17 SB Off-Ramp	NY-17 SB On-Ramp	NY-17 SB Off-Ramp	NY-17 SB On-Ramp	NY-17 SB Off-Ramp	NY-17 SB On-Ramp	Build_AM Peak Hour
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	2.7	0.0	0.0	0.0	1.6	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.10	0.00	0.00	0.00	0.14	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment		R		T+R				
Lanes in Grp	0	1	0	1	0	0	0	0
Grp Vol (v), veh/h	0	379	0	230	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1497	0	1586	0	0	0	0
Q Serve Time (g_s), s	0.0	11.5	0.0	6.2	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	11.5	0.0	6.2	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	490	0	440	0	0	0	0
V/C Ratio (X)	0.00	0.77	0.00	0.52	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	888	0	941	0	0	0	0
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	15.3	0.0	15.5	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	2.6	0.0	1.0	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	18.0	0.0	16.4	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	3.1	0.0	1.9	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.4	0.0	0.1	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	3.5	0.0	2.0	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.13	0.00	0.22	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay			14.7					
HCM 6th LOS			B					

**Intersection**

Int Delay, s/veh 5.6

Movement	WBL	WBR	NBT	NBR	SBL	SBT
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Lane Configurations						
Traffic Vol, veh/h	15	45	33	25	149	50
Future Vol, veh/h	15	45	33	25	149	50
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	3	11	17	4	7	9
Mvmt Flow	18	55	40	30	182	61

Major/Minor	Minor1	Major1	Major2
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Conflicting Flow All	480	55	0	0	70	0
Stage 1	55	-	-	-	-	-
Stage 2	425	-	-	-	-	-
Critical Hdwy	6.43	6.31	-	-	4.17	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.399	-	-	2.263	-
Pot Cap-1 Maneuver	543	987	-	-	1499	-
Stage 1	965	-	-	-	-	-
Stage 2	657	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	475	987	-	-	1499	-
Mov Cap-2 Maneuver	475	-	-	-	-	-
Stage 1	965	-	-	-	-	-
Stage 2	574	-	-	-	-	-

Approach	WB	NB	SB
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HCM Control Delay, s 10.1 0 5.8

HCM LOS B

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	777	1499	-
HCM Lane V/C Ratio	-	-	0.094	0.121	-
HCM Control Delay (s)	-	-	10.1	7.7	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.4	-

Intersection												
Int Delay, s/veh	8.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	+	+	+	+	+	+	+	+	+	+	+	+
Traffic Vol, veh/h	36	156	1	10	203	29	0	85	9	89	23	55
Future Vol, veh/h	36	156	1	10	203	29	0	85	9	89	23	55
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	7	8	0	0	6	0	0	4	0	6	4	5
Mvmt Flow	41	177	1	11	231	33	0	97	10	101	26	63
Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	264	0	0	178	0	0	574	546	178	583	530	248
Stage 1	-	-	-	-	-	-	260	260	-	270	270	-
Stage 2	-	-	-	-	-	-	314	286	-	313	260	-
Critical Hdwy	4.17	-	-	4.1	-	-	7.1	6.54	6.2	7.96	7.34	6.65
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.54	-	6.96	6.34	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.54	-	6.96	6.34	-
Follow-up Hdwy	2.263	-	-	2.2	-	-	3.5	4.036	3.3	3.554	4.036	3.345
Pot Cap-1 Maneuver	1272	-	-	1410	-	-	433	442	870	367	402	762
Stage 1	-	-	-	-	-	-	749	689	-	685	643	-
Stage 2	-	-	-	-	-	-	701	671	-	643	651	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1272	-	-	1410	-	-	364	422	870	289	384	762
Mov Cap-2 Maneuver	-	-	-	-	-	-	364	422	-	289	384	-
Stage 1	-	-	-	-	-	-	722	664	-	660	637	-
Stage 2	-	-	-	-	-	-	612	665	-	523	628	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	1.5		0.3		15.7		23.6					
HCM LOS					C		C					
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	444	1272	-	-	1410	-	-	380				
HCM Lane V/C Ratio	0.241	0.032	-	-	0.008	-	-	0.499				
HCM Control Delay (s)	15.7	7.9	0	-	7.6	0	-	23.6				
HCM Lane LOS	C	A	A	-	A	A	-	C				
HCM 95th %tile Q(veh)	0.9	0.1	-	-	0	-	-	2.7				

HCM 6th Signalized Intersection Capacity Analysis 50: NYS Route 208 & Museum Village Rd  
120-269; Craigville Logistics Warehouse Build\_AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗		
Traffic Volume (veh/h)	90	82	191	385	895	118		
Future Volume (veh/h)	90	82	191	385	895	118		
Number	7	14	5	2	6	16		
Initial Q, veh	0	0	0	0	0	0		
Ped-Bike Adj (A_pbT)	1.00	1.00	1.00		1.00			
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00		
Work Zone On Approach	No			No	No			
Lanes Open During Work Zone								
Adj Sat Flow, veh/h/ln	1610	1802	1862	1699	1997	2012		
Adj Flow Rate, veh/h	99	90	210	423	984	130		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91		
Percent Heavy Veh, %	18	5	1	12	4	3		
Opposing Right Turn Influence	Yes		Yes					
Cap, veh/h	134	214	393	1396	1452	1240		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00		
Prop Arrive On Green	0.09	0.09	0.05	0.82	0.73	0.73		
Unsig. Movement Delay								
Ln Grp Delay, s/veh	51.8	43.5	11.2	2.9	10.6	4.6		
Ln Grp LOS	D	D	B	A	B	A		
Approach Vol, veh/h	189			633	1114			
Approach Delay, s/veh	47.9			5.6	9.9			
Approach LOS	D			A	A			
Timer:	1	2	3	4	5	6	7	8
Assigned Phs		2		4	5	6		
Case No		4.0		9.0	1.2	7.0		
Phs Duration (G+Y+Rc), s	95.0			14.5	10.3	84.7		
Change Period (Y+Rc), s	5.0			5.0	4.5	5.0		
Max Green (Gmax), s	90.0			50.0	30.5	55.0		
Max Allow Headway (MAH), s	3.5			4.2	4.6	3.5		
Max Q Clear (g_c+l1), s	8.5			8.9	5.0	31.0		
Green Ext Time (g_e), s	1.1			0.7	0.9	3.8		
Prob of Phs Call (p_c)	1.00			1.00	1.00	1.00		
Prob of Max Out (p_x)	0.00			0.00	0.00	0.00		
Left-Turn Movement Data								
Assigned Mvmt				7	5	1		
Mvmt Sat Flow, veh/h				1533	1773	0		
Through Movement Data								
Assigned Mvmt		2		4		6		
Mvmt Sat Flow, veh/h		1699		0		1997		
Right-Turn Movement Data								
Assigned Mvmt		12		14		16		
Mvmt Sat Flow, veh/h		0		1527		1705		
Left Lane Group Data								
Assigned Mvmt	0	0	0	7	5	1	0	0
Lane Assignment				LL (Pr/Pm)				

HCM 6th Signalized Intersection Capacity Analysis 50: NYS Route 208 & Museum Village Rd  
 120-269; Craigville Logistics Warehouse Build\_AM Peak Hour

Lanes in Grp	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	99	210	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1533	1773	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	6.9	3.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	6.9	3.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	1533	503	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	81.7	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	50.6	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	22.2	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	79.7	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	134	393	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.74	0.53	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	700	792	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	48.8	10.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	3.0	1.1	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	51.8	11.2	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	2.6	1.7	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.1	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	2.7	1.8	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.52	0.30	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	423	0	0	0	984	0	0
Grp Sat Flow (s), veh/h/ln	0	1699	0	0	0	1997	0	0
Q Serve Time (g_s), s	0.0	6.5	0.0	0.0	0.0	29.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	6.5	0.0	0.0	0.0	29.0	0.0	0.0
Lane Grp Cap (c), veh/h	0	1396	0	0	0	1452	0	0
V/C Ratio (X)	0.00	0.30	0.00	0.00	0.00	0.68	0.00	0.00
Avail Cap (c_a), veh/h	0	1396	0	0	0	1452	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	2.3	0.0	0.0	0.0	8.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.6	0.0	0.0	0.0	2.6	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	2.9	0.0	0.0	0.0	10.6	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.8	0.0	0.0	0.0	8.3	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.2	0.0	0.0	0.0	1.0	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis 50: NYS Route 208 & Museum Village Rd  
 120-269; Craigville Logistics Warehouse

Build\_AM Peak Hour

3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	1.0	0.0	0.0	0.0	9.3	0.0
%ile Storage Ratio (RQ%)	0.00	0.13	0.00	0.00	0.00	1.01	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>							
Assigned Mvmt	0	12	0	14	0	16	0
Lane Assignment				R		R	
Lanes in Grp	0	0	0	1	0	1	0
Grp Vol (v), veh/h	0	0	0	90	0	130	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1527	0	1705	0
Q Serve Time (g_s), s	0.0	0.0	0.0	5.9	0.0	2.5	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	5.9	0.0	2.5	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	1527.4	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	5.8	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	214	0	1240	0
V/C Ratio (X)	0.00	0.00	0.00	0.42	0.00	0.10	0.00
Avail Cap (c_a), veh/h	0	0	0	779	0	1240	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	43.0	0.0	4.4	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.5	0.0	0.2	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	43.5	0.0	4.6	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.6	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.1	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.0	0.0	0.7	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.00	0.00	0.08	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0

**Intersection Summary**

HCM 6th Ctrl Delay 12.2

HCM 6th LOS B

**Notes**

User approved volume balancing among the lanes for turning movement.

**Intersection**

Int Delay, s/veh 2.3

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖	↗	↖ ↗	↗		
Traffic Vol, veh/h	3	18	57	118	53	13
Future Vol, veh/h	3	18	57	118	53	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	95	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	19	4	7	8	0
Mvmt Flow	3	20	62	128	58	14

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	317	65	72	0	-
Stage 1	65	-	-	-	-
Stage 2	252	-	-	-	-
Critical Hdwy	6.4	6.39	4.14	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.471	2.236	-	-
Pot Cap-1 Maneuver	680	953	1515	-	-
Stage 1	963	-	-	-	-
Stage 2	795	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	650	953	1515	-	-
Mov Cap-2 Maneuver	650	-	-	-	-
Stage 1	921	-	-	-	-
Stage 2	795	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.1	2.4	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1515	-	650	953	-	-
HCM Lane V/C Ratio	0.041	-	0.005	0.021	-	-
HCM Control Delay (s)	7.5	0	10.6	8.9	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	0	0.1	-	-

Intersection									
Int Delay, s/veh	1.8								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	W			↑	↑				
Traffic Vol, veh/h	44	7	0	86	172	0			
Future Vol, veh/h	44	7	0	86	172	0			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	-	-	-	-	-			
Veh in Median Storage, #	0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	87	87	87	87	87	87			
Heavy Vehicles, %	13	0	0	7	4	0			
Mvmt Flow	51	8	0	99	198	0			
Major/Minor	Minor2	Major1		Major2					
Conflicting Flow All	297	198	-	0	-	0			
Stage 1	198	-	-	-	-	-			
Stage 2	99	-	-	-	-	-			
Critical Hdwy	6.53	6.2	-	-	-	-			
Critical Hdwy Stg 1	5.53	-	-	-	-	-			
Critical Hdwy Stg 2	5.53	-	-	-	-	-			
Follow-up Hdwy	3.617	3.3	-	-	-	-			
Pot Cap-1 Maneuver	672	848	0	-	-	0			
Stage 1	810	-	0	-	-	0			
Stage 2	898	-	0	-	-	0			
Platoon blocked, %		-	-						
Mov Cap-1 Maneuver	672	848	-	-	-	-			
Mov Cap-2 Maneuver	672	-	-	-	-	-			
Stage 1	810	-	-	-	-	-			
Stage 2	898	-	-	-	-	-			
Approach	EB	NB		SB					
HCM Control Delay, s	10.7	0		0					
HCM LOS	B								
Minor Lane/Major Mvmt	NBT	EBLn1	SBT						
Capacity (veh/h)	-	692	-						
HCM Lane V/C Ratio	-	0.085	-						
HCM Control Delay (s)	-	10.7	-						
HCM Lane LOS	-	B	-						
HCM 95th %tile Q(veh)	-	0.3	-						

Intersection						
Int Delay, s/veh	3.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	19	139	299	67	69	111
Future Vol, veh/h	19	139	299	67	69	111
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	14	2	4	4	1	6
Mvmt Flow	21	154	332	74	77	123
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	406	0	-	0	565	369
Stage 1	-	-	-	-	369	-
Stage 2	-	-	-	-	196	-
Critical Hdwy	4.24	-	-	-	6.41	6.26
Critical Hdwy Stg 1	-	-	-	-	5.41	-
Critical Hdwy Stg 2	-	-	-	-	5.41	-
Follow-up Hdwy	2.326	-	-	-	3.509	3.354
Pot Cap-1 Maneuver	1091	-	-	-	488	668
Stage 1	-	-	-	-	702	-
Stage 2	-	-	-	-	840	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1091	-	-	-	478	668
Mov Cap-2 Maneuver	-	-	-	-	478	-
Stage 1	-	-	-	-	687	-
Stage 2	-	-	-	-	840	-
Approach	EB	WB	SB			
HCM Control Delay, s	1	0	14.4			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	1091	-	-	-	580	-
HCM Lane V/C Ratio	0.019	-	-	-	0.345	-
HCM Control Delay (s)	8.4	0	-	-	14.4	-
HCM Lane LOS	A	A	-	-	B	-
HCM 95th %tile Q(veh)	0.1	-	-	-	1.5	-

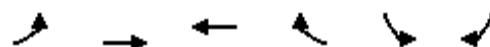
Intersection

Int Delay, s/veh 2.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	197	157	0	0	364	47	0	0	0	0	0	0
Future Vol, veh/h	197	157	0	0	364	47	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	84	84	84	84	84	84	84	84	84	84	84	84
Heavy Vehicles, %	3	2	0	0	4	0	2	2	2	2	2	2
Mvmt Flow	235	187	0	0	433	56	0	0	0	0	0	0

Major/Minor	Major1	Major2			Minor1			
Conflicting Flow All	489	0	-	-	0	-	1146	-
Stage 1	-	-	-	-	-	-	657	-
Stage 2	-	-	-	-	-	-	489	-
Critical Hdwy	4.13	-	-	-	-	-	6.52	-
Critical Hdwy Stg 1	-	-	-	-	-	-	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.52	-
Follow-up Hdwy	2.227	-	-	-	-	-	4.018	-
Pot Cap-1 Maneuver	1069	-	0	0	-	0	199	0
Stage 1	-	-	0	0	-	0	462	0
Stage 2	-	-	0	0	-	0	549	0
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1069	-	-	-	-	-	0	-
Mov Cap-2 Maneuver	-	-	-	-	-	-	0	-
Stage 1	-	-	-	-	-	-	0	-
Stage 2	-	-	-	-	-	-	0	-

Approach	EB	WB		NE	
HCM Control Delay, s	5.2	0		0	
HCM LOS		A			
<hr/>					
Minor Lane/Major Mvmt	NELn1	EBL	EBT	WBT	WBR
Capacity (veh/h)	-	1069	-	-	-
HCM Lane V/C Ratio	-	0.219	-	-	-
HCM Control Delay (s)	0	9.3	0	-	-
HCM Lane LOS	A	A	A	-	-
HCM 95th %tile Q(veh)	-	0.8	-	-	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	31	373	382	0	9	353
Future Volume (vph)	31	373	382	0	9	353
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.87	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1821	1810		1542	
Flt Permitted		0.95	1.00		1.00	
Satd. Flow (perm)		1734	1810		1542	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	32	381	390	0	9	360
RTOR Reduction (vph)	0	0	0	0	253	0
Lane Group Flow (vph)	0	413	390	0	116	0
Heavy Vehicles (%)	3%	4%	5%	0%	0%	7%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		85.0	40.0		40.0	
Effective Green, g (s)		85.0	40.0		40.0	
Actuated g/C Ratio		0.63	0.30		0.30	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1091	536		456		
v/s Ratio Prot		c0.22		c0.07		
v/s Ratio Perm		c0.24				
v/c Ratio		0.38	0.73		0.25	
Uniform Delay, d1	12.2	42.6		36.1		
Progression Factor	0.15	1.00		1.00		
Incremental Delay, d2	0.6	6.8		0.3		
Delay (s)	2.4	49.4		36.4		
Level of Service	A	D		D		
Approach Delay (s)	2.4	49.4		36.4		
Approach LOS	A	D		D		
<b>Intersection Summary</b>						
HCM 2000 Control Delay		28.8		HCM 2000 Level of Service	C	
HCM 2000 Volume to Capacity ratio		0.46				
Actuated Cycle Length (s)		135.0		Sum of lost time (s)	15.0	
Intersection Capacity Utilization		75.9%		ICU Level of Service	D	
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120;296; Craigville Logistics Warehouse

Build\_PM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↙	↖	↗
Traffic Volume (vph)	205	471	283	452	405	199
Future Volume (vph)	205	471	283	452	405	199
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	0.85	
Flt Protected	1.00	1.00	0.98	0.95	1.00	
Satd. Flow (prot)	1792	1583	1760	1752	1568	
Flt Permitted	1.00	1.00	0.78	0.95	1.00	
Satd. Flow (perm)	1792	1583	1407	1752	1568	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	209	481	289	461	413	203
RTOR Reduction (vph)	0	338	0	0	0	39
Lane Group Flow (vph)	209	143	0	750	413	164
Heavy Vehicles (%)	6%	2%	9%	4%	3%	3%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	40.0	40.0		85.0	40.0	40.0
Effective Green, g (s)	40.0	40.0		85.0	40.0	40.0
Actuated g/C Ratio	0.30	0.30		0.63	0.30	0.30
Clearance Time (s)	5.0	5.0		5.0	5.0	
Vehicle Extension (s)	6.0	6.0		3.0	3.0	
Lane Grp Cap (vph)	530	469		885	519	464
v/s Ratio Prot	0.12			c0.24		
v/s Ratio Perm		0.09		c0.53		0.10
v/c Ratio	0.39	0.30		0.85	0.80	0.35
Uniform Delay, d1	37.8	36.7		19.9	43.7	37.3
Progression Factor	1.00	1.00		0.93	1.00	1.00
Incremental Delay, d2	1.4	1.0		6.9	12.0	2.1
Delay (s)	39.2	37.8		25.3	55.7	39.4
Level of Service	D	D		C	E	D
Approach Delay (s)	38.2			25.3	50.3	
Approach LOS	D			C	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		37.1		HCM 2000 Level of Service		D
HCM 2000 Volume to Capacity ratio		0.86				
Actuated Cycle Length (s)		135.0		Sum of lost time (s)		15.0
Intersection Capacity Utilization		85.2%		ICU Level of Service		E
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120;296; Craigville Logistics Warehouse

Build\_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	114	322	214	456	378	22	266	196	284	70	178	113
Future Volume (veh/h)	114	322	214	456	378	22	266	196	284	70	178	113
Number	1	6	16	5	2	12	7	4	14	3	8	18
Initial Q, veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1900	1885	1841	1826	1870	1900	1870	1885	1826	1841	1856	1900
Adj Flow Rate, veh/h	116	329	218	465	386	22	271	200	290	71	182	115
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	1	4	5	2	0	2	1	5	4	3	0
Opposing Right Turn Influence	Yes			Yes			Yes			Yes		
Cap, veh/h	151	443	287	567	521	30	243	556	456	91	224	141
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.08	0.21	0.21	0.17	0.30	0.30	0.14	0.29	0.29	0.05	0.21	0.21
Unsig. Movement Delay												
Ln Grp Delay, s/veh	40.9	29.9	30.5	36.0	0.0	25.2	124.6	20.8	24.5	47.7	0.0	32.1
Ln Grp LOS	D	C	C	D	A	C	F	C	C	D	A	C
Approach Vol, veh/h	663			873			761			368		
Approach Delay, s/veh	32.1			31.0			59.2			35.1		
Approach LOS	C			C			E			D		
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Case No	2.0	4.0	2.0	3.0	2.0	4.0	2.0	4.0				
Phs Duration (G+Y+Rc), s	11.1	26.8	8.8	26.7	17.3	20.6	15.0	20.5				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Allow Headway (MAH), s	3.7	5.0	3.8	4.4	3.7	5.1	3.7	5.4				
Max Q Clear (g_c+l1), s	6.6	16.6	4.9	14.0	11.8	13.1	12.0	14.0				
Green Ext Time (g_e), s	0.2	2.3	0.1	1.6	0.6	2.5	0.0	1.3				
Prob of Phs Call (p_c)	0.91	1.00	0.77	1.00	1.00	1.00	1.00	1.00				
Prob of Max Out (p_x)	0.01	0.00	0.23	0.09	1.00	0.19	1.00	0.13				
Left-Turn Movement Data												
Assigned Mvmt	1		3		5		7					
Mvmt Sat Flow, veh/h	1810		1753		3374		1781					
Through Movement Data												
Assigned Mvmt	2		4		6		8					
Mvmt Sat Flow, veh/h	1752		1885		2082		1062					
Right-Turn Movement Data												
Assigned Mvmt	12		14		16		18					
Mvmt Sat Flow, veh/h	100		1546		1351		671					
Left Lane Group Data												
Assigned Mvmt	1	0	3	0	5	0	7	0				
Lane Assignment	L (Prot)		L (Prot)		L (Prot)		L (Prot)					

HCM 6th Signalized Intersection Capacity Analysis  
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	1	0	1	0	2	0	1	0
Grp Vol (v), veh/h	116	0	71	0	465	0	271	0
Grp Sat Flow (s), veh/h/ln	1810	0	1753	0	1687	0	1781	0
Q Serve Time (g_s), s	4.6	0.0	2.9	0.0	9.8	0.0	10.0	0.0
Cycle Q Clear Time (g_c), s	4.6	0.0	2.9	0.0	9.8	0.0	10.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Lane Grp Cap (c), veh/h	151	0	91	0	567	0	243	0
V/C Ratio (X)	0.77	0.00	0.78	0.00	0.82	0.00	1.12	0.00
Avail Cap (c_a), veh/h	370	0	239	0	689	0	243	0
Upstream Filter (l)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	33.0	0.0	34.4	0.0	29.5	0.0	31.7	0.0
Incr Delay (d2), s/veh	7.9	0.0	13.3	0.0	6.6	0.0	92.9	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	40.9	0.0	47.7	0.0	36.0	0.0	124.6	0.0
1st-Term Q (Q1), veh/ln	1.9	0.0	1.2	0.0	3.7	0.0	4.0	0.0
2nd-Term Q (Q2), veh/ln	0.3	0.0	0.3	0.0	0.5	0.0	6.3	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	2.2	0.0	1.6	0.0	4.2	0.0	10.3	0.0
%ile Storage Ratio (RQ%)	0.35	0.00	0.13	0.00	0.54	0.00	1.19	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	7.1	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.0

Middle Lane Group Data

Assigned Mvmt	0	2	0	4	0	6	0	8
Lane Assignment				T		T		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	200	0	282	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1885	0	1791	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	6.1	0.0	10.8	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	6.1	0.0	10.8	0.0	0.0
Lane Grp Cap (c), veh/h	0	0	0	556	0	381	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.36	0.00	0.74	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	642	0	610	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	20.4	0.0	27.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.4	0.0	2.8	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	20.8	0.0	29.9	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	2.5	0.0	4.3	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.3	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis  
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	3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	2.5	0.0	4.6	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.12	0.00	0.19	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	18
Lane Assignment		T+R		R		T+R		T+R
Lanes in Grp	0	1	0	1	0	1	0	1
Grp Vol (v), veh/h	0	408	0	290	0	265	0	297
Grp Sat Flow (s), veh/h/ln	0	1852	0	1546	0	1642	0	1734
Q Serve Time (g_s), s	0.0	14.6	0.0	12.0	0.0	11.1	0.0	12.0
Cycle Q Clear Time (g_c), s	0.0	14.6	0.0	12.0	0.0	11.1	0.0	12.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.05	0.00	1.00	0.00	0.82	0.00	0.39
Lane Grp Cap (c), veh/h	0	551	0	456	0	349	0	365
V/C Ratio (X)	0.00	0.74	0.00	0.64	0.00	0.76	0.00	0.81
Avail Cap (c_a), veh/h	0	1009	0	526	0	559	0	590
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00
Uniform Delay (d1), s/veh	0.0	23.3	0.0	22.5	0.0	27.1	0.0	27.6
Incr Delay (d2), s/veh	0.0	2.0	0.0	2.0	0.0	3.4	0.0	4.5
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	25.2	0.0	24.5	0.0	30.5	0.0	32.1
1st-Term Q (Q1), veh/ln	0.0	5.8	0.0	4.1	0.0	4.0	0.0	4.7
2nd-Term Q (Q2), veh/ln	0.0	0.3	0.0	0.3	0.0	0.3	0.0	0.5
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00
%ile Back of Q (50%), veh/ln	0.0	6.1	0.0	4.4	0.0	4.4	0.0	5.2
%ile Storage Ratio (RQ%)	0.00	0.49	0.00	0.22	0.00	0.18	0.00	0.43
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay			39.9					
HCM 6th LOS			D					

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse

Build\_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑	↑	↑
Traffic Volume (veh/h)	0	0	0	338	0	166	252	580	0	0	370	478
Future Volume (veh/h)	0	0	0	338	0	166	252	580	0	0	370	478
Number				7	4	14	5	2	12	1	6	16
Initial Q, veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)				1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		No
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln				1767	1900	1885	1811	1856	0	0	1856	1826
Adj Flow Rate, veh/h				345	0	169	257	592	0	0	378	488
Peak Hour Factor				0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %				9	0	1	6	3	0	0	3	5
Opposing Right Turn Influence				Yes			Yes			No		
Cap, veh/h				434	0	383	401	1097	0	0	675	602
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green				0.24	0.00	0.24	0.12	0.59	0.00	0.00	0.38	0.38
Unsig. Movement Delay												
Ln Grp Delay, s/veh				26.0	0.0	19.9	13.4	7.8	0.0	0.0	15.1	21.1
Ln Grp LOS				C	A	B	B	A	A	A	B	C
Approach Vol, veh/h				514				849			866	
Approach Delay, s/veh				24.0				9.5			18.5	
Approach LOS				C				A			B	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs				2		4	5	6				
Case No				4.0		11.0	1.2	8.0				
Phs Duration (G+Y+Rc), s				40.0		19.2	12.3	27.7				
Change Period (Y+Rc), s				5.0		5.0	5.0	5.0				
Max Green (Gmax), s				30.0		20.0	20.0	30.0				
Max Allow Headway (MAH), s				5.0		4.8	3.7	5.2				
Max Q Clear (g_c+l1), s				13.3		12.6	6.8	18.4				
Green Ext Time (g_e), s				3.4		1.6	0.6	4.2				
Prob of Phs Call (p_c)				1.00		1.00	1.00	1.00				
Prob of Max Out (p_x)				0.08		0.47	0.00	0.42				
Left-Turn Movement Data												
Assigned Mvmt					7	5	1					
Mvmt Sat Flow, veh/h					1810	1725	0					
Through Movement Data												
Assigned Mvmt				2		4		6				
Mvmt Sat Flow, veh/h				1856		0		1856				
Right-Turn Movement Data												
Assigned Mvmt				12		14		16				
Mvmt Sat Flow, veh/h				0		1598		1572				
Left Lane Group Data												
Assigned Mvmt	0	0	0	7	5	1	0	0				
Lane Assignment							L+TL (Pr/Pm)					

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse

Build\_PM Peak Hour

	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	345	257	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1810	1725	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	10.6	4.8	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	10.6	4.8	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	619	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	24.7	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	6.2	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	6.2	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	22.7	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	434	401	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.80	0.64	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	611	769	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	21.1	11.7	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	4.9	1.7	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	26.0	13.4	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	4.0	1.3	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.6	0.2	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	4.6	1.5	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.31	0.22	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	592	0	0	0	378	0	0
Grp Sat Flow (s), veh/h/ln	0	1856	0	0	0	1763	0	0
Q Serve Time (g_s), s	0.0	11.3	0.0	0.0	0.0	10.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	11.3	0.0	0.0	0.0	10.0	0.0	0.0
Lane Grp Cap (c), veh/h	0	1097	0	0	0	675	0	0
V/C Ratio (X)	0.00	0.54	0.00	0.00	0.00	0.56	0.00	0.00
Avail Cap (c_a), veh/h	0	1097	0	0	0	893	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	7.3	0.0	0.0	0.0	14.4	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.5	0.0	0.0	0.0	0.7	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	7.8	0.0	0.0	0.0	15.1	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	2.9	0.0	0.0	0.0	3.3	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.2	0.0	0.0	0.0	0.1	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse

Build\_PM Peak Hour

	1	2	3	4	5	6	7	8
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	3.1	0.0	0.0	0.0	3.4	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.26	0.00	0.00	0.00	0.17	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment				R		T+R		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	169	0	488	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1598	0	1572	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	5.3	0.0	16.4	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	5.3	0.0	16.4	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	383	0	602	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.44	0.00	0.81	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	540	0	797	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	19.1	0.0	16.3	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.8	0.0	4.8	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	19.9	0.0	21.1	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	4.9	0.0	4.8	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.8	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	5.0	0.0	5.6	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.34	0.00	0.28	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay				16.3				
HCM 6th LOS				B				

HCM 6th Signalized Intersection Cap\_NY-17 SB On-Ramp & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse

Build\_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	332	0	256	0	0	0	0	500	244	110	598	0
Future Volume (veh/h)	332	0	256	0	0	0	0	500	244	110	598	0
Number	7	4	14				5	2	12	1	6	16
Initial Q, veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00			1.00			1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No						No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1841	1900	1737				0	1841	1781	1885	1796	0
Adj Flow Rate, veh/h	346	0	267				0	521	254	115	623	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	4	0	11				0	4	8	1	7	0
Opposing Right Turn Influence	Yes						No			Yes		
Cap, veh/h	468	0	430				0	658	540	377	979	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.27	0.00	0.27				0.00	0.36	0.36	0.09	0.55	0.00
Unsig. Movement Delay												
Ln Grp Delay, s/veh	20.1	0.0	18.6				0.0	17.6	13.9	10.6	9.7	0.0
Ln Grp LOS	C	A	B				A	B	B	B	A	A
Approach Vol, veh/h	613							775			738	
Approach Delay, s/veh	19.5							16.4			9.8	
Approach LOS		B						B			A	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2		4		6						
Case No	1.2	7.0		10.0		4.0						
Phs Duration (G+Y+Rc), s	10.0	24.0		19.2		34.0						
Change Period (Y+Rc), s	5.0	5.0		5.0		5.0						
Max Green (Gmax), s	20.0	30.0		30.0		30.0						
Max Allow Headway (MAH), s	3.7	4.7		4.5		5.0						
Max Q Clear (g_c+l1), s	3.9	15.5		11.6		14.9						
Green Ext Time (g_e), s	0.2	3.5		2.6		3.5						
Prob of Phs Call (p_c)	1.00	1.00		1.00		1.00						
Prob of Max Out (p_x)	0.00	0.15		0.02		0.12						
Left-Turn Movement Data												
Assigned Mvmt	1	5		7								
Mvmt Sat Flow, veh/h	1795	0		1753								
Through Movement Data												
Assigned Mvmt	2		4		6							
Mvmt Sat Flow, veh/h	1841		0		1796							
Right-Turn Movement Data												
Assigned Mvmt	12		14		16							
Mvmt Sat Flow, veh/h	1510		1610		0							
Left Lane Group Data												
Assigned Mvmt	1	5	0	7	0	0	0	0				
Lane Assignment	L (Pr/Pm)			L								

HCM 6th Signalized Intersection Capacity Analysis & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse Build\_PM Peak Hour

Middle Lane Group Data

Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T			T			
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	521	0	0	0	623	0	0
Grp Sat Flow (s), veh/h/ln	0	1841	0	0	0	1796	0	0
Q Serve Time (g_s), s	0.0	13.5	0.0	0.0	0.0	12.9	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	13.5	0.0	0.0	0.0	12.9	0.0	0.0
Lane Grp Cap (c), veh/h	0	658	0	0	0	979	0	0
V/C Ratio (X)	0.00	0.79	0.00	0.00	0.00	0.64	0.00	0.00
Avail Cap (c_a), veh/h	0	1037	0	0	0	1012	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	15.3	0.0	0.0	0.0	8.4	0.0	0.0
Incr Delay (d2), s/veh	0.0	2.2	0.0	0.0	0.0	1.3	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	17.6	0.0	0.0	0.0	9.7	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	4.5	0.0	0.0	0.0	3.2	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.4	0.0	0.0	0.0	0.3	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse

	NY-17 SB On-Ramp	NY-17 SB Off-Ramp	NY-17 SB On-Ramp	NY-17 SB Off-Ramp	NY-17 SB On-Ramp	NY-17 SB Off-Ramp	NY-17 SB On-Ramp	Build_PM Peak Hour
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	4.9	0.0	0.0	0.0	3.6	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.17	0.00	0.00	0.00	0.31	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment		R		T+R				
Lanes in Grp	0	1	0	1	0	0	0	0
Grp Vol (v), veh/h	0	254	0	267	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1510	0	1610	0	0	0	0
Q Serve Time (g_s), s	0.0	6.9	0.0	7.8	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	6.9	0.0	7.8	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	540	0	430	0	0	0	0
V/C Ratio (X)	0.00	0.47	0.00	0.62	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	851	0	907	0	0	0	0
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	13.2	0.0	17.1	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.6	0.0	1.5	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	13.9	0.0	18.6	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	1.9	0.0	2.5	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.2	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	2.0	0.0	2.7	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.07	0.00	0.29	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay			15.0					
HCM 6th LOS			B					

**Intersection**

Int Delay, s/veh 7.1

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B	N			
Traffic Vol, veh/h	41	180	97	22	94	29
Future Vol, veh/h	41	180	97	22	94	29
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	3	4	3	10	6
Mvmt Flow	50	220	118	27	115	35

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	397	132	0	0	145
Stage 1	132	-	-	-	-
Stage 2	265	-	-	-	-
Critical Hdwy	6.4	6.23	-	-	4.2
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.327	-	-	2.29
Pot Cap-1 Maneuver	612	915	-	-	1390
Stage 1	899	-	-	-	-
Stage 2	784	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	561	915	-	-	1390
Mov Cap-2 Maneuver	561	-	-	-	-
Stage 1	899	-	-	-	-
Stage 2	718	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.5	0	6
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	819	1390	-
HCM Lane V/C Ratio	-	-	0.329	0.082	-
HCM Control Delay (s)	-	-	11.5	7.8	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	1.4	0.3	-

Intersection												
Int Delay, s/veh	33.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	+	+	+	+	+	+	+	+	+	+	+	+
Traffic Vol, veh/h	63	185	3	17	318	81	1	56	19	105	80	57
Future Vol, veh/h	63	185	3	17	318	81	1	56	19	105	80	57
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	1	2	0	6	4	4	0	0	0	1	3	5
Mvmt Flow	70	206	3	19	353	90	1	62	21	117	89	63
Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	443	0	0	209	0	0	860	829	208	825	785	398
Stage 1	-	-	-	-	-	-	348	348	-	436	436	-
Stage 2	-	-	-	-	-	-	512	481	-	389	349	-
Critical Hdwy	4.11	-	-	4.16	-	-	7.1	6.5	6.2	7.91	7.33	6.65
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.91	6.33	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.91	6.33	-
Follow-up Hdwy	2.209	-	-	2.254	-	-	3.5	4	3.3	3.509	4.027	3.345
Pot Cap-1 Maneuver	1122	-	-	1338	-	-	278	308	837	244	272	617
Stage 1	-	-	-	-	-	-	672	638	-	545	525	-
Stage 2	-	-	-	-	-	-	548	557	-	584	585	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1122	-	-	1338	-	-	168	281	837	184	248	617
Mov Cap-2 Maneuver	-	-	-	-	-	-	168	281	-	184	248	-
Stage 1	-	-	-	-	-	-	624	593	-	506	515	-
Stage 2	-	-	-	-	-	-	399	546	-	473	543	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	2.1		0.3		19.4		128					
HCM LOS					C		F					
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	333	1122	-	-	1338	-	-	246				
HCM Lane V/C Ratio	0.254	0.062	-	-	0.014	-	-	1.093				
HCM Control Delay (s)	19.4	8.4	0	-	7.7	0	-	128				
HCM Lane LOS	C	A	A	-	A	A	-	F				
HCM 95th %tile Q(veh)	1	0.2	-	-	0	-	-	11.6				

HCM 6th Signalized Intersection Capacity Analysis 50: NYS Route 208 & Museum Village Rd  
120;296; Craigville Logistics Warehouse Build\_PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	↑	↑	↑	↑	↑	↑		
Traffic Volume (veh/h)	184	225	175	1116	613	104		
Future Volume (veh/h)	184	225	175	1116	613	104		
Number	7	14	5	2	6	16		
Initial Q, veh	0	0	0	0	0	0		
Ped-Bike Adj (A_pbT)	1.00	1.00	1.00		1.00			
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00		
Work Zone On Approach	No			No	No			
Lanes Open During Work Zone								
Adj Sat Flow, veh/h/ln	1847	1876	1862	1847	1997	2012		
Adj Flow Rate, veh/h	196	239	186	1187	652	111		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94		
Percent Heavy Veh, %	2	0	1	2	4	3		
Opposing Right Turn Influence	Yes		Yes					
Cap, veh/h	291	347	502	1398	1330	1136		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00		
Prop Arrive On Green	0.17	0.17	0.05	0.76	0.67	0.67		
Unsig. Movement Delay								
Ln Grp Delay, s/veh	51.7	47.5	8.3	17.3	12.0	7.9		
Ln Grp LOS	D	D	A	B	B	A		
Approach Vol, veh/h	435			1373	763			
Approach Delay, s/veh	49.4			16.1	11.4			
Approach LOS	D			B	B			
Timer:	1	2	3	4	5	6	7	8
Assigned Phs		2		4	5	6		
Case No		4.0		9.0	1.2	7.0		
Phs Duration (G+Y+Rc), s		103.0		26.4	11.8	91.2		
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		
Max Green (Gmax), s		98.0		42.0	30.0	63.0		
Max Allow Headway (MAH), s		3.5		4.1	4.6	3.5		
Max Q Clear (g_c+l1), s		58.6		19.9	6.0	22.9		
Green Ext Time (g_e), s		5.4		1.5	0.8	2.2		
Prob of Phs Call (p_c)		1.00		1.00	1.00	1.00		
Prob of Max Out (p_x)		0.00		0.00	0.00	0.00		
Left-Turn Movement Data								
Assigned Mvmt			7	5	1			
Mvmt Sat Flow, veh/h			1759	1773	0			
Through Movement Data								
Assigned Mvmt		2		4		6		
Mvmt Sat Flow, veh/h		1847		0		1997		
Right-Turn Movement Data								
Assigned Mvmt		12		14		16		
Mvmt Sat Flow, veh/h		0		1590		1705		
Left Lane Group Data								
Assigned Mvmt	0	0	0	7	5	1	0	0
Lane Assignment				LL (Pr/Pm)				

HCM 6th Signalized Intersection Capacity Analysis 50: NYS Route 208 & Museum Village Rd  
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Build\_PM Peak Hour

Lanes in Grp	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	196	186	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1759	1773	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	13.5	4.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	13.5	4.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	1759	700	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	88.2	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	65.3	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	8.3	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	86.2	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	291	502	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.67	0.37	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	571	820	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	50.7	7.9	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	1.0	0.5	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	51.7	8.3	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	6.0	1.1	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.1	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	6.1	1.2	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	1.03	0.20	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	1187	0	0	0	652	0	0
Grp Sat Flow (s), veh/h/ln	0	1847	0	0	0	1997	0	0
Q Serve Time (g_s), s	0.0	56.6	0.0	0.0	0.0	20.9	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	56.6	0.0	0.0	0.0	20.9	0.0	0.0
Lane Grp Cap (c), veh/h	0	1398	0	0	0	1330	0	0
V/C Ratio (X)	0.00	0.85	0.00	0.00	0.00	0.49	0.00	0.00
Avail Cap (c_a), veh/h	0	1398	0	0	0	1330	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	10.7	0.0	0.0	0.0	10.7	0.0	0.0
Incr Delay (d2), s/veh	0.0	6.6	0.0	0.0	0.0	1.3	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	17.3	0.0	0.0	0.0	12.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	15.7	0.0	0.0	0.0	7.8	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	2.6	0.0	0.0	0.0	0.5	0.0	0.0

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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	18.3	0.0	0.0	0.0	8.2	0.0
%ile Storage Ratio (RQ%)	0.00	2.12	0.00	0.00	0.00	0.89	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>							
Assigned Mvmt	0	12	0	14	0	16	0
Lane Assignment				R		R	
Lanes in Grp	0	0	0	1	0	1	0
Grp Vol (v), veh/h	0	0	0	239	0	111	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1590	0	1705	0
Q Serve Time (g_s), s	0.0	0.0	0.0	17.9	0.0	3.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	17.9	0.0	3.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	1590.2	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	6.8	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	347	0	1136	0
V/C Ratio (X)	0.00	0.00	0.00	0.69	0.00	0.10	0.00
Avail Cap (c_a), veh/h	0	0	0	599	0	1136	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	46.6	0.0	7.7	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.9	0.0	0.2	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	47.5	0.0	7.9	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	0.9	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.1	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.1	0.0	1.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.01	0.00	0.13	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0

**Intersection Summary**

HCM 6th Ctrl Delay 20.3

HCM 6th LOS C

**Notes**

User approved volume balancing among the lanes for turning movement.

**Intersection**

Int Delay, s/veh 2.6

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖	↗	↖ ↗	↗		
Traffic Vol, veh/h	12	56	23	108	116	3
Future Vol, veh/h	12	56	23	108	116	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	95	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	6	22	3	1	0
Mvmt Flow	13	61	25	117	126	3

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	295	128	129	0	-	0
Stage 1	128	-	-	-	-	-
Stage 2	167	-	-	-	-	-
Critical Hdwy	6.4	6.26	4.32	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.354	2.398	-	-	-
Pot Cap-1 Maneuver	700	911	1342	-	-	-
Stage 1	903	-	-	-	-	-
Stage 2	867	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	686	911	1342	-	-	-
Mov Cap-2 Maneuver	686	-	-	-	-	-
Stage 1	885	-	-	-	-	-
Stage 2	867	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.4	1.4	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1342	-	686	911	-	-
HCM Lane V/C Ratio	0.019	-	0.019	0.067	-	-
HCM Control Delay (s)	7.7	0	10.4	9.2	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	0.1	0.2	-	-

Intersection									
Int Delay, s/veh	1.3								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	W			↑	↑				
Traffic Vol, veh/h	10	4	0	46	38	0			
Future Vol, veh/h	10	4	0	46	38	0			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	-	-	-	-	-			
Veh in Median Storage, #	0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	83	83	83	83	83	83			
Heavy Vehicles, %	0	0	0	0	0	0			
Mvmt Flow	12	5	0	55	46	0			
Major/Minor	Minor2	Major1		Major2					
Conflicting Flow All	101	46	-	0	-	0			
Stage 1	46	-	-	-	-	-			
Stage 2	55	-	-	-	-	-			
Critical Hdwy	6.4	6.2	-	-	-	-			
Critical Hdwy Stg 1	5.4	-	-	-	-	-			
Critical Hdwy Stg 2	5.4	-	-	-	-	-			
Follow-up Hdwy	3.5	3.3	-	-	-	-			
Pot Cap-1 Maneuver	902	1029	0	-	-	0			
Stage 1	982	-	0	-	-	0			
Stage 2	973	-	0	-	-	0			
Platoon blocked, %			-	-					
Mov Cap-1 Maneuver	902	1029	-	-	-	-			
Mov Cap-2 Maneuver	902	-	-	-	-	-			
Stage 1	982	-	-	-	-	-			
Stage 2	973	-	-	-	-	-			
Approach	EB	NB		SB					
HCM Control Delay, s	8.9	0		0					
HCM LOS	A								
Minor Lane/Major Mvmt	NBT	EBLn1	SBT						
Capacity (veh/h)	-	935	-						
HCM Lane V/C Ratio	-	0.018	-						
HCM Control Delay (s)	-	8.9	-						
HCM Lane LOS	-	A	-						
HCM 95th %tile Q(veh)	-	0.1	-						

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	11	132	204	35	28	13
Future Vol, veh/h	11	132	204	35	28	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	3	3	0	0	0
Mvmt Flow	12	147	227	39	31	14
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	266	0	-	0	418	247
Stage 1	-	-	-	-	247	-
Stage 2	-	-	-	-	171	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	1310	-	-	-	595	797
Stage 1	-	-	-	-	799	-
Stage 2	-	-	-	-	864	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1310	-	-	-	589	797
Mov Cap-2 Maneuver	-	-	-	-	589	-
Stage 1	-	-	-	-	791	-
Stage 2	-	-	-	-	864	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.6	0	11			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	1310	-	-	-	642	
HCM Lane V/C Ratio	0.009	-	-	-	0.071	
HCM Control Delay (s)	7.8	0	-	-	11	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.2	

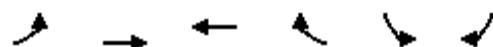
Intersection

Int Delay, s/veh 2.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	169	148	0	0	217	1	0	0	0	0	0	0
Future Vol, veh/h	169	148	0	0	217	1	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	1	1	0	0	3	0	2	2	2	2	2	2
Mvmt Flow	182	159	0	0	233	1	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	234	0	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.11	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.209	-	-
Pot Cap-1 Maneuver	1339	-	-
Stage 1	-	0	0
Stage 2	-	0	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1339	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NE
HCM Control Delay, s	4.3	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1339	-
HCM Lane V/C Ratio	-	0.136	-
HCM Control Delay (s)	0	8.1	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.5	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	46	335	237	11	12	196
Future Volume (vph)	46	335	237	11	12	196
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	0.99		0.87	
Flt Protected		0.99	1.00		1.00	
Satd. Flow (prot)		1868	1836		1639	
Flt Permitted		0.93	1.00		1.00	
Satd. Flow (perm)		1754	1836		1639	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	48	349	247	11	12	204
RTOR Reduction (vph)	0	0	1	0	156	0
Lane Group Flow (vph)	0	397	257	0	61	0
Heavy Vehicles (%)	2%	1%	3%	0%	0%	1%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		83.9	38.7		28.8	
Effective Green, g (s)		83.9	38.7		28.8	
Actuated g/C Ratio		0.68	0.32		0.23	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1199	579		384		
v/s Ratio Prot		c0.14		c0.04		
v/s Ratio Perm		c0.23				
v/c Ratio		0.33	0.44		0.16	
Uniform Delay, d1		7.9	33.4		37.3	
Progression Factor		0.13	1.00		1.00	
Incremental Delay, d2		0.4	1.5		0.2	
Delay (s)		1.4	35.0		37.5	
Level of Service		A	C		D	
Approach Delay (s)		1.4	35.0		37.5	
Approach LOS		A	C		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		20.3		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.33				
Actuated Cycle Length (s)		122.7		Sum of lost time (s)		15.0
Intersection Capacity Utilization		58.6%		ICU Level of Service		B
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
 Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
 120;296; Craigville Logistics Warehouse

Build\_SAT Midday Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↙	↖	↗
Traffic Volume (vph)	249	419	145	288	425	132
Future Volume (vph)	249	419	145	288	425	132
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1881	1599		1832	1787	1583
Flt Permitted	1.00	1.00		0.79	0.95	1.00
Satd. Flow (perm)	1881	1599		1476	1787	1583
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	259	436	151	300	443	138
RTOR Reduction (vph)	0	298	0	0	0	24
Lane Group Flow (vph)	259	138	0	451	443	114
Heavy Vehicles (%)	1%	1%	2%	2%	1%	2%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	38.7	38.7		72.5	40.2	40.2
Effective Green, g (s)	38.7	38.7		72.5	40.2	40.2
Actuated g/C Ratio	0.32	0.32		0.59	0.33	0.33
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	593	504		872	585	518
v/s Ratio Prot	0.14			c0.25		
v/s Ratio Perm		0.09		c0.31		0.07
v/c Ratio	0.44	0.27		0.52	0.76	0.22
Uniform Delay, d1	33.3	31.5		14.8	36.9	29.9
Progression Factor	1.00	1.00		0.45	1.00	1.00
Incremental Delay, d2	1.5	0.8		1.3	8.9	1.0
Delay (s)	34.8	32.3		7.9	45.8	30.9
Level of Service	C	C		A	D	C
Approach Delay (s)	33.2			7.9	42.2	
Approach LOS	C			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		29.6		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.63				
Actuated Cycle Length (s)		122.7		Sum of lost time (s)		15.0
Intersection Capacity Utilization		72.3%		ICU Level of Service		C
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120;296; Craigville Logistics Warehouse

Build\_SAT Midday Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘		↑ ↗	↑ ↘		↑ ↗	↑ ↘		↑ ↗	↑ ↘	
Traffic Volume (veh/h)	124	330	212	341	348	24	256	159	287	57	192	143
Future Volume (veh/h)	124	330	212	341	348	24	256	159	287	57	192	143
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1900	1900	1856	1885	1885	1781	1900	1870	1856	1900	1870	1900
Adj Flow Rate, veh/h	133	355	228	367	374	26	275	171	309	61	206	154
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	3	1	1	8	0	2	3	0	2	0
Cap, veh/h	171	468	296	476	458	32	241	622	523	79	242	181
Arrive On Green	0.09	0.22	0.22	0.14	0.26	0.26	0.13	0.33	0.33	0.04	0.24	0.24
Sat Flow, veh/h	1810	2124	1340	3483	1742	121	1810	1870	1572	1810	994	743
Grp Volume(v), veh/h	133	301	282	367	0	400	275	171	309	61	0	360
Grp Sat Flow(s), veh/h/ln	1810	1805	1659	1742	0	1863	1810	1870	1572	1810	0	1737
Q Serve(g_s), s	5.4	11.7	12.0	7.6	0.0	15.1	10.0	5.0	12.3	2.5	0.0	14.9
Cycle Q Clear(g_c), s	5.4	11.7	12.0	7.6	0.0	15.1	10.0	5.0	12.3	2.5	0.0	14.9
Prop In Lane	1.00		0.81	1.00		0.06	1.00		1.00	1.00		0.43
Lane Grp Cap(c), veh/h	171	398	366	476	0	490	241	622	523	79	0	422
V/C Ratio(X)	0.78	0.76	0.77	0.77	0.00	0.82	1.14	0.27	0.59	0.77	0.00	0.85
Avail Cap(c_a), veh/h	362	601	552	696	0	993	241	623	524	241	0	578
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	33.2	27.4	27.5	31.3	0.0	26.0	32.5	18.4	20.8	35.5	0.0	27.1
Incr Delay (d2), s/veh	7.4	2.9	3.7	3.2	0.0	3.4	101.2	0.2	1.8	14.7	0.0	8.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/lr	2.6	5.0	4.8	3.2	0.0	6.6	10.9	2.0	4.5	1.4	0.0	6.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	40.6	30.3	31.2	34.5	0.0	29.4	133.8	18.6	22.6	50.2	0.0	36.0
LnGrp LOS	D	C	C	C	A	C	F	B	C	D	A	D
Approach Vol, veh/h		716			767			755			421	
Approach Delay, s/veh		32.6			31.8			62.2			38.1	
Approach LOS		C			C			E			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	2.1	24.7	8.3	30.0	15.3	21.6	15.0	23.2				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (Gmax), s	40.0	10.0	25.0	15.0	25.0	10.0	25.0					
Max Q Clear Time (g_c+l), s	17.1	4.5	14.3	9.6	14.0	12.0	16.9					
Green Ext Time (p_c), s	0.2	2.3	0.0	1.5	0.6	2.6	0.0	1.4				
Intersection Summary												
HCM 6th Ctrl Delay		41.6										
HCM 6th LOS		D										

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120;296; Craigville Logistics Warehouse Build\_SAT Midday Peak Hour



HCM 6th Signalized Intersection Summary Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120;296; Craigville Logistics Warehouse Build\_SAT Midday Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘						↑ ↗	↑ ↘	↑ ↗	↑ ↘	
Traffic Volume (veh/h)	309	1	176	0	0	0	0	385	257	146	414	0
Future Volume (veh/h)	309	1	176	0	0	0	0	385	257	146	414	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No						No		No			
Adj Sat Flow, veh/h/ln	1870	1900	1856				0	1856	1841	1856	1870	0
Adj Flow Rate, veh/h	340	1	193				0	423	282	160	455	0
Peak Hour Factor	0.91	0.91	0.91				0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	0	3				0	3	4	3	2	0
Cap, veh/h	466	2	420				0	583	490	422	984	0
Arrive On Green	0.26	0.26	0.26				0.00	0.31	0.31	0.11	0.53	0.00
Sat Flow, veh/h	1781	8	1603				0	1856	1560	1767	1870	0
Grp Volume(v), veh/h	340	0	194				0	423	282	160	455	0
Grp Sat Flow(s), veh/h/ln	1781	0	1611				0	1856	1560	1767	1870	0
Q Serve(g_s), s	8.2	0.0	4.8				0.0	9.6	7.1	2.5	7.2	0.0
Cycle Q Clear(g_c), s	8.2	0.0	4.8				0.0	9.6	7.1	2.5	7.2	0.0
Prop In Lane	1.00		0.99				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	466	0	422				0	583	490	422	984	0
V/C Ratio(X)	0.73	0.00	0.46				0.00	0.73	0.58	0.38	0.46	0.00
Avail Cap(c_a), veh/h	1133	0	1025				0	1180	992	984	1190	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	15.9	0.0	14.6				0.0	14.4	13.5	9.3	7.0	0.0
Incr Delay (d2), s/veh	2.2	0.0	0.8				0.0	1.7	1.1	0.6	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	8.0	0.0	1.5				0.0	3.4	2.1	0.7	1.8	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	18.1	0.0	15.4				0.0	16.1	14.6	9.9	7.3	0.0
LnGrp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	534						705			615		
Approach Delay, s/veh	17.1						15.5			8.0		
Approach LOS	B						B			A		
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+Rc), s	0.0	19.8	17.3	29.8								
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	30.0	30.0	30.0	30.0								
Max Q Clear Time (g_c+l), s	11.6	10.2	9.2									
Green Ext Time (p_c), s	0.3	3.3	2.1	2.6								
Intersection Summary												
HCM 6th Ctrl Delay		13.5										
HCM 6th LOS		B										

Intersection

Int Delay, s/veh 5.5

Movement	WBL	WBR	NBT	NBR	SBL	SBT
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Lane Configurations						
Traffic Vol, veh/h	8	58	32	7	61	24
Future Vol, veh/h	8	58	32	7	61	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	0	3	0	0	2	0
Mvmt Flow	10	69	38	8	73	29

Major/Minor	Minor1	Major1	Major2
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Conflicting Flow All	217	42	0	0	46	0
Stage 1	42	-	-	-	-	-
Stage 2	175	-	-	-	-	-
Critical Hdwy	6.4	6.23	-	-	4.12	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.327	-	-	2.218	-
Pot Cap-1 Maneuver	776	1026	-	-	1562	-
Stage 1	986	-	-	-	-	-
Stage 2	860	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	740	1026	-	-	1562	-
Mov Cap-2 Maneuver	740	-	-	-	-	-
Stage 1	986	-	-	-	-	-
Stage 2	820	-	-	-	-	-

Approach	WB	NB	SB
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HCM Control Delay, s	9	0	5.3
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
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Capacity (veh/h)	-	-	980	1562	-
HCM Lane V/C Ratio	-	-	0.08	0.046	-
HCM Control Delay (s)	-	-	9	7.4	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.1	-

Intersection

Int Delay, s/veh 6.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	21	141	1	6	195	36	0	58	4	91	51	26
Future Vol, veh/h	21	141	1	6	195	36	0	58	4	91	51	26
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	1	0	17	2	0	0	0	25	1	0	4
Mvmt Flow	23	157	1	7	217	40	0	64	4	101	57	29

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	257	0	0	158	0	0	498	475	158	489	455	237
Stage 1	-	-	-	-	-	-	204	204	-	251	251	-
Stage 2	-	-	-	-	-	-	294	271	-	238	204	-
Critical Hdwy	4.1	-	-	4.27	-	-	7.1	6.5	6.45	7.91	7.3	6.64
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Follow-up Hdwy	2.2	-	-	2.353	-	-	3.5	4	3.525	3.509	4	3.336
Pot Cap-1 Maneuver	1320	-	-	1335	-	-	486	491	831	440	456	776
Stage 1	-	-	-	-	-	-	803	737	-	714	665	-
Stage 2	-	-	-	-	-	-	719	689	-	728	704	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1320	-	-	1335	-	-	415	479	831	385	445	776
Mov Cap-2 Maneuver	-	-	-	-	-	-	415	479	-	385	445	-
Stage 1	-	-	-	-	-	-	788	723	-	700	661	-
Stage 2	-	-	-	-	-	-	629	685	-	647	691	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	1	0.2			13.5			19.2				
HCM LOS					B			C				
<hr/>												
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	492	1320	-	-	1335	-	-	437				
HCM Lane V/C Ratio	0.14	0.018	-	-	0.005	-	-	0.427				
HCM Control Delay (s)	13.5	7.8	0	-	7.7	0	-	19.2				
HCM Lane LOS	B	A	A	-	A	A	-	C				
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0	-	-	2.1				

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	102	166	227	449	600	88
Future Volume (veh/h)	102	166	227	449	600	88
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00		1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No	No		
Adj Sat Flow, veh/h/ln	1832	1876	1876	1862	2042	2012
Adj Flow Rate, veh/h	106	173	236	468	625	92
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	3	0	0	1	1	3
Cap, veh/h	218	297	574	1469	1398	1168
Arrive On Green	0.13	0.13	0.06	0.79	0.68	0.68
Sat Flow, veh/h	1745	1590	1787	1862	2042	1705
Grp Volume(v), veh/h	106	173	236	468	625	92
Grp Sat Flow(s), veh/h/ln	1745	1590	1787	1862	2042	1705
Q Serve(g_s), s	6.6	11.6	4.2	8.3	16.2	2.1
Cycle Q Clear(g_c), s	6.6	11.6	4.2	8.3	16.2	2.1
Prop In Lane	1.00	1.00	1.00		1.00	
Lane Grp Cap(c), veh/h	218	297	574	1469	1398	1168
V/C Ratio(X)	0.49	0.58	0.41	0.32	0.45	0.08
Avail Cap(c_a), veh/h	718	753	924	1469	1398	1168
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	47.5	43.3	5.9	3.5	8.4	6.1
Incr Delay (d2), s/veh	0.6	0.7	0.5	0.6	1.0	0.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.9	10.2	1.1	2.0	6.0	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	48.1	43.9	6.3	4.0	9.4	6.3
LnGrp LOS	D	D	A	A	A	A
Approach Vol, veh/h	279			704	717	
Approach Delay, s/veh	45.5			4.8	9.0	
Approach LOS	D			A	A	
Timer - Assigned Phs	2		4	5	6	
Phs Duration (G+Y+R <sub>c</sub> ), s	97.0		19.6	12.2	84.8	
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0	
Max Green Setting (Gmax), s	92.0		48.0	30.0	57.0	
Max Q Clear Time (g_c+l1), s	10.3		13.6	6.2	18.2	
Green Ext Time (p_c), s	1.3		1.0	1.0	2.0	
Intersection Summary						
HCM 6th Ctrl Delay			13.3			
HCM 6th LOS			B			

Intersection

Int Delay, s/veh 1.9

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	2	8	14	42	30	4
Future Vol, veh/h	2	8	14	42	30	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	95	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	2	9	15	46	33	4

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All	111	35	37	0	-	0
Stage 1	35	-	-	-	-	-
Stage 2	76	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	891	1044	1587	-	-	-
Stage 1	993	-	-	-	-	-
Stage 2	952	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	882	1044	1587	-	-	-
Mov Cap-2 Maneuver	882	-	-	-	-	-
Stage 1	983	-	-	-	-	-
Stage 2	952	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s	8.6	1.8	0
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HCM LOS	A
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Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1587	-	882	1044	-	-
HCM Lane V/C Ratio	0.01	-	0.002	0.008	-	-
HCM Control Delay (s)	7.3	0	9.1	8.5	-	-
HCM Lane LOS	A	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0	0	-	-

**Intersection**

Int Delay, s/veh 3.4

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	79	7	0	97	72	0
Future Vol, veh/h	79	7	0	97	72	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	4	14	0	4	12	0
Mvmt Flow	88	8	0	108	80	0

Major/Minor	Minor2	Major1	Major2	
Conflicting Flow All	188	80	-	0
Stage 1	80	-	-	-
Stage 2	108	-	-	-
Critical Hdwy	6.44	6.34	-	-
Critical Hdwy Stg 1	5.44	-	-	-
Critical Hdwy Stg 2	5.44	-	-	-
Follow-up Hdwy	3.536	3.426	-	-
Pot Cap-1 Maneuver	797	948	0	-
Stage 1	938	-	0	-
Stage 2	911	-	0	-
Platoon blocked, %		-	-	-
Mov Cap-1 Maneuver	797	948	-	-
Mov Cap-2 Maneuver	797	-	-	-
Stage 1	938	-	-	-
Stage 2	911	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.1	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT
Capacity (veh/h)	-	807	-
HCM Lane V/C Ratio	-	0.118	-
HCM Control Delay (s)	-	10.1	-
HCM Lane LOS	-	B	-
HCM 95th %tile Q(veh)	-	0.4	-

Intersection						
Int Delay, s/veh	2.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	48	121	175	49	45	34
Future Vol, veh/h	48	121	175	49	45	34
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	3	5	7	5	5	21
Mvmt Flow	53	133	192	54	49	37
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	246	0	-	0	458	219
Stage 1	-	-	-	-	219	-
Stage 2	-	-	-	-	239	-
Critical Hdwy	4.13	-	-	-	6.45	6.41
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	2.227	-	-	-	3.545	3.489
Pot Cap-1 Maneuver	1314	-	-	-	555	775
Stage 1	-	-	-	-	810	-
Stage 2	-	-	-	-	794	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1314	-	-	-	531	775
Mov Cap-2 Maneuver	-	-	-	-	531	-
Stage 1	-	-	-	-	774	-
Stage 2	-	-	-	-	794	-
Approach	EB	WB	SB			
HCM Control Delay, s	2.2	0	11.8			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1314	-	-	-	614	
HCM Lane V/C Ratio	0.04	-	-	-	0.141	
HCM Control Delay (s)	7.9	0	-	-	11.8	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0.1	-	-	-	0.5	

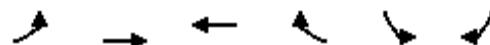
Intersection

Int Delay, s/veh 3.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	244	169	0	0	194	10	0	0	0	0	0	0
Future Vol, veh/h	244	169	0	0	194	10	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	8	4	0	0	9	36	2	2	2	2	2	2
Mvmt Flow	271	188	0	0	216	11	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	227	0	- - - 0 - 957 -
Stage 1	-	-	- - - 730 -
Stage 2	-	-	- - - 227 -
Critical Hdwy	4.18	- - - - -	- 6.52 -
Critical Hdwy Stg 1	-	- - - - -	- 5.52 -
Critical Hdwy Stg 2	-	- - - - -	- 5.52 -
Follow-up Hdwy	2.272	- - - - -	- 4.018 -
Pot Cap-1 Maneuver	1307	- 0 0 - - 0 258 0	
Stage 1	-	- 0 0 - - 0 428 0	
Stage 2	-	- 0 0 - - 0 716 0	
Platoon blocked, %	-	- - -	
Mov Cap-1 Maneuver	1307	- - - - -	- 0 -
Mov Cap-2 Maneuver	-	- - - - -	- 0 -
Stage 1	-	- - - - -	- 0 -
Stage 2	-	- - - - -	- 0 -

Approach	EB	WB	NE
HCM Control Delay, s	5	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1307	- - -
HCM Lane V/C Ratio	-	0.207	- - -
HCM Control Delay (s)	0	8.5	0 - -
HCM Lane LOS	A	A	A - -
HCM 95th %tile Q(veh)	-	0.8	- - -



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	15	404	195	0	15	182
Future Volume (vph)	15	404	195	0	15	182
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.88	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1781	1759		1504	
Flt Permitted		0.99	1.00		1.00	
Satd. Flow (perm)		1760	1759		1504	
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	17	464	224	0	17	209
RTOR Reduction (vph)	0	0	0	0	157	0
Lane Group Flow (vph)	0	481	224	0	69	0
Heavy Vehicles (%)	20%	6%	8%	0%	0%	11%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		84.1	38.9		31.3	
Effective Green, g (s)		84.1	38.9		31.3	
Actuated g/C Ratio		0.67	0.31		0.25	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1180	545		375		
v/s Ratio Prot		0.13		c0.05		
v/s Ratio Perm		c0.27				
v/c Ratio		0.41	0.41		0.18	
Uniform Delay, d1		9.4	34.2		37.0	
Progression Factor		0.12	1.00		1.00	
Incremental Delay, d2		0.6	1.4		0.2	
Delay (s)		1.7	35.6		37.2	
Level of Service		A	D		D	
Approach Delay (s)		1.7	35.6		37.2	
Approach LOS		A	D		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		18.5		HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio		0.36				
Actuated Cycle Length (s)		125.4		Sum of lost time (s)		15.0
Intersection Capacity Utilization		53.8%		ICU Level of Service		A
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_AM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↙	↖	↗
Traffic Volume (vph)	237	339	163	214	387	182
Future Volume (vph)	237	339	163	214	387	182
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1776	1524		1702	1719	1524
Flt Permitted	1.00	1.00		0.74	0.95	1.00
Satd. Flow (perm)	1776	1524		1280	1719	1524
Peak-hour factor, PHF	0.87	0.87	0.87	0.87	0.87	0.87
Adj. Flow (vph)	272	390	187	246	445	209
RTOR Reduction (vph)	0	269	0	0	0	36
Lane Group Flow (vph)	272	121	0	433	445	173
Heavy Vehicles (%)	7%	6%	11%	8%	5%	6%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	38.9	38.9		75.2	40.2	40.2
Effective Green, g (s)	38.9	38.9		75.2	40.2	40.2
Actuated g/C Ratio	0.31	0.31		0.60	0.32	0.32
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	550	472		767	551	488
v/s Ratio Prot	0.15			c0.26		
v/s Ratio Perm		0.08		c0.34		0.11
v/c Ratio	0.49	0.26		0.56	0.81	0.35
Uniform Delay, d1	35.2	32.4		15.2	39.1	32.7
Progression Factor	1.00	1.00		0.49	1.00	1.00
Incremental Delay, d2	2.0	0.8		1.9	12.0	2.0
Delay (s)	37.2	33.2		9.4	51.1	34.7
Level of Service	D	C		A	D	C
Approach Delay (s)	34.9			9.4	45.8	
Approach LOS	C			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		32.7		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.68				
Actuated Cycle Length (s)		125.4		Sum of lost time (s)		15.0
Intersection Capacity Utilization		66.7%		ICU Level of Service		C
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Summary 6: NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	65	196	140	391	200	10	104	135	334	48	220	83
Future Volume (veh/h)	65	196	140	391	200	10	104	135	334	48	220	83
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			0.99	1.00		0.99	1.00		1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1767	1841	1781	1826	1796	1604	1811	1856	1811	1767	1767	1737
Adj Flow Rate, veh/h	68	206	147	412	211	11	109	142	352	51	232	87
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	9	4	8	5	7	20	6	3	6	9	9	11
Cap, veh/h	85	359	243	552	497	26	140	523	433	62	291	109
Arrive On Green	0.05	0.18	0.18	0.16	0.29	0.29	0.08	0.28	0.28	0.04	0.24	0.24
Sat Flow, veh/h	1682	1986	1347	3374	1691	88	1725	1856	1535	1682	1225	459
Grp Volume(v), veh/h	68	180	173	412	0	222	109	142	352	51	0	319
Grp Sat Flow(s), veh/h/ln	1682	1749	1585	1687	0	1779	1725	1856	1535	1682	0	1684
Q Serve(g_s), s	2.4	5.6	6.0	6.9	0.0	6.0	3.7	3.5	12.7	1.8	0.0	10.6
Cycle Q Clear(g_c), s	2.4	5.6	6.0	6.9	0.0	6.0	3.7	3.5	12.7	1.8	0.0	10.6
Prop In Lane	1.00			0.85	1.00		0.05	1.00		1.00	1.00	0.27
Lane Grp Cap(c), veh/h	85	316	286	552	0	522	140	523	433	62	0	400
V/C Ratio(X)	0.80	0.57	0.60	0.75	0.00	0.42	0.78	0.27	0.81	0.83	0.00	0.80
Avail Cap(c_a), veh/h	425	737	668	853	0	1200	291	782	647	284	0	710
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	27.9	22.2	22.4	23.6	0.0	16.9	26.7	16.6	19.8	28.4	0.0	21.3
Incr Delay (d2), s/veh	15.5	1.6	2.0	2.0	0.0	0.5	9.1	0.3	4.9	23.3	0.0	3.7
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/lr	1.2	2.2	2.1	2.6	0.0	2.2	1.7	1.3	4.7	1.1	0.0	4.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	43.3	23.8	24.4	25.7	0.0	17.5	35.8	16.8	24.7	51.7	0.0	24.9
LnGrp LOS	D	C	C	C	A	B	D	B	C	D	A	C
Approach Vol, veh/h		421			634			603			370	
Approach Delay, s/veh		27.2			22.8			24.9			28.6	
Approach LOS		C			C			C			C	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+R <sub>c</sub> ), s	8.0	22.4	7.2	21.7	14.7	15.7	9.8	19.1				
Change Period (Y+R <sub>c</sub> ), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green Setting (G <sub>max</sub> ), s	40.0	10.0	25.0	15.0	25.0	10.0	25.0					
Max Q Clear Time (g <sub>c+l</sub> ), s	14.6	8.0	3.8	14.7	8.9	8.0	5.7	12.6				
Green Ext Time (p <sub>c</sub> ), s	0.1	1.2	0.0	1.5	0.8	1.8	0.1	1.5				
Intersection Summary												
HCM 6th Ctrl Delay			25.4									
HCM 6th LOS			C									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_AM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	0	0	0	161	0	60	127	513	0	0	307	445
Future Volume (veh/h)	0	0	0	161	0	60	127	513	0	0	307	445
Initial Q (Q <sub>b</sub> ), veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)				1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		
Adj Sat Flow, veh/h/ln	1693	1900	1826	1752	1826		0	0	1767	1826		
Adj Flow Rate, veh/h	179	0	67	141	570		0	0	341	494		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	14	0	5	10	5		0	0	9	5		
Cap, veh/h	273	0	234	401	1150		0	0	720	642		
Arrive On Green	0.15	0.00	0.15	0.09	0.63	0.00	0.00	0.43	0.43			
Sat Flow, veh/h	1810	0	1547	1668	1826		0	0	1767	1497		
Grp Volume(v), veh/h	179	0	67	141	570		0	0	341	494		
Grp Sat Flow(s), veh/h/ln	1810	0	1547	1668	1826		0	0	1678	1497		
Q Serve(g_s), s	4.2	0.0	1.8	1.8	7.7	0.0	0.0	0.0	6.6	12.8		
Cycle Q Clear(g_c), s	4.2	0.0	1.8	1.8	7.7	0.0	0.0	0.0	6.6	12.8		
Prop In Lane	1.00		1.00	1.00		0.00	0.00		1.00			
Lane Grp Cap(c), veh/h	273	0	234	401	1150		0	0	720	642		
V/C Ratio(X)	0.66	0.00	0.29	0.35	0.50	0.00	0.00	0.47	0.77			
Avail Cap(c_a), veh/h	794	0	679	982	1202		0	0	1105	986		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Upstream Filter(l)	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00			
Uniform Delay (d), s/veh	18.2	0.0	17.2	7.9	4.5	0.0	0.0	9.3	11.1			
Incr Delay (d2), s/veh	2.7	0.0	0.7	0.5	0.3	0.0	0.0	0.5	2.0			
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0			
%ile BackOfQ(50%), veh/ln	1.7	0.0	1.6	0.4	1.2	0.0	0.0	1.8	3.2			
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	20.9	0.0	17.8	8.4	4.9	0.0	0.0	9.8	13.1			
LnGrp LOS	C	A	B	A	A	A	A	A	A	B		
Approach Vol, veh/h				246			711			835		
Approach Delay, s/veh				20.1			5.6			11.7		
Approach LOS				C			A			B		
Timer - Assigned Phs	2		4	5	6							
Phs Duration (G+Y+R <sub>c</sub> ), s	33.7		11.9	9.1	24.6							
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	5.0	5.0							
Max Green Setting (Gmax), s	30.0		20.0	20.0	30.0							
Max Q Clear Time (g <sub>c+l1</sub> ), s	9.7		6.2	3.8	14.8							
Green Ext Time (p <sub>c</sub> ), s	3.4		1.0	0.3	4.7							
Intersection Summary												
HCM 6th Ctrl Delay			10.4									
HCM 6th LOS			B									

HCM 6th Signalized Intersection Summary Route 94 & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_AM Peak Hour



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑ ↗	↑ ↘						↑ ↗	↑ ↘	↑ ↗	↑ ↘	
Traffic Volume (veh/h)	328	1	210	0	0	0	0	310	346	138	330	0
Future Volume (veh/h)	328	1	210	0	0	0	0	310	346	138	330	0
Initial Q (Q <sub>b</sub> ), veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No						No		No			
Adj Sat Flow, veh/h/ln	1811	1870	1737				0	1796	1767	1796	1722	0
Adj Flow Rate, veh/h	364	1	233				0	344	384	153	367	0
Peak Hour Factor	0.90	0.90	0.90				0.90	0.90	0.90	0.90	0.90	0.90
Percent Heavy Veh, %	6	2	11				0	7	9	7	12	0
Cap, veh/h	482	2	442				0	593	494	427	904	0
Arrive On Green	0.28	0.28	0.28				0.00	0.33	0.33	0.10	0.53	0.00
Sat Flow, veh/h	1725	7	1579				0	1796	1497	1711	1722	0
Grp Volume(v), veh/h	364	0	234				0	344	384	153	367	0
Grp Sat Flow(s), veh/h/ln	1725	0	1586				0	1796	1497	1711	1722	0
Q Serve(g_s), s	9.9	0.0	6.4				0.0	8.1	11.8	2.7	6.6	0.0
Cycle Q Clear(g_c), s	9.9	0.0	6.4				0.0	8.1	11.8	2.7	6.6	0.0
Prop In Lane	1.00		1.00				0.00		1.00	1.00		0.00
Lane Grp Cap(c), veh/h	482	0	444				0	593	494	427	904	0
V/C Ratio(X)	0.75	0.00	0.53				0.00	0.58	0.78	0.36	0.41	0.00
Avail Cap(c_a), veh/h	1010	0	929				0	1052	877	928	1008	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	0.00	1.00				0.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	16.8	0.0	15.6				0.0	14.2	15.5	9.3	7.3	0.0
Incr Delay (d2), s/veh	2.4	0.0	1.0				0.0	0.9	2.7	0.5	0.3	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0				0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	8.6	0.0	2.1				0.0	2.8	3.6	0.8	1.7	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	19.3	0.0	16.6				0.0	15.1	18.2	9.8	7.6	0.0
LnGrp LOS	B	A	B				A	B	B	A	A	A
Approach Vol, veh/h	598						728			520		
Approach Delay, s/veh	18.2						16.7			8.3		
Approach LOS	B						B			A		
Timer - Assigned Phs	1	2	4	6								
Phs Duration (G+Y+Rc), s	0.0	21.9	19.3	31.9								
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0								
Max Green Setting (Gmax), s	30.0	30.0	30.0	30.0								
Max Q Clear Time (g_c+l), s	13.8	11.9	8.6									
Green Ext Time (p_c), s	0.3	3.1	2.5	2.0								
Intersection Summary												
HCM 6th Ctrl Delay			14.8									
HCM 6th LOS			B									

Intersection

Int Delay, s/veh 5.5

Movement	WBL	WBR	NBT	NBR	SBL	SBT
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Lane Configurations						
Traffic Vol, veh/h	15	46	34	27	152	51
Future Vol, veh/h	15	46	34	27	152	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	3	11	17	4	7	9
Mvmt Flow	18	56	41	33	185	62

Major/Minor	Minor1	Major1	Major2
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Conflicting Flow All	490	58	0	0	74	0
Stage 1	58	-	-	-	-	-
Stage 2	432	-	-	-	-	-
Critical Hdwy	6.43	6.31	-	-	4.17	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.399	-	-	2.263	-
Pot Cap-1 Maneuver	536	983	-	-	1494	-
Stage 1	962	-	-	-	-	-
Stage 2	652	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	467	983	-	-	1494	-
Mov Cap-2 Maneuver	467	-	-	-	-	-
Stage 1	962	-	-	-	-	-
Stage 2	569	-	-	-	-	-

Approach	WB	NB	SB
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HCM Control Delay, s	10.2	0	5.8
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HCM LOS	B
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Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	773	1494	-
HCM Lane V/C Ratio	-	-	0.096	0.124	-
HCM Control Delay (s)	-	-	10.2	7.8	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.4	-

Intersection												
Int Delay, s/veh	8.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	+	+	+	+	+	+	+	+	+	+	+	+
Traffic Vol, veh/h	37	159	1	10	207	30	0	86	9	91	23	56
Future Vol, veh/h	37	159	1	10	207	30	0	86	9	91	23	56
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	88	88	88	88	88	88	88	88	88	88	88	88
Heavy Vehicles, %	7	8	0	0	6	0	0	4	0	6	4	5
Mvmt Flow	42	181	1	11	235	34	0	98	10	103	26	64
Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	269	0	0	182	0	0	585	557	182	594	540	252
Stage 1	-	-	-	-	-	-	266	266	-	274	274	-
Stage 2	-	-	-	-	-	-	319	291	-	320	266	-
Critical Hdwy	4.17	-	-	4.1	-	-	7.1	6.54	6.2	7.96	7.34	6.65
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.54	-	6.96	6.34	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.54	-	6.96	6.34	-
Follow-up Hdwy	2.263	-	-	2.2	-	-	3.5	4.036	3.3	3.554	4.036	3.345
Pot Cap-1 Maneuver	1266	-	-	1405	-	-	425	436	866	360	395	758
Stage 1	-	-	-	-	-	-	744	685	-	681	639	-
Stage 2	-	-	-	-	-	-	697	668	-	636	646	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1266	-	-	1405	-	-	356	416	866	282	377	758
Mov Cap-2 Maneuver	-	-	-	-	-	-	356	416	-	282	377	-
Stage 1	-	-	-	-	-	-	716	660	-	656	633	-
Stage 2	-	-	-	-	-	-	607	662	-	516	622	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	1.5		0.3		15.9		24.7					
HCM LOS					C		C					
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	438	1266	-	-	1405	-	-	372				
HCM Lane V/C Ratio	0.246	0.033	-	-	0.008	-	-	0.519				
HCM Control Delay (s)	15.9	7.9	0	-	7.6	0	-	24.7				
HCM Lane LOS	C	A	A	-	A	A	-	C				
HCM 95th %tile Q(veh)	1	0.1	-	-	0	-	-	2.9				

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑
Traffic Volume (veh/h)	92	82	191	392	911	136
Future Volume (veh/h)	92	82	191	392	911	136
Initial Q (Q <sub>b</sub> ), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00	1.00			1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No	No		
Adj Sat Flow, veh/h/ln	1610	1802	1862	1699	1997	2012
Adj Flow Rate, veh/h	101	90	210	431	1001	149
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	18	5	1	12	4	3
Cap, veh/h	136	217	380	1394	1450	1238
Arrive On Green	0.09	0.09	0.05	0.82	0.73	0.73
Sat Flow, veh/h	1533	1527	1773	1699	1997	1705
Grp Volume(v), veh/h	101	90	210	431	1001	149
Grp Sat Flow(s), veh/h/ln	1533	1527	1773	1699	1997	1705
Q Serve(g_s), s	7.1	5.9	3.0	6.7	30.2	2.9
Cycle Q Clear(g_c), s	7.1	5.9	3.0	6.7	30.2	2.9
Prop In Lane	1.00	1.00	1.00			1.00
Lane Grp Cap(c), veh/h	136	217	380	1394	1450	1238
V/C Ratio(X)	0.74	0.42	0.55	0.31	0.69	0.12
Avail Cap(c_a), veh/h	699	778	778	1394	1450	1238
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	48.8	42.9	10.9	2.4	8.3	4.5
Incr Delay (d2), s/veh	3.0	0.5	1.3	0.6	2.7	0.2
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.8	0.0	2.0	1.1	9.8	0.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d), s/veh	51.8	43.4	12.1	2.9	11.0	4.7
LnGrp LOS	D	D	B	A	B	A
Approach Vol, veh/h	191			641	1150	
Approach Delay, s/veh	47.9			6.0	10.2	
Approach LOS	D			A	B	
Timer - Assigned Phs	2		4	5	6	
Phs Duration (G+Y+R <sub>c</sub> ), s	95.0		14.7	10.4	84.6	
Change Period (Y+R <sub>c</sub> ), s	5.0		5.0	4.5	5.0	
Max Green Setting (Gmax), s	90.0		50.0	30.5	55.0	
Max Q Clear Time (g_c+l1), s	8.7		9.1	5.0	32.2	
Green Ext Time (p_c), s	1.2		0.7	0.9	4.0	
Intersection Summary						
HCM 6th Ctrl Delay			12.4			
HCM 6th LOS			B			

**Intersection**

Int Delay, s/veh 2.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖	↗	↖	↗		
Traffic Vol, veh/h	3	13	57	119	54	13
Future Vol, veh/h	3	13	57	119	54	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	95	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	19	4	7	8	0
Mvmt Flow	3	14	62	129	59	14

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	319	66	73	0	-	0
Stage 1	66	-	-	-	-	-
Stage 2	253	-	-	-	-	-
Critical Hdwy	6.4	6.39	4.14	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.471	2.236	-	-	-
Pot Cap-1 Maneuver	678	952	1514	-	-	-
Stage 1	962	-	-	-	-	-
Stage 2	794	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	648	952	1514	-	-	-
Mov Cap-2 Maneuver	648	-	-	-	-	-
Stage 1	920	-	-	-	-	-
Stage 2	794	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	9.1	2.4	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1514	-	648	952	-	-
HCM Lane V/C Ratio	0.041	-	0.005	0.015	-	-
HCM Control Delay (s)	7.5	0	10.6	8.8	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	0	0	-	-

**Intersection**

Int Delay, s/veh 1.8

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			↑	↑	
Traffic Vol, veh/h	44	7	0	87	173	0
Future Vol, veh/h	44	7	0	87	173	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	13	0	0	7	4	0
Mvmt Flow	51	8	0	100	199	0

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	299	199	-	0	-
Stage 1	199	-	-	-	-
Stage 2	100	-	-	-	-
Critical Hdwy	6.53	6.2	-	-	-
Critical Hdwy Stg 1	5.53	-	-	-	-
Critical Hdwy Stg 2	5.53	-	-	-	-
Follow-up Hdwy	3.617	3.3	-	-	-
Pot Cap-1 Maneuver	670	847	0	-	0
Stage 1	809	-	0	-	0
Stage 2	897	-	0	-	0
Platoon blocked, %			-	-	
Mov Cap-1 Maneuver	670	847	-	-	-
Mov Cap-2 Maneuver	670	-	-	-	-
Stage 1	809	-	-	-	-
Stage 2	897	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	10.7	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT
Capacity (veh/h)	-	690	-
HCM Lane V/C Ratio	-	0.085	-
HCM Control Delay (s)	-	10.7	-
HCM Lane LOS	-	B	-
HCM 95th %tile Q(veh)	-	0.3	-

Intersection						
Int Delay, s/veh	4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	19	141	305	68	70	112
Future Vol, veh/h	19	141	305	68	70	112
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	14	2	4	4	1	6
Mvmt Flow	21	157	339	76	78	124
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	415	0	-	0	576	377
Stage 1	-	-	-	-	377	-
Stage 2	-	-	-	-	199	-
Critical Hdwy	4.24	-	-	-	6.41	6.26
Critical Hdwy Stg 1	-	-	-	-	5.41	-
Critical Hdwy Stg 2	-	-	-	-	5.41	-
Follow-up Hdwy	2.326	-	-	-	3.509	3.354
Pot Cap-1 Maneuver	1082	-	-	-	481	661
Stage 1	-	-	-	-	696	-
Stage 2	-	-	-	-	837	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1082	-	-	-	471	661
Mov Cap-2 Maneuver	-	-	-	-	471	-
Stage 1	-	-	-	-	681	-
Stage 2	-	-	-	-	837	-
Approach	EB	WB	SB			
HCM Control Delay, s	1	0	14.7			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1082	-	-	-	572	
HCM Lane V/C Ratio	0.02	-	-	-	0.354	
HCM Control Delay (s)	8.4	0	-	-	14.7	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0.1	-	-	-	1.6	

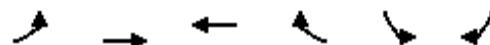
Intersection

Int Delay, s/veh 2.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	200	160	0	0	370	47	0	0	0	0	0	0
Future Vol, veh/h	200	160	0	0	370	47	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	84	84	84	84	84	84	84	84	84	84	84	84
Heavy Vehicles, %	3	2	0	0	4	0	2	2	2	2	2	2
Mvmt Flow	238	190	0	0	440	56	0	0	0	0	0	0

Major/Minor	Major1	Major2		Minor1		
Conflicting Flow All	496	0	-	-	0	-
Stage 1	-	-	-	-	-	666
Stage 2	-	-	-	-	-	496
Critical Hdwy	4.13	-	-	-	-	6.52
Critical Hdwy Stg 1	-	-	-	-	-	5.52
Critical Hdwy Stg 2	-	-	-	-	-	5.52
Follow-up Hdwy	2.227	-	-	-	-	4.018
Pot Cap-1 Maneuver	1063	-	0	0	-	0
Stage 1	-	-	0	0	-	0
Stage 2	-	-	0	0	-	545
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1063	-	-	-	-	0
Mov Cap-2 Maneuver	-	-	-	-	-	0
Stage 1	-	-	-	-	-	0
Stage 2	-	-	-	-	-	0

Approach	EB	WB	NE
HCM Control Delay, s	5.2	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1063	-
HCM Lane V/C Ratio	-	0.224	-
HCM Control Delay (s)	0	9.4	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.9	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	31	379	389	0	9	360
Future Volume (vph)	31	379	389	0	9	360
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.87	
Flt Protected		1.00	1.00		1.00	
Satd. Flow (prot)		1821	1827		1542	
Flt Permitted		0.95	1.00		1.00	
Satd. Flow (perm)		1734	1827		1542	
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	32	387	397	0	9	367
RTOR Reduction (vph)	0	0	0	0	258	0
Lane Group Flow (vph)	0	419	397	0	118	0
Heavy Vehicles (%)	3%	4%	4%	0%	0%	7%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		85.0	40.0		40.0	
Effective Green, g (s)		85.0	40.0		40.0	
Actuated g/C Ratio		0.63	0.30		0.30	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1091	541		456		
v/s Ratio Prot		c0.22		c0.08		
v/s Ratio Perm		c0.24				
v/c Ratio		0.38	0.73		0.26	
Uniform Delay, d1	12.2	42.7		36.2		
Progression Factor	0.15	1.00		1.00		
Incremental Delay, d2	0.6	7.0		0.3		
Delay (s)	2.4	49.7		36.5		
Level of Service	A	D		D		
Approach Delay (s)	2.4	49.7		36.5		
Approach LOS	A	D		D		
<b>Intersection Summary</b>						
HCM 2000 Control Delay		28.9		HCM 2000 Level of Service	C	
HCM 2000 Volume to Capacity ratio		0.47				
Actuated Cycle Length (s)		135.0		Sum of lost time (s)	15.0	
Intersection Capacity Utilization		76.7%		ICU Level of Service	D	
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
 Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
 120-269; Craigville Logistics Warehouse

2029 ETC+5\_PM Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↙	↖	↗
Traffic Volume (vph)	208	480	288	461	412	202
Future Volume (vph)	208	480	288	461	412	202
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1792	1583		1760	1752	1568
Flt Permitted	1.00	1.00		0.78	0.95	1.00
Satd. Flow (perm)	1792	1583		1405	1752	1568
Peak-hour factor, PHF	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	212	490	294	470	420	206
RTOR Reduction (vph)	0	345	0	0	0	39
Lane Group Flow (vph)	212	145	0	764	420	167
Heavy Vehicles (%)	6%	2%	9%	4%	3%	3%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	40.0	40.0		85.0	40.0	40.0
Effective Green, g (s)	40.0	40.0		85.0	40.0	40.0
Actuated g/C Ratio	0.30	0.30		0.63	0.30	0.30
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	530	469		884	519	464
v/s Ratio Prot	0.12			c0.24		
v/s Ratio Perm		0.09		c0.54		0.11
v/c Ratio	0.40	0.31		0.86	0.81	0.36
Uniform Delay, d1	37.9	36.8		20.3	44.0	37.4
Progression Factor	1.00	1.00		0.94	1.00	1.00
Incremental Delay, d2	1.4	1.1		7.9	12.8	2.2
Delay (s)	39.3	37.9		27.1	56.8	39.6
Level of Service	D	D		C	E	D
Approach Delay (s)	38.3			27.1	51.1	
Approach LOS	D			C	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		38.0		HCM 2000 Level of Service		D
HCM 2000 Volume to Capacity ratio		0.88				
Actuated Cycle Length (s)		135.0		Sum of lost time (s)		15.0
Intersection Capacity Utilization		86.5%		ICU Level of Service		E
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑↑	↑		↑	↑	↑	↑	↑↑	
Traffic Volume (veh/h)	116	328	218	464	385	22	271	200	289	71	181	115
Future Volume (veh/h)	116	328	218	464	385	22	271	200	289	71	181	115
Number	1	6	16	5	2	12	7	4	14	3	8	18
Initial Q, veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No		No		No	
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1900	1885	1841	1826	1870	1900	1870	1885	1826	1841	1856	1900
Adj Flow Rate, veh/h	118	335	222	473	393	22	277	204	295	72	185	117
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	1	4	5	2	0	2	1	5	4	3	0
Opposing Right Turn Influence	Yes		Yes			Yes		Yes		Yes		
Cap, veh/h	153	447	290	572	526	29	239	555	455	92	226	143
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.08	0.21	0.21	0.17	0.30	0.30	0.13	0.29	0.29	0.05	0.21	0.21
Unsig. Movement Delay												
Ln Grp Delay, s/veh	41.3	30.3	31.0	37.1	0.0	25.6	139.9	21.2	25.2	47.9	0.0	33.0
Ln Grp LOS	D	C	C	D	A	C	F	C	C	D	A	C
Approach Vol, veh/h	675			888			776			374		
Approach Delay, s/veh	32.5			31.7			65.1			35.9		
Approach LOS	C			C			E			D		
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Case No	2.0	4.0	2.0	3.0	2.0	4.0	2.0	4.0				
Phs Duration (G+Y+Rc), s	11.3	27.3	8.9	26.9	17.6	21.0	15.0	20.9				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Allow Headway (MAH), s	3.7	5.0	3.8	4.4	3.7	5.1	3.7	5.4				
Max Q Clear (g_c+l1), s	6.8	17.1	5.0	14.4	12.1	13.5	12.0	14.4				
Green Ext Time (g_e), s	0.2	2.4	0.1	1.6	0.5	2.5	0.0	1.3				
Prob of Phs Call (p_c)	0.91	1.00	0.77	1.00	1.00	1.00	1.00	1.00				
Prob of Max Out (p_x)	0.01	0.00	0.25	0.11	1.00	0.22	1.00	0.15				
Left-Turn Movement Data												
Assigned Mvmt	1		3		5		7					
Mvmt Sat Flow, veh/h	1810		1753		3374		1781					
Through Movement Data												
Assigned Mvmt	2		4		6		8					
Mvmt Sat Flow, veh/h	1754		1885		2081		1062					
Right-Turn Movement Data												
Assigned Mvmt	12		14		16		18					
Mvmt Sat Flow, veh/h	98		1546		1351		672					
Left Lane Group Data												
Assigned Mvmt	1	0	3	0	5	0	7	0				
Lane Assignment	L (Prot)		L (Prot)		L (Prot)		L (Prot)					

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_PM Peak Hour

Lanes in Grp	1	0	1	0	2	0	1	0
Grp Vol (v), veh/h	118	0	72	0	473	0	277	0
Grp Sat Flow (s), veh/h/ln	1810	0	1753	0	1687	0	1781	0
Q Serve Time (g_s), s	4.8	0.0	3.0	0.0	10.1	0.0	10.0	0.0
Cycle Q Clear Time (g_c), s	4.8	0.0	3.0	0.0	10.1	0.0	10.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Lane Grp Cap (c), veh/h	153	0	92	0	572	0	239	0
V/C Ratio (X)	0.77	0.00	0.78	0.00	0.83	0.00	1.16	0.00
Avail Cap (c_a), veh/h	364	0	235	0	679	0	239	0
Upstream Filter (l)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	33.4	0.0	34.8	0.0	29.9	0.0	32.2	0.0
Incr Delay (d2), s/veh	7.9	0.0	13.1	0.0	7.2	0.0	107.6	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	41.3	0.0	47.9	0.0	37.1	0.0	139.9	0.0
1st-Term Q (Q1), veh/ln	2.0	0.0	1.3	0.0	3.8	0.0	4.0	0.0
2nd-Term Q (Q2), veh/ln	0.3	0.0	0.3	0.0	0.6	0.0	7.2	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	2.3	0.0	1.6	0.0	4.4	0.0	11.2	0.0
%ile Storage Ratio (RQ%)	0.36	0.00	0.13	0.00	0.57	0.00	1.29	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	9.5	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	8
Lane Assignment				T		T		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	204	0	287	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1885	0	1791	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	6.4	0.0	11.2	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	6.4	0.0	11.2	0.0	0.0
Lane Grp Cap (c), veh/h	0	0	0	555	0	385	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.37	0.00	0.75	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	633	0	601	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	20.8	0.0	27.3	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.4	0.0	2.9	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	21.2	0.0	30.3	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	2.6	0.0	4.4	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.3	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_PM Peak Hour

3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	2.6	0.0	4.7	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.13	0.00	0.20	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>							
Assigned Mvmt	0	12	0	14	0	16	0
Lane Assignment		T+R		R		T+R	T+R
Lanes in Grp	0	1	0	1	0	1	0
Grp Vol (v), veh/h	0	415	0	295	0	270	0
Grp Sat Flow (s), veh/h/ln	0	1853	0	1546	0	1642	0
Q Serve Time (g_s), s	0.0	15.1	0.0	12.4	0.0	11.5	0.0
Cycle Q Clear Time (g_c), s	0.0	15.1	0.0	12.4	0.0	11.5	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.05	0.00	1.00	0.00	0.82	0.00
Lane Grp Cap (c), veh/h	0	555	0	455	0	353	0
V/C Ratio (X)	0.00	0.75	0.00	0.65	0.00	0.76	0.00
Avail Cap (c_a), veh/h	0	995	0	519	0	551	0
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	0.0	23.5	0.0	22.9	0.0	27.5	0.0
Incr Delay (d2), s/veh	0.0	2.0	0.0	2.3	0.0	3.5	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	25.6	0.0	25.2	0.0	31.0	0.0
1st-Term Q (Q1), veh/ln	0.0	6.0	0.0	4.3	0.0	4.2	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.3	0.0	0.3	0.0	0.3	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	6.3	0.0	4.6	0.0	4.5	0.0
%ile Storage Ratio (RQ%)	0.00	0.51	0.00	0.23	0.00	0.19	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>							
HCM 6th Ctrl Delay			42.0				
HCM 6th LOS			D				

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_PM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑↑	↑↑	
Traffic Volume (veh/h)	0	0	0	343	0	169	257	591	0	0	376	487
Future Volume (veh/h)	0	0	0	343	0	169	257	591	0	0	376	487
Number				7	4	14	5	2	12	1	6	16
Initial Q, veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)				1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln				1767	1900	1885	1811	1856	0	0	1856	1826
Adj Flow Rate, veh/h				350	0	172	262	603	0	0	384	497
Peak Hour Factor				0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %				9	0	1	6	3	0	0	3	5
Opposing Right Turn Influence				Yes			Yes			No		
Cap, veh/h				436	0	385	397	1101	0	0	680	607
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green				0.24	0.00	0.24	0.12	0.59	0.00	0.00	0.39	0.39
Unsig. Movement Delay												
Ln Grp Delay, s/veh				27.1	0.0	20.3	13.9	8.0	0.0	0.0	15.3	22.1
Ln Grp LOS				C	A	C	B	A	A	A	B	C
Approach Vol, veh/h								865			881	
Approach Delay, s/veh							24.8	9.8			19.1	
Approach LOS							C		A		B	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs				2		4	5	6				
Case No				4.0		11.0	1.2	8.0				
Phs Duration (G+Y+Rc), s				40.8		19.5	12.5	28.3				
Change Period (Y+Rc), s				5.0		5.0	5.0	5.0				
Max Green (Gmax), s				30.0		20.0	20.0	30.0				
Max Allow Headway (MAH), s				5.0		4.8	3.7	5.2				
Max Q Clear (g_c+l1), s				13.8		13.0	6.9	19.1				
Green Ext Time (g_e), s				3.4		1.6	0.6	4.1				
Prob of Phs Call (p_c)				1.00		1.00	1.00	1.00				
Prob of Max Out (p_x)				0.09		0.54	0.00	0.47				
Left-Turn Movement Data												
Assigned Mvmt					7	5	1					
Mvmt Sat Flow, veh/h					1810	1725	0					
Through Movement Data												
Assigned Mvmt				2		4		6				
Mvmt Sat Flow, veh/h				1856		0		1856				
Right-Turn Movement Data												
Assigned Mvmt				12		14		16				
Mvmt Sat Flow, veh/h				0		1598		1572				
Left Lane Group Data												
Assigned Mvmt	0	0	0	7	5	1	0	0				
Lane Assignment							L+TL (Pr/Pm)					

HCM 6th Signalized Intersection Capacity Analysis & NY-17 NB On-Ramp/NY-17 NB Off-Ramp  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_PM Peak Hour

Lanes in Grp	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	350	262	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1810	1725	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	11.0	4.9	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	11.0	4.9	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	610	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	25.3	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	6.1	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	6.1	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	23.3	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	436	397	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.80	0.66	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	600	753	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	21.6	12.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	5.5	1.9	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	27.1	13.9	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	4.2	1.3	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.7	0.2	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	4.8	1.5	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.33	0.24	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	603	0	0	0	384	0	0
Grp Sat Flow (s), veh/h/ln	0	1856	0	0	0	1763	0	0
Q Serve Time (g_s), s	0.0	11.8	0.0	0.0	0.0	10.3	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	11.8	0.0	0.0	0.0	10.3	0.0	0.0
Lane Grp Cap (c), veh/h	0	1101	0	0	0	680	0	0
V/C Ratio (X)	0.00	0.55	0.00	0.00	0.00	0.56	0.00	0.00
Avail Cap (c_a), veh/h	0	1101	0	0	0	876	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	7.4	0.0	0.0	0.0	14.6	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.6	0.0	0.0	0.0	0.7	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	8.0	0.0	0.0	0.0	15.3	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	3.1	0.0	0.0	0.0	3.4	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.2	0.0	0.0	0.0	0.1	0.0	0.0

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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	3.3	0.0	0.0	0.0	3.5	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.28	0.00	0.00	0.00	0.18	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment				R		T+R		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	172	0	497	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1598	0	1572	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	5.5	0.0	17.1	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	5.5	0.0	17.1	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	385	0	607	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.45	0.00	0.82	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	529	0	782	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	19.5	0.0	16.7	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.8	0.0	5.4	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	20.3	0.0	22.1	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	5.1	0.0	5.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.9	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	5.2	0.0	6.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.36	0.00	0.30	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay				16.9				
HCM 6th LOS				B				

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓						↑	↑	↑	↑	
Traffic Volume (veh/h)	338	0	260	0	0	0	0	509	248	112	607	0
Future Volume (veh/h)	338	0	260	0	0	0	0	509	248	112	607	0
Number	7	4	14				5	2	12	1	6	16
Initial Q, veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00			1.00			1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No						No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1841	1900	1737				0	1841	1781	1885	1796	0
Adj Flow Rate, veh/h	352	0	271				0	530	258	117	632	0
Peak Hour Factor	0.96	0.96	0.96				0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	4	0	11				0	4	8	1	7	0
Opposing Right Turn Influence	Yes						No			Yes		
Cap, veh/h	473	0	434				0	664	545	370	980	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.27	0.00	0.27				0.00	0.36	0.36	0.09	0.55	0.00
Unsig. Movement Delay												
Ln Grp Delay, s/veh	20.4	0.0	18.8				0.0	18.1	14.0	10.8	10.0	0.0
Ln Grp LOS	C	A	B				A	B	B	B	B	A
Approach Vol, veh/h	623							788			749	
Approach Delay, s/veh	19.7							16.7			10.2	
Approach LOS	B							B			B	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2		4		6						
Case No	1.2	7.0		10.0		4.0						
Phs Duration (G+Y+Rc), s	10.0	24.5		19.6		34.5						
Change Period (Y+Rc), s	5.0	5.0		5.0		5.0						
Max Green (Gmax), s	20.0	30.0		30.0		30.0						
Max Allow Headway (MAH), s	3.7	4.7		4.5		5.0						
Max Q Clear (g_c+l1), s	3.9	16.0		11.9		15.3						
Green Ext Time (g_e), s	0.2	3.5		2.7		3.5						
Prob of Phs Call (p_c)	1.00	1.00		1.00		1.00						
Prob of Max Out (p_x)	0.00	0.17		0.02		0.14						
Left-Turn Movement Data												
Assigned Mvmt	1	5		7								
Mvmt Sat Flow, veh/h	1795	0		1753								
Through Movement Data												
Assigned Mvmt	2		4		6							
Mvmt Sat Flow, veh/h	1841		0		1796							
Right-Turn Movement Data												
Assigned Mvmt	12		14		16							
Mvmt Sat Flow, veh/h	1510		1610		0							
Left Lane Group Data												
Assigned Mvmt	1	5	0	7	0	0	0	0				
Lane Assignment	L (Pr/Pm)			L								

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Lanes in Grp	1	0	0	1	0	0	0	0
Grp Vol (v), veh/h	117	0	0	352	0	0	0	0
Grp Sat Flow (s), veh/h/ln	1795	0	0	1753	0	0	0	0
Q Serve Time (g_s), s	1.9	0.0	0.0	9.9	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	1.9	0.0	0.0	9.9	0.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	693	0	0	1753	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	21.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	5.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	3.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	19.5	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	370	0	0	473	0	0	0	0
V/C Ratio (X)	0.32	0.00	0.00	0.74	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	868	0	0	972	0	0	0	0
Upstream Filter (l)	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	10.4	0.0	0.0	18.1	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.5	0.0	0.0	2.4	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	10.8	0.0	0.0	20.4	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.5	0.0	0.0	3.5	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.3	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.6	0.0	0.0	3.8	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.05	0.00	0.00	0.42	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	530	0	0	0	632	0	0
Grp Sat Flow (s), veh/h/ln	0	1841	0	0	0	1796	0	0
Q Serve Time (g_s), s	0.0	14.0	0.0	0.0	0.0	13.3	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	14.0	0.0	0.0	0.0	13.3	0.0	0.0
Lane Grp Cap (c), veh/h	0	664	0	0	0	980	0	0
V/C Ratio (X)	0.00	0.80	0.00	0.00	0.00	0.64	0.00	0.00
Avail Cap (c_a), veh/h	0	1020	0	0	0	996	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	15.5	0.0	0.0	0.0	8.6	0.0	0.0
Incr Delay (d2), s/veh	0.0	2.6	0.0	0.0	0.0	1.4	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	18.1	0.0	0.0	0.0	10.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	4.7	0.0	0.0	0.0	3.4	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.5	0.0	0.0	0.0	0.4	0.0	0.0

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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	5.1	0.0	0.0	0.0	3.8	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.18	0.00	0.00	0.00	0.33	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment		R		T+R				
Lanes in Grp	0	1	0	1	0	0	0	0
Grp Vol (v), veh/h	0	258	0	271	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1510	0	1610	0	0	0	0
Q Serve Time (g_s), s	0.0	7.1	0.0	8.0	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	7.1	0.0	8.0	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	545	0	434	0	0	0	0
V/C Ratio (X)	0.00	0.47	0.00	0.62	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	837	0	893	0	0	0	0
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	13.3	0.0	17.4	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.6	0.0	1.5	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	14.0	0.0	18.8	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	1.9	0.0	2.6	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.2	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	2.0	0.0	2.7	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.07	0.00	0.30	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay			15.3					
HCM 6th LOS			B					

**Intersection**

Int Delay, s/veh 7.2

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B	A			
Traffic Vol, veh/h	42	184	99	22	96	30
Future Vol, veh/h	42	184	99	22	96	30
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	3	4	3	10	6
Mvmt Flow	51	224	121	27	117	37

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	406	135	0	0	148
Stage 1	135	-	-	-	-
Stage 2	271	-	-	-	-
Critical Hdwy	6.4	6.23	-	-	4.2
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.327	-	-	2.29
Pot Cap-1 Maneuver	605	911	-	-	1386
Stage 1	896	-	-	-	-
Stage 2	779	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	553	911	-	-	1386
Mov Cap-2 Maneuver	553	-	-	-	-
Stage 1	896	-	-	-	-
Stage 2	712	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	11.7	0	6
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	813	1386	-
HCM Lane V/C Ratio	-	-	0.339	0.084	-
HCM Control Delay (s)	-	-	11.7	7.8	0
HCM Lane LOS	-	-	B	A	A
HCM 95th %tile Q(veh)	-	-	1.5	0.3	-

Intersection												
Int Delay, s/veh	38.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	+	+	+	+	+	+	+	+	+	+	+	
Traffic Vol, veh/h	64	188	3	17	324	83	1	57	19	107	81	58
Future Vol, veh/h	64	188	3	17	324	83	1	57	19	107	81	58
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	1	2	0	6	4	4	0	0	0	1	3	5
Mvmt Flow	71	209	3	19	360	92	1	63	21	119	90	64
Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	452	0	0	212	0	0	874	843	211	839	798	406
Stage 1	-	-	-	-	-	-	353	353	-	444	444	-
Stage 2	-	-	-	-	-	-	521	490	-	395	354	-
Critical Hdwy	4.11	-	-	4.16	-	-	7.1	6.5	6.2	7.91	7.33	6.65
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.91	6.33	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.91	6.33	-
Follow-up Hdwy	2.209	-	-	2.254	-	-	3.5	4	3.3	3.509	4.027	3.345
Pot Cap-1 Maneuver	1114	-	-	1335	-	-	272	303	834	238	266	610
Stage 1	-	-	-	-	-	-	668	634	-	539	520	-
Stage 2	-	-	-	-	-	-	542	552	-	579	581	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1114	-	-	1335	-	-	162	276	834	179	242	610
Mov Cap-2 Maneuver	-	-	-	-	-	-	162	276	-	179	242	-
Stage 1	-	-	-	-	-	-	620	588	-	500	510	-
Stage 2	-	-	-	-	-	-	392	542	-	467	539	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	2.1		0.3		19.9		146.2					
HCM LOS					C		F					
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	327	1114	-	-	1335	-	-	239				
HCM Lane V/C Ratio	0.262	0.064	-	-	0.014	-	-	1.144				
HCM Control Delay (s)	19.9	8.5	0	-	7.7	0	-	146.2				
HCM Lane LOS	C	A	A	-	A	A	-	F				
HCM 95th %tile Q(veh)	1	0.2	-	-	0	-	-	12.5				

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Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗	↑ ↗		
Traffic Volume (veh/h)	187	226	175	1135	623	105		
Future Volume (veh/h)	187	226	175	1135	623	105		
Number	7	14	5	2	6	16		
Initial Q, veh	0	0	0	0	0	0		
Ped-Bike Adj (A_pbT)	1.00	1.00	1.00		1.00			
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00		
Work Zone On Approach	No			No	No			
Lanes Open During Work Zone								
Adj Sat Flow, veh/h/ln	1847	1876	1862	1847	1997	2012		
Adj Flow Rate, veh/h	199	240	186	1207	663	112		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94		
Percent Heavy Veh, %	2	0	1	2	4	3		
Opposing Right Turn Influence	Yes		Yes					
Cap, veh/h	292	348	494	1397	1329	1135		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00		
Prop Arrive On Green	0.17	0.17	0.05	0.76	0.67	0.67		
Unsig. Movement Delay								
Ln Grp Delay, s/veh	51.8	47.5	8.5	18.4	12.2	7.9		
Ln Grp LOS	D	D	A	B	B	A		
Approach Vol, veh/h	439			1393	775			
Approach Delay, s/veh	49.4			17.1	11.6			
Approach LOS	D			B	B			
Timer:	1	2	3	4	5	6	7	8
Assigned Phs		2		4	5	6		
Case No		4.0		9.0	1.2	7.0		
Phs Duration (G+Y+Rc), s		103.0		26.5	11.8	91.2		
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		
Max Green (Gmax), s		98.0		42.0	30.0	63.0		
Max Allow Headway (MAH), s		3.5		4.1	4.6	3.5		
Max Q Clear (g_c+l1), s		61.5		20.0	6.0	23.5		
Green Ext Time (g_e), s		5.6		1.5	0.8	2.2		
Prob of Phs Call (p_c)		1.00		1.00	1.00	1.00		
Prob of Max Out (p_x)		0.00		0.00	0.00	0.00		
Left-Turn Movement Data								
Assigned Mvmt			7	5	1			
Mvmt Sat Flow, veh/h			1759	1773	0			
Through Movement Data								
Assigned Mvmt		2		4		6		
Mvmt Sat Flow, veh/h		1847		0		1997		
Right-Turn Movement Data								
Assigned Mvmt		12		14		16		
Mvmt Sat Flow, veh/h		0		1590		1705		
Left Lane Group Data								
Assigned Mvmt	0	0	0	7	5	1	0	0
Lane Assignment				LL (Pr/Pm)				

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Lanes in Grp	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	199	186	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1759	1773	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	13.8	4.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	13.8	4.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	1759	692	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	88.2	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	64.7	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	8.6	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	86.2	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	292	494	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.68	0.38	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	570	812	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	50.8	8.1	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	1.0	0.5	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	51.8	8.5	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	6.1	1.2	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.1	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	6.2	1.2	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	1.04	0.20	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	1207	0	0	0	663	0	0
Grp Sat Flow (s), veh/h/ln	0	1847	0	0	0	1997	0	0
Q Serve Time (g_s), s	0.0	59.5	0.0	0.0	0.0	21.5	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	59.5	0.0	0.0	0.0	21.5	0.0	0.0
Lane Grp Cap (c), veh/h	0	1397	0	0	0	1329	0	0
V/C Ratio (X)	0.00	0.86	0.00	0.00	0.00	0.50	0.00	0.00
Avail Cap (c_a), veh/h	0	1397	0	0	0	1329	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	11.1	0.0	0.0	0.0	10.8	0.0	0.0
Incr Delay (d2), s/veh	0.0	7.3	0.0	0.0	0.0	1.3	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	18.4	0.0	0.0	0.0	12.2	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	16.6	0.0	0.0	0.0	8.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	2.8	0.0	0.0	0.0	0.5	0.0	0.0

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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	19.4	0.0	0.0	0.0	8.5	0.0
%ile Storage Ratio (RQ%)	0.00	2.25	0.00	0.00	0.00	0.92	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>							
Assigned Mvmt	0	12	0	14	0	16	0
Lane Assignment				R		R	
Lanes in Grp	0	0	0	1	0	1	0
Grp Vol (v), veh/h	0	0	0	240	0	112	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1590	0	1705	0
Q Serve Time (g_s), s	0.0	0.0	0.0	18.0	0.0	3.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	18.0	0.0	3.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	1590.2	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	6.8	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	348	0	1135	0
V/C Ratio (X)	0.00	0.00	0.00	0.69	0.00	0.10	0.00
Avail Cap (c_a), veh/h	0	0	0	599	0	1135	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	46.6	0.0	7.8	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.9	0.0	0.2	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	47.5	0.0	7.9	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	1.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.1	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.1	0.0	1.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.01	0.00	0.13	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0

**Intersection Summary**

HCM 6th Ctrl Delay 20.9

HCM 6th LOS C

**Notes**

User approved volume balancing among the lanes for turning movement.

Intersection

Int Delay, s/veh 2.5

Movement	EBL	EBR	NBL	NBT	SBT	SBR
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Lane Configurations						
Traffic Vol, veh/h	12	56	23	110	117	3
Future Vol, veh/h	12	56	23	110	117	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	95	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	6	22	3	1	0
Mvmt Flow	13	61	25	120	127	3

Major/Minor	Minor2	Major1	Major2
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Conflicting Flow All	299	129	130	0	-	0
Stage 1	129	-	-	-	-	-
Stage 2	170	-	-	-	-	-
Critical Hdwy	6.4	6.26	4.32	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.354	2.398	-	-	-
Pot Cap-1 Maneuver	697	910	1341	-	-	-
Stage 1	902	-	-	-	-	-
Stage 2	865	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	683	910	1341	-	-	-
Mov Cap-2 Maneuver	683	-	-	-	-	-
Stage 1	884	-	-	-	-	-
Stage 2	865	-	-	-	-	-

Approach	EB	NB	SB
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HCM Control Delay, s	9.4	1.3	0
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HCM LOS	A
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Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1341	-	683	910	-	-
HCM Lane V/C Ratio	0.019	-	0.019	0.067	-	-
HCM Control Delay (s)	7.7	0	10.4	9.2	-	-
HCM Lane LOS	A	A	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	0.1	0.2	-	-

Intersection									
Int Delay, s/veh	1.2								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	W			↑	↑				
Traffic Vol, veh/h	10	4	0	47	39	0			
Future Vol, veh/h	10	4	0	47	39	0			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	-	-	-	-	-			
Veh in Median Storage, #	0	-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	83	83	83	83	83	83			
Heavy Vehicles, %	0	0	0	0	0	0			
Mvmt Flow	12	5	0	57	47	0			
Major/Minor	Minor2	Major1		Major2					
Conflicting Flow All	104	47	-	0	-	0			
Stage 1	47	-	-	-	-	-			
Stage 2	57	-	-	-	-	-			
Critical Hdwy	6.4	6.2	-	-	-	-			
Critical Hdwy Stg 1	5.4	-	-	-	-	-			
Critical Hdwy Stg 2	5.4	-	-	-	-	-			
Follow-up Hdwy	3.5	3.3	-	-	-	-			
Pot Cap-1 Maneuver	899	1028	0	-	-	0			
Stage 1	981	-	0	-	-	0			
Stage 2	971	-	0	-	-	0			
Platoon blocked, %			-	-					
Mov Cap-1 Maneuver	899	1028	-	-	-	-			
Mov Cap-2 Maneuver	899	-	-	-	-	-			
Stage 1	981	-	-	-	-	-			
Stage 2	971	-	-	-	-	-			
Approach	EB	NB		SB					
HCM Control Delay, s	8.9	0		0					
HCM LOS	A								
Minor Lane/Major Mvmt	NBT	EBLn1	SBT						
Capacity (veh/h)	-	932	-						
HCM Lane V/C Ratio	-	0.018	-						
HCM Control Delay (s)	-	8.9	-						
HCM Lane LOS	-	A	-						
HCM 95th %tile Q(veh)	-	0.1	-						

Intersection						
Int Delay, s/veh	1.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	11	134	208	36	28	13
Future Vol, veh/h	11	134	208	36	28	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	3	3	0	0	0
Mvmt Flow	12	149	231	40	31	14
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	271	0	-	0	424	251
Stage 1	-	-	-	-	251	-
Stage 2	-	-	-	-	173	-
Critical Hdwy	4.1	-	-	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	1304	-	-	-	591	793
Stage 1	-	-	-	-	795	-
Stage 2	-	-	-	-	862	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1304	-	-	-	585	793
Mov Cap-2 Maneuver	-	-	-	-	585	-
Stage 1	-	-	-	-	787	-
Stage 2	-	-	-	-	862	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.6	0	11.1			
HCM LOS			B			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1304	-	-	-	638	
HCM Lane V/C Ratio	0.009	-	-	-	0.071	
HCM Control Delay (s)	7.8	0	-	-	11.1	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.2	

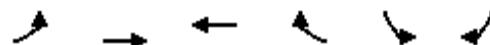
Intersection

Int Delay, s/veh 2.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations												
Traffic Vol, veh/h	172	150	0	0	221	1	0	0	0	0	0	0
Future Vol, veh/h	172	150	0	0	221	1	0	0	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	1	1	0	0	3	0	2	2	2	2	2	2
Mvmt Flow	185	161	0	0	238	1	0	0	0	0	0	0

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	239	0	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.11	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.209	-	-
Pot Cap-1 Maneuver	1334	-	-
Stage 1	-	0	0
Stage 2	-	0	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1334	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NE
HCM Control Delay, s	4.3	0	0
HCM LOS			A
<hr/>			
Minor Lane/Major Mvmt	NELn1	EBL	EBT
Capacity (veh/h)	-	1334	-
HCM Lane V/C Ratio	-	0.139	-
HCM Control Delay (s)	0	8.1	0
HCM Lane LOS	A	A	A
HCM 95th %tile Q(veh)	-	0.5	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	47	341	342	11	12	200
Future Volume (vph)	47	341	342	11	12	200
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		5.0	5.0		5.0	
Lane Util. Factor		1.00	1.00		1.00	
Frt		1.00	1.00		0.87	
Flt Protected		0.99	1.00		1.00	
Satd. Flow (prot)		1868	1839		1623	
Flt Permitted		0.92	1.00		1.00	
Satd. Flow (perm)		1723	1839		1623	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	49	355	356	11	12	208
RTOR Reduction (vph)	0	0	1	0	159	0
Lane Group Flow (vph)	0	404	366	0	62	0
Heavy Vehicles (%)	2%	1%	3%	0%	0%	2%
Turn Type	Perm	NA	NA		Prot	
Protected Phases		1 2	1		3	
Permitted Phases		1 2				
Actuated Green, G (s)		85.2	40.1		29.4	
Effective Green, g (s)		85.2	40.1		29.4	
Actuated g/C Ratio		0.68	0.32		0.24	
Clearance Time (s)			5.0		5.0	
Vehicle Extension (s)			6.0		3.0	
Lane Grp Cap (vph)	1178	591		382		
v/s Ratio Prot		c0.20		c0.04		
v/s Ratio Perm		c0.23				
v/c Ratio		0.34	0.62		0.16	
Uniform Delay, d1		8.1	35.8		37.8	
Progression Factor		0.13	1.00		1.00	
Incremental Delay, d2		0.5	3.4		0.2	
Delay (s)		1.5	39.2		38.0	
Level of Service		A	D		D	
Approach Delay (s)		1.5	39.2		38.0	
Approach LOS		A	D		D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		23.6		HCM 2000 Level of Service	C	
HCM 2000 Volume to Capacity ratio		0.40				
Actuated Cycle Length (s)		124.6		Sum of lost time (s)	15.0	
Intersection Capacity Utilization		64.7%		ICU Level of Service	C	
Analysis Period (min)		15				
c Critical Lane Group						

HCM Signalized Intersection Capacity Analysis  
Kings Hwy (CR 13) & NYS 17M/NYS Route 17M  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_SAT Midday Peak Hour



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↗	↖	↙	↖	↗
Traffic Volume (vph)	254	427	148	294	433	134
Future Volume (vph)	254	427	148	294	433	134
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		1.00	1.00	1.00
Frt	1.00	0.85		1.00	1.00	0.85
Flt Protected	1.00	1.00		0.98	0.95	1.00
Satd. Flow (prot)	1881	1599		1832	1787	1583
Flt Permitted	1.00	1.00		0.79	0.95	1.00
Satd. Flow (perm)	1881	1599		1470	1787	1583
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	265	445	154	306	451	140
RTOR Reduction (vph)	0	302	0	0	0	24
Lane Group Flow (vph)	265	143	0	460	451	116
Heavy Vehicles (%)	1%	1%	2%	2%	1%	2%
Turn Type	NA	Perm	Perm	NA	Prot	Perm
Protected Phases	1			1 3	2	
Permitted Phases		1	1 3			2
Actuated Green, G (s)	40.1	40.1		74.5	40.1	40.1
Effective Green, g (s)	40.1	40.1		74.5	40.1	40.1
Actuated g/C Ratio	0.32	0.32		0.60	0.32	0.32
Clearance Time (s)	5.0	5.0			5.0	5.0
Vehicle Extension (s)	6.0	6.0			3.0	3.0
Lane Grp Cap (vph)	605	514		878	575	509
v/s Ratio Prot	0.14			c0.25		
v/s Ratio Perm		0.09		c0.31		0.07
v/c Ratio	0.44	0.28		0.52	0.78	0.23
Uniform Delay, d1	33.4	31.5		14.7	38.3	30.9
Progression Factor	1.00	1.00		0.35	1.00	1.00
Incremental Delay, d2	1.4	0.8		1.2	10.3	1.0
Delay (s)	34.8	32.3		6.4	48.6	32.0
Level of Service	C	C		A	D	C
Approach Delay (s)	33.2			6.4	44.7	
Approach LOS	C			A	D	
<b>Intersection Summary</b>						
HCM 2000 Control Delay		30.1		HCM 2000 Level of Service		C
HCM 2000 Volume to Capacity ratio		0.64				
Actuated Cycle Length (s)		124.6		Sum of lost time (s)		15.0
Intersection Capacity Utilization		73.5%		ICU Level of Service		D
Analysis Period (min)		15				
c Critical Lane Group						

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

2029 ETC+5\_SAT Midday Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑↓		↑↑	↑↓		↑	↑	↑	↑	↑↓	
Traffic Volume (veh/h)	126	339	216	351	357	24	261	162	397	58	195	146
Future Volume (veh/h)	126	339	216	351	357	24	261	162	397	58	195	146
Number	1	6	16	5	2	12	7	4	14	3	8	18
Initial Q, veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00			1.00	1.00		1.00	1.00		1.00	1.00	1.00
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1900	1900	1856	1885	1885	1781	1900	1870	1856	1900	1870	1900
Adj Flow Rate, veh/h	135	365	232	377	384	26	281	174	427	62	210	157
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Percent Heavy Veh, %	0	0	3	1	1	8	0	2	3	0	2	0
Opposing Right Turn Influence	Yes			Yes			Yes			Yes		
Cap, veh/h	173	476	298	483	465	31	237	621	522	80	244	183
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.10	0.22	0.22	0.14	0.27	0.27	0.13	0.33	0.33	0.04	0.25	0.25
Unsig. Movement Delay												
Ln Grp Delay, s/veh	41.3	31.4	32.3	35.6	0.0	29.9	152.1	19.1	33.3	50.6	0.0	37.5
Ln Grp LOS	D	C	C	D	A	C	F	B	C	D	A	D
Approach Vol, veh/h	732				787			882			429	
Approach Delay, s/veh	33.6				32.7			68.3			39.4	
Approach LOS	C				C			E			D	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2	3	4	5	6	7	8				
Case No	2.0	4.0	2.0	3.0	2.0	4.0	2.0	4.0				
Phs Duration (G+Y+Rc), s	12.3	25.4	8.4	30.4	15.6	22.1	15.0	23.8				
Change Period (Y+Rc), s	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0				
Max Green (Gmax), s	15.0	40.0	10.0	25.0	15.0	25.0	10.0	25.0				
Max Allow Headway (MAH), s	3.7	5.0	3.8	4.3	3.7	5.1	3.7	5.4				
Max Q Clear (g_c+l1), s	7.6	17.8	4.6	21.0	10.0	14.5	12.0	17.5				
Green Ext Time (g_e), s	0.2	2.3	0.0	1.0	0.6	2.6	0.0	1.4				
Prob of Phs Call (p_c)	0.94	1.00	0.73	1.00	1.00	1.00	1.00	1.00				
Prob of Max Out (p_x)	0.03	0.00	0.13	1.00	0.46	0.30	1.00	0.51				
Left-Turn Movement Data												
Assigned Mvmt	1		3		5		7					
Mvmt Sat Flow, veh/h	1810		1810		3483		1810					
Through Movement Data												
Assigned Mvmt	2		4		6		8					
Mvmt Sat Flow, veh/h	1746		1870		2132		994					
Right-Turn Movement Data												
Assigned Mvmt	12		14		16		18					
Mvmt Sat Flow, veh/h	118		1572		1333		743					
Left Lane Group Data												
Assigned Mvmt	1	0	3	0	5	0	7	0				
Lane Assignment	L (Prot)		L (Prot)		L (Prot)		L (Prot)					

HCM 6th Signalized Intersection Capacity Analysis  
NYS Route 94/Academy Ave & NYS Route 17M  
120-269; Craigville Logistics Warehouse

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Lanes in Grp	1	0	1	0	2	0	1	0
Grp Vol (v), veh/h	135	0	62	0	377	0	281	0
Grp Sat Flow (s), veh/h/ln	1810	0	1810	0	1742	0	1810	0
Q Serve Time (g_s), s	5.6	0.0	2.6	0.0	8.0	0.0	10.0	0.0
Cycle Q Clear Time (g_c), s	5.6	0.0	2.6	0.0	8.0	0.0	10.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Lane Grp Cap (c), veh/h	173	0	80	0	483	0	237	0
V/C Ratio (X)	0.78	0.00	0.77	0.00	0.78	0.00	1.19	0.00
Avail Cap (c_a), veh/h	355	0	237	0	683	0	237	0
Upstream Filter (l)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	33.8	0.0	36.2	0.0	31.8	0.0	33.3	0.0
Incr Delay (d2), s/veh	7.4	0.0	14.4	0.0	3.8	0.0	118.9	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	41.3	0.0	50.6	0.0	35.6	0.0	152.1	0.0
1st-Term Q (Q1), veh/ln	2.3	0.0	1.1	0.0	3.2	0.0	4.1	0.0
2nd-Term Q (Q2), veh/ln	0.4	0.0	0.3	0.0	0.3	0.0	7.8	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	2.7	0.0	1.4	0.0	3.4	0.0	11.9	0.0
%ile Storage Ratio (RQ%)	0.42	0.00	0.12	0.00	0.43	0.00	1.36	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	11.1	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.3	0.0

Middle Lane Group Data

Assigned Mvmt	0	2	0	4	0	6	0	8
Lane Assignment				T		T		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	174	0	308	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1870	0	1805	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	5.2	0.0	12.2	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	5.2	0.0	12.2	0.0	0.0
Lane Grp Cap (c), veh/h	0	0	0	621	0	403	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.28	0.00	0.76	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	621	0	590	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	18.8	0.0	27.8	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.2	0.0	3.5	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	19.1	0.0	31.4	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	2.1	0.0	4.9	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.0	0.4	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis  
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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	2.1	0.0	5.3	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.10	0.00	0.22	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>							
Assigned Mvmt	0	12	0	14	0	16	0
Lane Assignment		T+R		R		T+R	T+R
Lanes in Grp	0	1	0	1	0	1	0
Grp Vol (v), veh/h	0	410	0	427	0	289	0
Grp Sat Flow (s), veh/h/ln	0	1864	0	1572	0	1660	0
Q Serve Time (g_s), s	0.0	15.8	0.0	19.0	0.0	12.5	0.0
Cycle Q Clear Time (g_c), s	0.0	15.8	0.0	19.0	0.0	12.5	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.06	0.00	1.00	0.00	0.80	0.00
Lane Grp Cap (c), veh/h	0	497	0	522	0	371	0
V/C Ratio (X)	0.00	0.83	0.00	0.82	0.00	0.78	0.00
Avail Cap (c_a), veh/h	0	974	0	522	0	542	0
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
Uniform Delay (d1), s/veh	0.0	26.4	0.0	23.4	0.0	27.9	0.0
Incr Delay (d2), s/veh	0.0	3.5	0.0	9.8	0.0	4.4	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	29.9	0.0	33.3	0.0	32.3	0.0
1st-Term Q (Q1), veh/ln	0.0	6.4	0.0	6.6	0.0	4.6	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.5	0.0	1.4	0.0	0.5	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00
%ile Back of Q (50%), veh/ln	0.0	6.9	0.0	8.0	0.0	5.1	0.0
%ile Storage Ratio (RQ%)	0.00	0.55	0.00	0.40	0.00	0.21	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>							
HCM 6th Ctrl Delay		45.0					
HCM 6th LOS		D					

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					↑	↑	↑	↑		↑	↑	
Traffic Volume (veh/h)	0	0	0	181	1	139	127	577	0	0	388	372
Future Volume (veh/h)	0	0	0	181	1	139	127	577	0	0	388	372
Number				7	4	14	5	2	12	1	6	16
Initial Q, veh				0	0	0	0	0	0	0	0	0
Ped-Bike Adj (A_pbT)				1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus Adj				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach				No			No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln				1870	1900	1900	1856	1870	0	0	1870	1870
Adj Flow Rate, veh/h				197	1	151	138	627	0	0	422	404
Peak Hour Factor				0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %				2	0	0	3	2	0	0	2	2
Opposing Right Turn Influence				Yes			Yes			No		
Cap, veh/h				318	2	284	444	1102	0	0	670	598
HCM Platoon Ratio				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green				0.18	0.18	0.18	0.09	0.59	0.00	0.00	0.38	0.38
Unsig. Movement Delay												
Ln Grp Delay, s/veh				18.2	0.0	17.5	7.6	5.9	0.0	0.0	11.8	12.4
Ln Grp LOS				B	A	B	A	A	A	A	B	B
Approach Vol, veh/h				349			765				826	
Approach Delay, s/veh				17.9			6.2				12.1	
Approach LOS					B			A			B	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs				2		4	5	6				
Case No				4.0		11.0	1.2	8.0				
Phs Duration (G+Y+Rc), s				30.1		12.5	9.0	21.1				
Change Period (Y+Rc), s				5.0		5.0	5.0	5.0				
Max Green (Gmax), s				30.0		20.0	20.0	30.0				
Max Allow Headway (MAH), s				5.0		4.7	3.7	5.2				
Max Q Clear (g_c+l1), s				10.8		6.3	3.7	11.1				
Green Ext Time (g_e), s				3.8		1.3	0.3	5.0				
Prob of Phs Call (p_c)				1.00		0.98	1.00	1.00				
Prob of Max Out (p_x)				0.05		0.02	0.00	0.13				
Left-Turn Movement Data												
Assigned Mvmt					7	5	1					
Mvmt Sat Flow, veh/h					1801	1767	0					
Through Movement Data												
Assigned Mvmt				2		4		6				
Mvmt Sat Flow, veh/h				1870		9		1870				
Right-Turn Movement Data												
Assigned Mvmt				12		14		16				
Mvmt Sat Flow, veh/h				0		1610		1585				
Left Lane Group Data												
Assigned Mvmt	0	0	0	7	5	1	0	0				
Lane Assignment					L+TL	(Pr/Pm)						

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Lanes in Grp	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	198	138	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1810	1767	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	4.3	1.7	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	4.3	1.7	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	0	658	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	18.1	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	7.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	2.9	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	16.1	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	0.99	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	319	444	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.62	0.31	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	849	1106	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	16.2	7.2	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	2.0	0.4	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	18.2	7.6	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	1.5	0.3	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	1.7	0.4	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.11	0.06	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	627	0	0	0	422	0	0
Grp Sat Flow (s), veh/h/ln	0	1870	0	0	0	1777	0	0
Q Serve Time (g_s), s	0.0	8.8	0.0	0.0	0.0	8.3	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	8.8	0.0	0.0	0.0	8.3	0.0	0.0
Lane Grp Cap (c), veh/h	0	1102	0	0	0	670	0	0
V/C Ratio (X)	0.00	0.57	0.00	0.00	0.00	0.63	0.00	0.00
Avail Cap (c_a), veh/h	0	1317	0	0	0	1251	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	5.4	0.0	0.0	0.0	10.8	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.5	0.0	0.0	0.0	1.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	5.9	0.0	0.0	0.0	11.8	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	1.4	0.0	0.0	0.0	2.2	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.0	0.0	0.2	0.0	0.0

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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	1.6	0.0	0.0	0.0	2.4	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.13	0.00	0.00	0.00	0.12	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment				R		T+R		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	151	0	404	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1610	0	1585	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	3.6	0.0	9.1	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	3.6	0.0	9.1	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	284	0	598	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.53	0.00	0.68	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	756	0	1116	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	16.0	0.0	11.1	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	1.5	0.0	1.3	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	17.5	0.0	12.4	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	0.0	0.0	2.2	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.2	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	0.1	0.0	2.4	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.01	0.00	0.12	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay				10.8				
HCM 6th LOS				B				

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑						↑	↑	↑	↑	
Traffic Volume (veh/h)	312	1	179	0	0	0	0	391	261	149	420	0
Future Volume (veh/h)	312	1	179	0	0	0	0	391	261	149	420	0
Number	7	4	14				5	2	12	1	6	16
Initial Q, veh	0	0	0				0	0	0	0	0	0
Ped-Bike Adj (A_pbT)	1.00		1.00				1.00		1.00	1.00		1.00
Parking Bus Adj	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No						No			No		
Lanes Open During Work Zone												
Adj Sat Flow, veh/h/ln	1870	1900	1856				0	1856	1841	1856	1870	0
Adj Flow Rate, veh/h	343	1	197				0	430	287	164	462	0
Peak Hour Factor	0.91	0.91	0.91				0.91	0.91	0.91	0.91	0.91	0.91
Percent Heavy Veh, %	2	0	3				0	3	4	3	2	0
Opposing Right Turn Influence	Yes						No			Yes		
Cap, veh/h	469	2	422				0	589	495	418	986	0
HCM Platoon Ratio	1.00	1.00	1.00				1.00	1.00	1.00	1.00	1.00	1.00
Prop Arrive On Green	0.26	0.26	0.26				0.00	0.32	0.32	0.10	0.53	0.00
Unsig. Movement Delay												
Ln Grp Delay, s/veh	18.3	0.0	15.6				0.0	16.2	14.7	10.0	7.4	0.0
Ln Grp LOS	B	A	B				A	B	B	B	A	A
Approach Vol, veh/h	541							717			626	
Approach Delay, s/veh	17.3							15.6			8.1	
Approach LOS	B							B			A	
Timer:	1	2	3	4	5	6	7	8				
Assigned Phs	1	2		4		6						
Case No	1.2	7.0		10.0		4.0						
Phs Duration (G+Y+Rc), s	10.0	20.1		17.5		30.1						
Change Period (Y+Rc), s	5.0	5.0		5.0		5.0						
Max Green (Gmax), s	20.0	30.0		30.0		30.0						
Max Allow Headway (MAH), s	3.7	4.6		4.4		5.0						
Max Q Clear (g_c+l1), s	4.6	11.8		10.4		9.4						
Green Ext Time (g_e), s	0.4	3.3		2.2		2.6						
Prob of Phs Call (p_c)	1.00	1.00		1.00		1.00						
Prob of Max Out (p_x)	0.00	0.05		0.01		0.01						
Left-Turn Movement Data												
Assigned Mvmt	1	5		7								
Mvmt Sat Flow, veh/h	1767	0		1781								
Through Movement Data												
Assigned Mvmt	2		4		6							
Mvmt Sat Flow, veh/h	1856		8		1870							
Right-Turn Movement Data												
Assigned Mvmt	12		14		16							
Mvmt Sat Flow, veh/h	1560		1603		0							
Left Lane Group Data												
Assigned Mvmt	1	5	0	7	0	0	0	0				
Lane Assignment	L (Pr/Pm)			L								

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Lanes in Grp	1	0	0	1	0	0	0	0
Grp Vol (v), veh/h	164	0	0	343	0	0	0	0
Grp Sat Flow (s), veh/h/ln	1767	0	0	1781	0	0	0	0
Q Serve Time (g_s), s	2.6	0.0	0.0	8.4	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	2.6	0.0	0.0	8.4	0.0	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	729	0	0	1781	0	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	17.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	5.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	3.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	15.1	0.0	0.0	0.0	0.0	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	418	0	0	469	0	0	0	0
V/C Ratio (X)	0.39	0.00	0.00	0.73	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	974	0	0	1121	0	0	0	0
Upstream Filter (l)	1.00	0.00	0.00	1.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	9.4	0.0	0.0	16.0	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.6	0.0	0.0	2.2	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	10.0	0.0	0.0	18.3	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.7	0.0	0.0	2.8	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.1	0.0	0.0	0.3	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	1.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.7	0.0	0.0	3.1	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.06	0.00	0.00	0.34	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	430	0	0	0	462	0	0
Grp Sat Flow (s), veh/h/ln	0	1856	0	0	0	1870	0	0
Q Serve Time (g_s), s	0.0	9.8	0.0	0.0	0.0	7.4	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	9.8	0.0	0.0	0.0	7.4	0.0	0.0
Lane Grp Cap (c), veh/h	0	589	0	0	0	986	0	0
V/C Ratio (X)	0.00	0.73	0.00	0.00	0.00	0.47	0.00	0.00
Avail Cap (c_a), veh/h	0	1167	0	0	0	1177	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	14.5	0.0	0.0	0.0	7.1	0.0	0.0
Incr Delay (d2), s/veh	0.0	1.8	0.0	0.0	0.0	0.3	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	16.2	0.0	0.0	0.0	7.4	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	3.2	0.0	0.0	0.0	1.8	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.3	0.0	0.0	0.0	0.1	0.0	0.0

HCM 6th Signalized Intersection Capacity Analysis & NY-17 SB Off-Ramp/NY-17 SB On-Ramp  
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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	3.5	0.0	0.0	0.0	1.9	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.12	0.00	0.00	0.00	0.16	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment		R		T+R				
Lanes in Grp	0	1	0	1	0	0	0	0
Grp Vol (v), veh/h	0	287	0	198	0	0	0	0
Grp Sat Flow (s), veh/h/ln	0	1560	0	1611	0	0	0	0
Q Serve Time (g_s), s	0.0	7.3	0.0	4.9	0.0	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	7.3	0.0	4.9	0.0	0.0	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	1.00	0.00	0.99	0.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	495	0	424	0	0	0	0
V/C Ratio (X)	0.00	0.58	0.00	0.47	0.00	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	981	0	1014	0	0	0	0
Upstream Filter (l)	0.00	1.00	0.00	1.00	0.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	13.6	0.0	14.8	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	1.1	0.0	0.8	0.0	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	14.7	0.0	15.6	0.0	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	2.0	0.0	1.5	0.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.1	0.0	0.1	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	2.2	0.0	1.6	0.0	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.08	0.00	0.17	0.00	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay			13.6					
HCM 6th LOS			B					

Intersection

Int Delay, s/veh 5.5

Movement	WBL	WBR	NBT	NBR	SBL	SBT
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Lane Configurations						
Traffic Vol, veh/h	8	59	33	7	62	24
Future Vol, veh/h	8	59	33	7	62	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	0	3	0	0	2	0
Mvmt Flow	10	70	39	8	74	29

Major/Minor	Minor1	Major1	Major2		
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Conflicting Flow All	220	43	0	0	47	0
Stage 1	43	-	-	-	-	-
Stage 2	177	-	-	-	-	-
Critical Hdwy	6.4	6.23	-	-	4.12	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.327	-	-	2.218	-
Pot Cap-1 Maneuver	773	1025	-	-	1560	-
Stage 1	985	-	-	-	-	-
Stage 2	859	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	736	1025	-	-	1560	-
Mov Cap-2 Maneuver	736	-	-	-	-	-
Stage 1	985	-	-	-	-	-
Stage 2	818	-	-	-	-	-

Approach	WB	NB	SB		
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HCM Control Delay, s	9	0	5.4		
HCM LOS	A				

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	979	1560	-
HCM Lane V/C Ratio	-	-	0.081	0.047	-
HCM Control Delay (s)	-	-	9	7.4	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.3	0.1	-

Intersection

Int Delay, s/veh 6.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	21	144	1	6	199	37	0	58	4	93	51	27
Future Vol, veh/h	21	144	1	6	199	37	0	58	4	93	51	27
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	4	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	1	0	17	2	0	0	0	25	1	0	4
Mvmt Flow	23	160	1	7	221	41	0	64	4	103	57	30

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	262	0	0	161	0	0	506	483	161	497	463	242
Stage 1	-	-	-	-	-	-	207	207	-	256	256	-
Stage 2	-	-	-	-	-	-	299	276	-	241	207	-
Critical Hdwy	4.1	-	-	4.27	-	-	7.1	6.5	6.45	7.91	7.3	6.64
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.91	6.3	-
Follow-up Hdwy	2.2	-	-	2.353	-	-	3.5	4	3.525	3.509	4	3.336
Pot Cap-1 Maneuver	1314	-	-	1332	-	-	480	486	827	434	450	771
Stage 1	-	-	-	-	-	-	800	734	-	709	661	-
Stage 2	-	-	-	-	-	-	714	685	-	725	701	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1314	-	-	1332	-	-	408	474	827	379	439	771
Mov Cap-2 Maneuver	-	-	-	-	-	-	408	474	-	379	439	-
Stage 1	-	-	-	-	-	-	785	720	-	696	657	-
Stage 2	-	-	-	-	-	-	623	681	-	644	688	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	1	0.2			13.6			19.8			
HCM LOS					B			C			
<hr/>											
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	487	1314	-	-	1332	-	-	431			
HCM Lane V/C Ratio	0.141	0.018	-	-	0.005	-	-	0.441			
HCM Control Delay (s)	13.6	7.8	0	-	7.7	0	-	19.8			
HCM Lane LOS	B	A	A	-	A	A	-	C			
HCM 95th %tile Q(veh)	0.5	0.1	-	-	0	-	-	2.2			

HCM 6th Signalized Intersection Capacity Analysis 50: NYS Route 208 & Museum Village Rd  
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Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	↑	↑	↑	↑	↑	↑		
Traffic Volume (veh/h)	104	166	227	456	611	89		
Future Volume (veh/h)	104	166	227	456	611	89		
Number	7	14	5	2	6	16		
Initial Q, veh	0	0	0	0	0	0		
Ped-Bike Adj (A_pbT)	1.00	1.00	1.00		1.00			
Parking Bus Adj	1.00	1.00	1.00	1.00	1.00	1.00		
Work Zone On Approach	No			No	No			
Lanes Open During Work Zone								
Adj Sat Flow, veh/h/ln	1832	1876	1876	1862	2042	2012		
Adj Flow Rate, veh/h	108	173	236	475	636	93		
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96		
Percent Heavy Veh, %	3	0	0	1	1	3		
Opposing Right Turn Influence	Yes		Yes					
Cap, veh/h	218	297	567	1469	1398	1167		
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00		
Prop Arrive On Green	0.13	0.13	0.06	0.79	0.68	0.68		
Unsig. Movement Delay								
Ln Grp Delay, s/veh	48.2	43.9	6.5	4.1	9.5	6.3		
Ln Grp LOS	D	D	A	A	A	A		
Approach Vol, veh/h	281			711	729			
Approach Delay, s/veh	45.6			4.9	9.1			
Approach LOS	D			A	A			
Timer:	1	2	3	4	5	6	7	8
Assigned Phs		2		4	5	6		
Case No		4.0		9.0	1.2	7.0		
Phs Duration (G+Y+Rc), s		97.0		19.6	12.2	84.8		
Change Period (Y+Rc), s		5.0		5.0	5.0	5.0		
Max Green (Gmax), s		92.0		48.0	30.0	57.0		
Max Allow Headway (MAH), s		3.5		4.2	4.6	3.4		
Max Q Clear (g_c+l1), s		10.4		13.6	6.2	18.6		
Green Ext Time (g_e), s		1.3		1.0	1.0	2.0		
Prob of Phs Call (p_c)		1.00		1.00	1.00	1.00		
Prob of Max Out (p_x)		0.00		0.00	0.00	0.00		
Left-Turn Movement Data								
Assigned Mvmt			7	5	1			
Mvmt Sat Flow, veh/h			1745	1787	0			
Through Movement Data								
Assigned Mvmt		2		4		6		
Mvmt Sat Flow, veh/h		1862		0		2042		
Right-Turn Movement Data								
Assigned Mvmt		12		14		16		
Mvmt Sat Flow, veh/h		0		1590		1705		
Left Lane Group Data								
Assigned Mvmt	0	0	0	7	5	1	0	0
Lane Assignment				LL (Pr/Pm)				

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Lanes in Grp	0	0	0	1	1	0	0	0
Grp Vol (v), veh/h	0	0	0	108	236	0	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1745	1787	0	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	6.7	4.2	0.0	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	6.7	4.2	0.0	0.0	0.0
Perm LT Sat Flow (s_l), veh/h/ln	0	0	0	1745	729	0	0	0
Shared LT Sat Flow (s_sh), veh/h/ln	0	0	0	0	0	0	0	0
Perm LT Eff Green (g_p), s	0.0	0.0	0.0	0.0	81.8	0.0	0.0	0.0
Perm LT Serve Time (g_u), s	0.0	0.0	0.0	0.0	63.2	0.0	0.0	0.0
Perm LT Q Serve Time (g_ps), s	0.0	0.0	0.0	0.0	8.9	0.0	0.0	0.0
Time to First Blk (g_f), s	0.0	0.0	0.0	0.0	0.0	79.8	0.0	0.0
Serve Time pre Blk (g_fs), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Prop LT Inside Lane (P_L)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	218	567	0	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.49	0.42	0.00	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	718	916	0	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	1.00	0.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	47.6	6.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.6	0.5	0.0	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	48.2	6.5	0.0	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	2.9	1.0	0.0	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	3.0	1.1	0.0	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.51	0.18	0.00	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Middle Lane Group Data								
Assigned Mvmt	0	2	0	4	0	6	0	0
Lane Assignment		T				T		
Lanes in Grp	0	1	0	0	0	1	0	0
Grp Vol (v), veh/h	0	475	0	0	0	636	0	0
Grp Sat Flow (s), veh/h/ln	0	1862	0	0	0	2042	0	0
Q Serve Time (g_s), s	0.0	8.4	0.0	0.0	0.0	16.6	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	8.4	0.0	0.0	0.0	16.6	0.0	0.0
Lane Grp Cap (c), veh/h	0	1469	0	0	0	1398	0	0
V/C Ratio (X)	0.00	0.32	0.00	0.00	0.00	0.45	0.00	0.00
Avail Cap (c_a), veh/h	0	1469	0	0	0	1398	0	0
Upstream Filter (l)	0.00	1.00	0.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	3.5	0.0	0.0	0.0	8.4	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.6	0.0	0.0	0.0	1.1	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	4.1	0.0	0.0	0.0	9.5	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	1.8	0.0	0.0	0.0	5.7	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.2	0.0	0.0	0.0	0.4	0.0	0.0

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3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	2.0	0.0	0.0	0.0	6.1	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.23	0.00	0.00	0.00	0.65	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Right Lane Group Data</b>								
Assigned Mvmt	0	12	0	14	0	16	0	0
Lane Assignment				R		R		
Lanes in Grp	0	0	0	1	0	1	0	0
Grp Vol (v), veh/h	0	0	0	173	0	93	0	0
Grp Sat Flow (s), veh/h/ln	0	0	0	1590	0	1705	0	0
Q Serve Time (g_s), s	0.0	0.0	0.0	11.6	0.0	2.1	0.0	0.0
Cycle Q Clear Time (g_c), s	0.0	0.0	0.0	11.6	0.0	2.1	0.0	0.0
Prot RT Sat Flow (s_R), veh/h/ln	0.0	0.0	0.0	1590.2	0.0	0.0	0.0	0.0
Prot RT Eff Green (g_R), s	0.0	0.0	0.0	7.2	0.0	0.0	0.0	0.0
Prop RT Outside Lane (P_R)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Lane Grp Cap (c), veh/h	0	0	0	297	0	1167	0	0
V/C Ratio (X)	0.00	0.00	0.00	0.58	0.00	0.08	0.00	0.00
Avail Cap (c_a), veh/h	0	0	0	753	0	1167	0	0
Upstream Filter (l)	0.00	0.00	0.00	1.00	0.00	1.00	0.00	0.00
Uniform Delay (d1), s/veh	0.0	0.0	0.0	43.3	0.0	6.1	0.0	0.0
Incr Delay (d2), s/veh	0.0	0.0	0.0	0.7	0.0	0.1	0.0	0.0
Initial Q Delay (d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh	0.0	0.0	0.0	43.9	0.0	6.3	0.0	0.0
1st-Term Q (Q1), veh/ln	0.0	0.0	0.0	10.2	0.0	0.6	0.0	0.0
2nd-Term Q (Q2), veh/ln	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0
3rd-Term Q (Q3), veh/ln	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile Back of Q Factor (f_B%)	0.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00
%ile Back of Q (50%), veh/ln	0.0	0.0	0.0	10.2	0.0	0.7	0.0	0.0
%ile Storage Ratio (RQ%)	0.00	0.00	0.00	0.93	0.00	0.08	0.00	0.00
Initial Q (Qb), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Final (Residual) Q (Qe), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Delay (ds), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Q (Qs), veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Sat Cap (cs), veh/h	0	0	0	0	0	0	0	0
Initial Q Clear Time (tc), h	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
<b>Intersection Summary</b>								
HCM 6th Ctrl Delay				13.3				
HCM 6th LOS				B				

Intersection

Int Delay, s/veh 1.8

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖	↗	↖	↗		
Traffic Vol, veh/h	2	8	14	43	31	4
Future Vol, veh/h	2	8	14	43	31	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	95	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	2	9	15	47	34	4

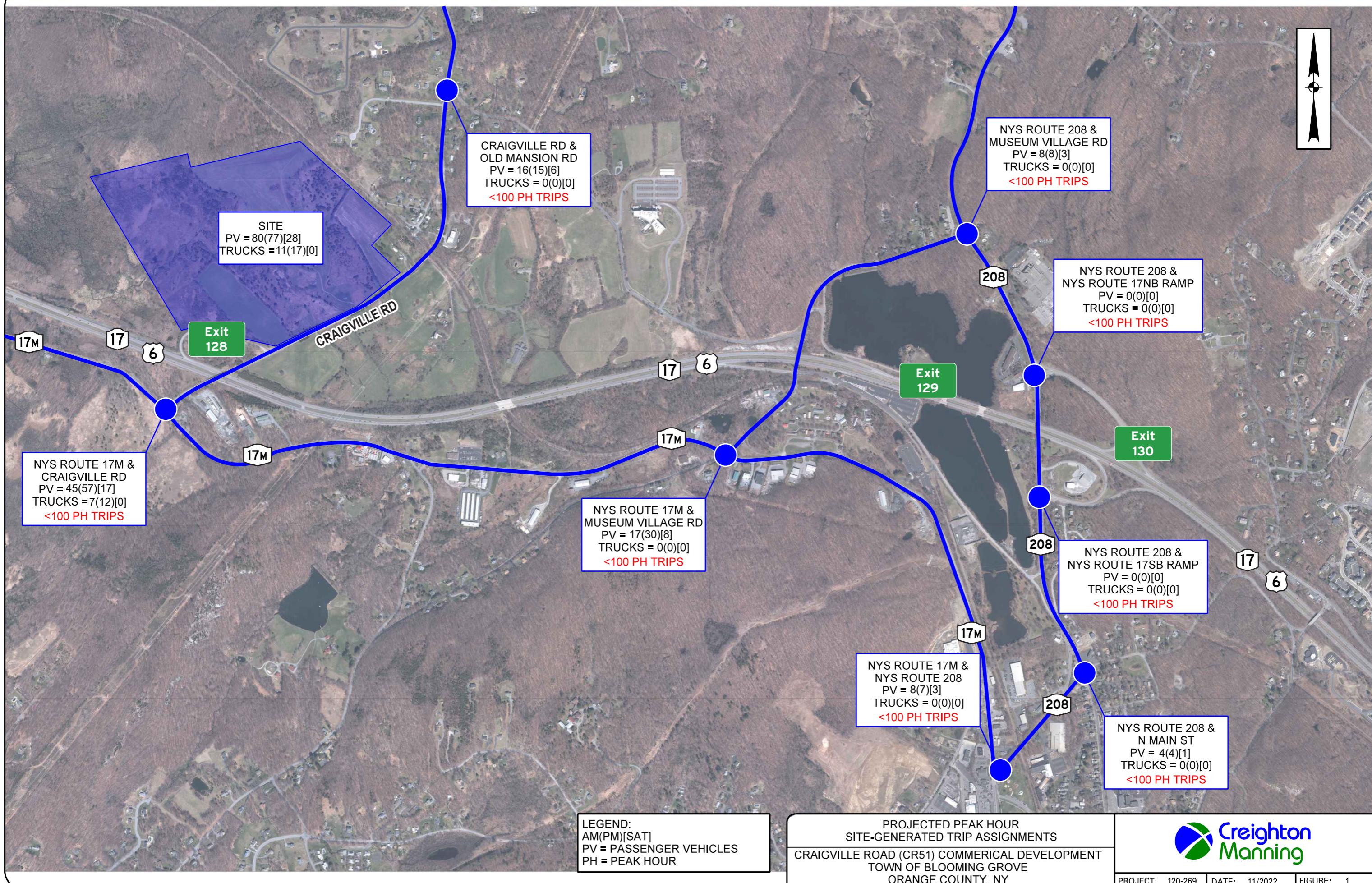
Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	113	36	38	0	-
Stage 1	36	-	-	-	-
Stage 2	77	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-
Pot Cap-1 Maneuver	888	1042	1585	-	-
Stage 1	992	-	-	-	-
Stage 2	951	-	-	-	-
Platoon blocked, %				-	-
Mov Cap-1 Maneuver	879	1042	1585	-	-
Mov Cap-2 Maneuver	879	-	-	-	-
Stage 1	982	-	-	-	-
Stage 2	951	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	8.6	1.8	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	EBLn2	SBT	SBR
Capacity (veh/h)	1585	-	879	1042	-	-
HCM Lane V/C Ratio	0.01	-	0.002	0.008	-	-
HCM Control Delay (s)	7.3	0	9.1	8.5	-	-
HCM Lane LOS	A	A	A	A	-	-
HCM 95th %tile Q(veh)	0	-	0	0	-	-

**ATTACHMENT F**  
**FIGURE OF OTHER OFF-SITE INTERSECTIONS**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK



## **ATTACHMENT G**

### **SIGNAL WARRANT ANALYSIS**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK

Project:	120-269; Indigo Contracting																									
Intersection:	NYS Route 17M/Craigville Rd																									
Date:	5/9/2022																									
Analyst:	FMP																									
Select your lane configuration																										
	Main 1, Side 1	4	1																							
		3		100% (a)	80% (b)	70% (c)	56% (d)		100% (a)	80% (b)	70% (c)	56% (d)														
				500	400	350	280		750	600	525	420														
				150	120	105	84		75	60	53	42														
Add your volumes																										
Hour	17M	Craigville	Condition A										Condition B													
Beginning			100%			80%			70%			56%			100%			80%			70%			56%		
			Main	SS 1	SS 2	Overall	Main	SS 1	SS 2	Overall	Main	SS 1	SS 2	Overall	Main	SS 1	SS 2	Overall	Main	SS 1	SS 2	Overall	Main	SS 1	SS 2	Overall
12:00 AM	30	3																								
1:00 AM	13	3																								
2:00 AM	9	1																								
3:00 AM	6	1																								
4:00 AM	19	1																								
5:00 AM	58	4																								
6:00 AM	143	24																								
7:00 AM	337	42																								
8:00 AM	402	40																								
9:00 AM	378	43																								
10:00 AM	393	39																								
11:00 AM	400	45																								
12:00 PM	414	51																								
1:00 PM	442	54																								
2:00 PM	480	63																								
3:00 PM	513	72																								
4:00 PM	542	88																								
5:00 PM	530	88																								
6:00 PM	363	51																								
7:00 PM	239	36																								
8:00 PM	162	30																								
9:00 PM	97	16																								
10:00 PM	63	10																								
11:00 PM	41	7																								
	Hours Met	0		0		0		2		0		0		2		8		8		2		8		5		
	Required	8		8		8		8		8		8		8		No		No		No		No		8		
	Warrant Met?	No		No		No		No		No		No		No		No		No		No		No		No		
<b>NOTES:</b>																										
(a) Basic minimum hourly volume.																										
(b) Used for combination of Conditions A and B after adequate trial of other remedial measures.																										
(c) May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000.																										
(d) May be used for combination of Conditions A and B after adequate trial of other remedial measures when the major street speed exceeds 40 mph or in an isolated community with a population of less than 10,000.																										

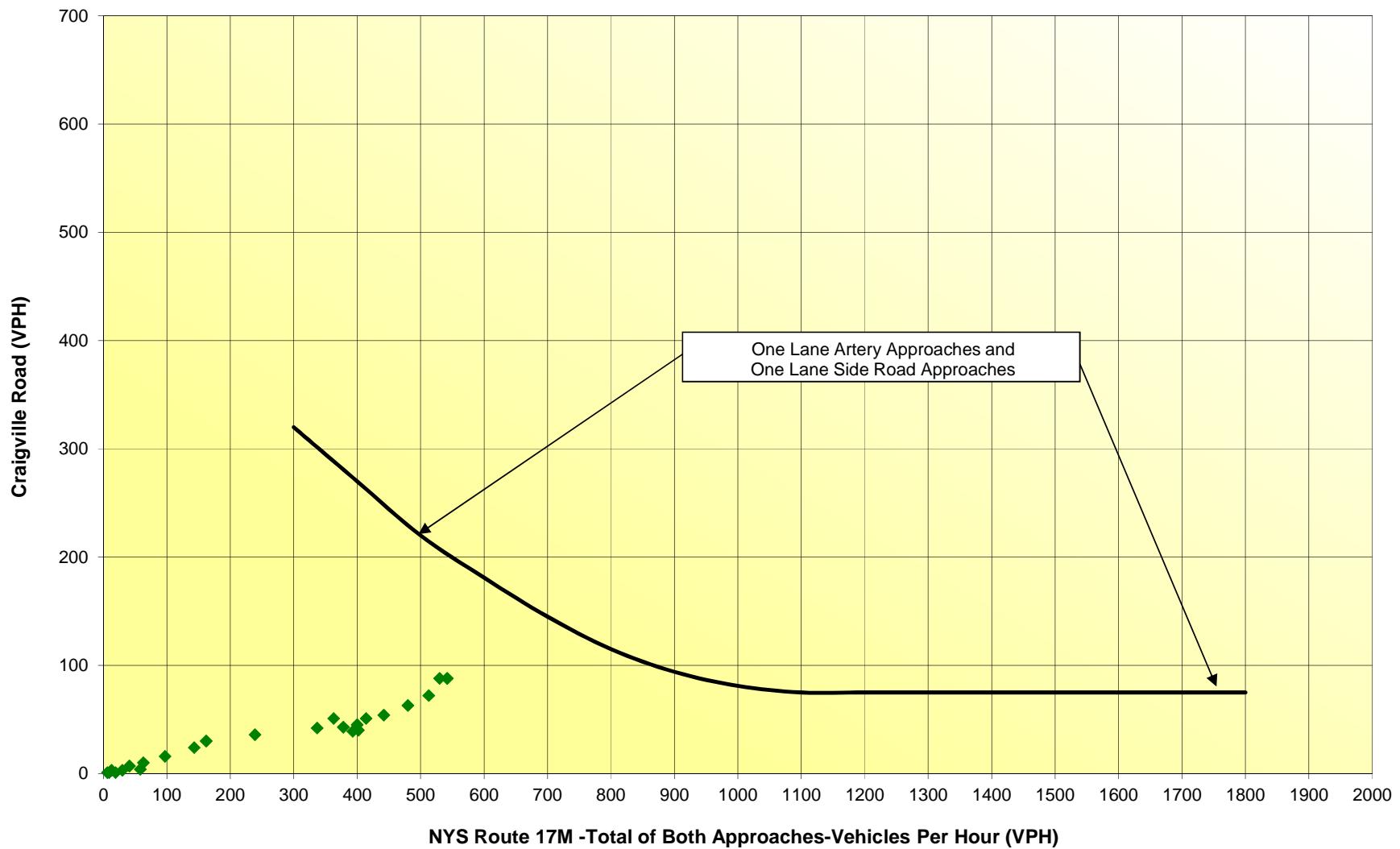


**NYS Route 17M/Craigville Road (No-Build Conditions)**

**Figure 4C-4**

**Reduced Peak Hour Volume Warrant (No-Build)**

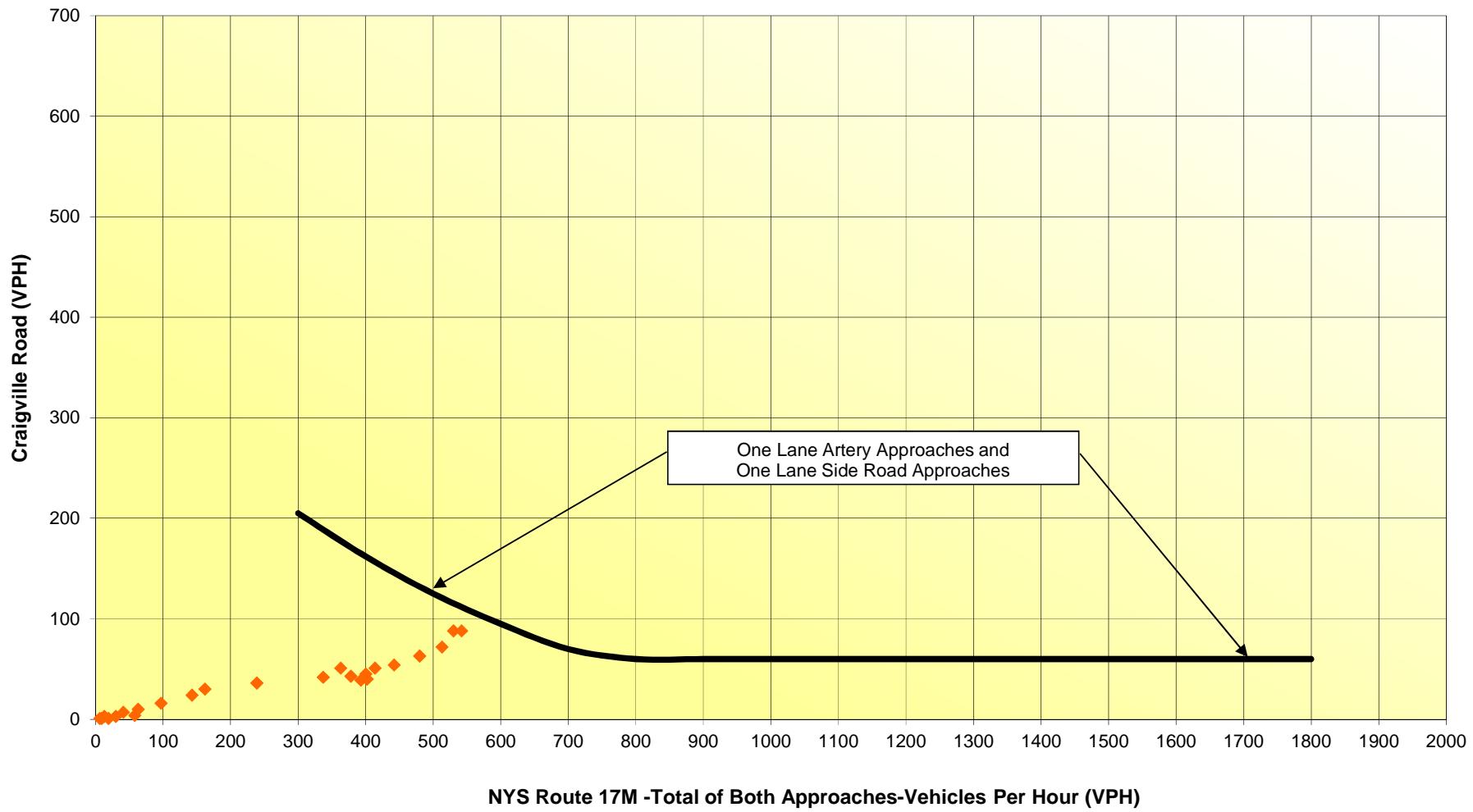
Source: Federal MUTCD



**NYS Route 17M/Craigville Road (No-Build Conditions)**

**Figure 4C-2**  
**Reduced Four-Hour Vehicular Volume Warrant**

Source: Federal MUTCD

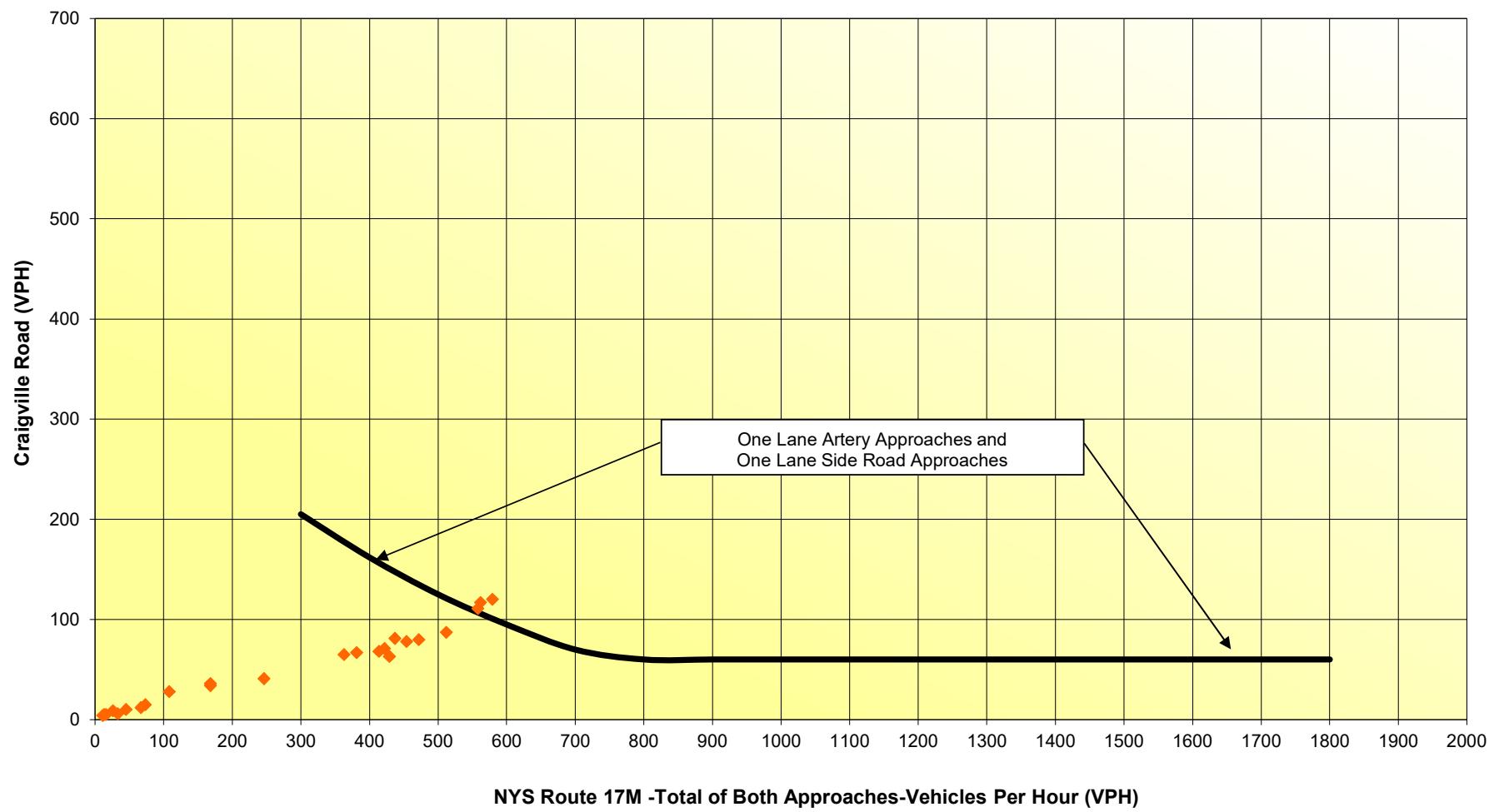


**NYS Route 17M/ Craigville Road (Build Conditions)**

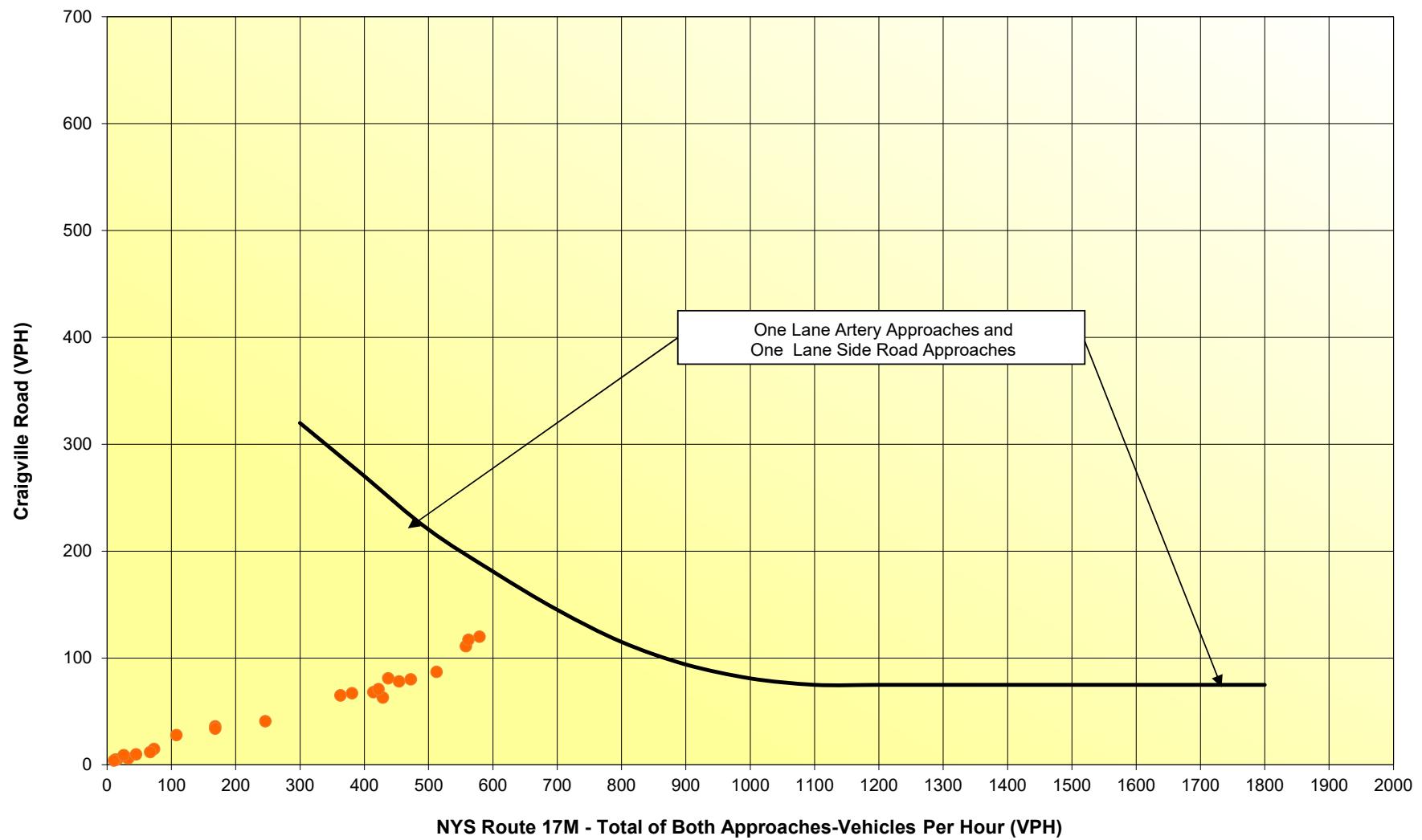
**Figure 4C-2**

**Reduced Four-Hour Vehicular Volume Warrant**

Source: Federal MUTCD



**NYS Route17M/Craigville Road (Build Conditions)**  
**Figure 4C-4**  
**Reduced Peak Hour Volume Warrant Source: Federal MUTCD**



**ATTACHMENT H**  
**OCDPW SITE DRIVEWAY LOCATION APPROVAL**

CRAIGVILLE ROAD WAREHOUSE  
TOWN OF BLOOMING GROVE  
ORANGE COUNTY, NEW YORK



## ORANGE COUNTY DEPARTMENT OF PUBLIC WORKS

**Steven M. Neuhaus**  
*County Executive*

**Erik Denega, P.E., P.M.P.**

*Commissioner*

**P.O. Box 509, 2455-2459 Route 17M**

**Goshen, NY 10924-0509**

[www.orangecountygov.com](http://www.orangecountygov.com)

**TEL (845) 291-2750 FAX (845) 291-2778**

**VIA E-MAIL:** shipp@cmellp.com

Starke Hipp, P.E.  
Creighton Manning  
145 Main Street  
Ossining, NY 10562

April 11, 2022

Re: **Conceptual Approval for Access and Driveway Location**  
Craigville Logistics Warehouse  
County Road 51 (Craigville Rd)  
Town of Blooming Grove, SBL: 52-5-11 & 54-1-50.1  
OCDPW Review #20-0057

Dear Mr. Hipp:

This Department has reviewed the following documents:

- Site Plan Prepared by Arden Consulting Engineers, PLLC titled "Turning Diagrams" Sheet 11 of 11 dated October 26, 2020, last revised June 25, 2021.
- Email Correspondence dated January 27, 2022, from Creighton Manning.

Based on our review of the documents noted, conceptual approval is hereby granted by the Orange County Department of Public Works for the location of the access driveway for the proposed warehouse. The location will provide for adequate sight distance as requested during a field meeting on October 6, 2020. We note that this conceptual approval is limited to driveway location only.

A full technical review is still required prior to full approval being granted by this office. A full technical review will include traffic impacts and mitigation, and driveway entrance standards.

Please provide one (1) hard copy and a digital copy (pdf) of all submitted materials. Should you have any questions, please contact this office at your earliest convenience.

Yours truly,  
Orange County Department of Public Works

A handwritten signature in black ink, appearing to read "Anthony Trochiano".

Anthony Trochiano, P.E.  
Principal Engineer

copy: Travis Ewald, PE, Deputy Commissioner (via e-mail)  
Michael Villarosa, PE, Principal Engineer (via e-mail)  
Mike Carroll, Senior Engineer (via e-mail)  
Megan Tennermann, AICP, OC Planning (via e-mail)  
Town of Blooming Grove Planning Board (via e-mail)  
Frank Filiciotto, PE (via e-mail)  
Michael Morgante, PE (via email)  
Steve Esposito, RLA (via email)